

**ADDENDUM #5**

FROM : ARCHITECTURICA  
555 W. BENJAMIN HOLT DRIVE  
SUITE 423  
STOCKTON, CA 95207  
(209) 952-5850 FAX (209) 952-2442

PROJECT : CODESTACK ACADEMY

LOCATION : 201 North California Street  
Stockton CA 95202

REF. # : ARCH PROJ. No. 2023-04

OWNER : San Joaquin County of Education  
2922 Transworld Drive  
Stockton, CA 95206

DATE : 6 December 2024

**NOTICE TO ALL BIDDERS**

**IT IS THE PURPOSE AND INTENT OF THIS ADDENDUM TO MODIFY AND/OR CLARIFY THE DRAWINGS AND SPECIFICATIONS FOR THIS PROJECT AND THIS ADDENDUM SHALL BECOME A PART OF THE CONTRACT DOCUMENTS. THESE CHANGES AND/OR INTERPRETATIONS SHALL BE INCORPORATED INTO YOUR BID.**

**REFER TO PROJECT PLANS AND SPECIFICATIONS PREPARED BY ARCHITECTURICA, COVER SHEET DATED 4 November 2024**

**REQUEST FOR INFORMATION (RFI) - Responses to RFI's (those noted as CLOSED), are included at end of this Addenda document. If OPEN they will be addressed in a future Addenda.**

RFI #	VENDOR	COMMENT	DATE RECEIVED	CLOSED
01 (Items 1-8)	STRUCTURAL STEEL (for F&H Construction)	NEW STRUCTURAL DRAWINGS PROVIDED. ITEM 6 – TO BE ADDRESSED IN FUTURE ADDENDA	12-02-24	ITEMS 1-5, 7-8
02 (Item 1)	STRUCTURAL STEEL (for F&H Construction)	NEW STRUCTURAL DRAWINGS PROVIDED.	12-02-24	X
02 (Item 2)	STRUCTURAL STEEL (for F&H Construction)	STAIR SUPPORTS TO BE ADDRESSED FUTURE ADDENDA	12-02-24	OPEN
03	F&H CONSTRUCTION	SOILS REPORT PROVIDED ON WEB LINK	12-02-24	X
04	STRUCTURAL STEEL (for F&H Construction)	STAIR DETAILING	12-02-24	OPEN

**ADDENDUM #5**

05	F&H CONSTRUCTION	DOOR SCHEDULE FOR 3 <sup>RD</sup> FLOOR ATTACHED.	12-03-24	X
06	F&H CONSTRUCTION	SKYFOLD SPECIFICATION 10 22 39 PROVIDED	12-03-24	X
07	HOIRZON GLASS (for F&H Construction)	INTERIOR STOREFRONT OR PARTITION	12-03-24	OPEN

**GENERAL**

ITEM 1 PROJECT MANUAL – updates and additions to project manual

SECTION	TITLE	NOTES	PAGES
07 21 00	Thermal Insulation	Replaces existing section Exterior insulation is Dryvit system EIFS.	(4)
10 22 39	Skyfold Wall System	New section	(7)

**ARCHITECTURAL**

ITEM 1 SHEETS: A8.1 & A8.2

Sheets updated to include door schedule for 3<sup>rd</sup> floor that was missing.

**STRUCTURAL**

ITEM 1 NEW DOCUMENT SET

ALL PAGES REPLACED. The following is a summary of pages that are most affected by revisions.

SHEET	SHEET NAME	NOTE
S2.1	BASEMENT FOUNDATION PLAN	CHANGE IN POST SIZES AT BRACE FRAME
S2.2	1 <sup>ST</sup> FLOOR FRAMING PLAN	REVISED MARGIN NOTES, NOTES AND DETAIL CALLOUTS ADDED TO PLAN
S2.2	1 <sup>ST</sup> FLOOR FRAMING PLAN	CHANGE IN POST SIZES AT BRACE FRAME
S2.3	2 <sup>ND</sup> FLOOR FRAMING PLAN	REVISED MARGIN NOTES, NOTES AND DETAIL CALLOUTS ADDED TO PLAN
S2.4	3 <sup>RD</sup> FLOOR FRAMING PLAN	REVISED MARGIN NOTES, NOTES AND DETAIL CALLOUTS ADDED TO PLAN
S2.5	ROOF FRAMING PLAN	REVISED MARGIN NOTES, NOTES AND DETAIL CALLOUTS ADDED TO PLAN

**ADDENDUM #5**

S2.6	COLUMN LOAD SUMMARY AT FOUNDATION	REVISED CALCULATIONS
S3.1, S3.2, S3.3	FRAME ELEVATIONS	REVISIONS TO MEMBERS
S4.1	SECTIONS	ADDED SECTIONS & NOTATIONS
S4.2, - S4.6	SECTIONS	ADDED OR CORRECTED NOTATIONS
S4.8	DETAILS	WAS SHEET S4.9
S4.9	DETAILS	WAS SHEET S4.8, ADDED DETAILS
S5.1 - S5.6	DETAILS	ADDED OR REVISED DETAILS

Attachments: Sheet A8.1, A8.2, all structural sheets.

END OF ADDENDUM #5

ARCHITECTURICA

By



Tim Dearborn, AIA  
Architect



PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-1

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:			
FROM:		EMAIL:	
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

<p><b>REQUESTED CLARIFICATION:</b></p> <p>1)At 1st floor (ground level) on S2.2 there are many beams that land at perimeter walls, some go into columns but beams that land at wall have zero details to attach to the 24" grade beams. Further complicating this is there are solid lines at perimeter grid lines (between all perimeter columns) that could be steel beams which might carry beam ends but these lines are not called out as steel beams. Please define lines at perimeter walls at 1st level and or provide connection details for beams to connect to grade beams.</p>
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<p><b>RESPONSE TO CLARIFICATION:</b></p> <p style="text-align: center; color: red;">See detail 9/S5.1 for beam pocket at concrete wall/grade beam. This detail is referenced as typ at grids J&amp;9.</p>
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Attach additional numbered sheets as necessary; however, only one (1) request shall be contained on each submitted form.



PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-2

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
<p>2)All S drawing plans, grid lines are too faint to see, please have grid lines darkened so they can be followed correctly.</p>

RESPONSE TO CLARIFICATION:
<p style="text-align: center;">Grid lines darkened.</p>

Attach additional numbered sheets as necessary; however, only one (1) request shall be contained on each submitted form.

PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-3

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
<p>3) Braced frame details drawing S5.2 has many issues. Det 3/S5.2 provides base plate thickness for 3 sizes of HSS columns but it does not provide for HSS 7x7 or 8x8 columns with varying wall thicknesses, anchor bolt info is also needed for the larger HSS sizes, see also details 7&amp;8/S5.2 which show different details for HSS5x5 and HSS6x6 braced column bases. Again no details for HSS7x7 or HSS8x8. Details 1&amp;2/S5.2 call out drag bars welded to braced base plates and say “per braced frame elevations” drag bars are not called out in elevations S3.1-S3.3. There are other typos on this S5.2 but this drawing simply needs to be finished or provide all missing or conflicting info.</p>

RESPONSE TO CLARIFICATION:
<p>Drawings have been updated.</p>

Attach additional numbered sheets as necessary; however, only one (1) request shall be contained on each submitted form.

PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-4

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
<p>4)B/S3.1 bottom of Line 1.1 shows a brace end connection to basement floor without a column base plate, please provide a connection detail for this brace to floor/wall connection.</p>

RESPONSE TO CLARIFICATION:
<p>Drawings have been updated.</p>

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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-5

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
<p>5)Second Floor plan S2.3. There appear to be pop outs along grid but all column base plates are shown only on Line 1.1Both W18x50 braced beams shown in B&amp;C/S3.2 Line 1.1 are shown inset on plan towards 1.2. Section cuts on this line A/S4.2 are very obviously unfinished, Please clarify pop outs on Line 1.1 and Line 1.</p>

RESPONSE TO CLARIFICATION:
<p>See B/S4.2</p>

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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-6

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
<p>6)Awnings shown on A9.5.1 and A9.5.2 give brief descriptions of steel framing and say see SSD's. Drawing S2.3 show awning posts but no callouts, drawing S5.6 provide few details but not enough to do proper take-off, please provide balance of awning framing info. Please also note the custom perforated panels atop awnings are aluminum, a dissimilar metals solution should be provided to attach aluminum panels to carbon steel framing.</p>

RESPONSE TO CLARIFICATION:
<p>Details will be coordinated with architectural</p>

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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-7

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
7)“FDST” is called out on roof plan S2.5 along Line 9 and at elevator, please define FDST?

RESPONSE TO CLARIFICATION:
see Floor Framing Note #19 and 2/S5.3

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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #01-8

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	December 2, 2024		
FROM:	F&H Construction Brian	EMAIL:	estimating@f-hconst.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
<p>8) Braced Frame connections next to existing walls South, East and West sides. Only East and West sides have section cuts showing distance between centerlines of steel framing and face of existing walls, see A/S4.4 and A/S4.5 which show that gap to be 1'-1", with grid lines so faint as noted in question #2 we cannot determine the gap dimension on Line 9. We simply want to bring to the attention of the design team we are very concerned about making all field welds in this small gap, especially when many joints will be deemed AESS. Some joints might be accessed through existing wall openings but even without AESS requirements we are concerned about having adequate access to make all welds between perimeter steel and existing concrete walls.</p>

RESPONSE TO CLARIFICATION:
<p>Drawings updated and grid lines darkened</p>

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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #02-1

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:	DECEMBER 2, 2024		
FROM:	F&H Construction Brian Roek	EMAIL:	estimating@f-hconstruction.com
DOCUMENT/DIVISION NUMBER:	Structural Steel	DRAWING NUMBER:	

<p>REQUESTED CLARIFICATION:</p> <p>9) Gravity column bases are shown in basement S2.1 with detail 4/S5.1, however this detail does not address HSS columns over HSS 5x5x 1/2" there are 6", 7" and even 12" columns that need base plate and AB info.</p>
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<p>RESPONSE TO CLARIFICATION:</p> <p>See the HSS column base plate schedule in 6/S5.1. Detail 4/S5.1 refers to detail 6/S5.1 for base plate information.</p> <p>by Teri Baughman - Bevier Structural Engineering, Inc 12-3-2024</p>
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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #03

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:			
FROM:	F&H Construction Brian Roek	EMAIL:	estimating@f-hconstruction.com
DOCUMENT/DIVISION NUMBER:	00 31 32 Geotechnical Data	DRAWING NUMBER:	

REQUESTED CLARIFICATION:
Section 00 31 32 Geotechnical Data states that a soil investigation was done and that the report is available for review. May we have a copy of the report?

RESPONSE TO CLARIFICATION:
<p>12-02-24 - LINK TO SOILS REPORT PROVIDED THRU INITIAL WEB POSTING. TIM DEARBORN, ARCHITECHNICA</p>

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PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #05

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:			
FROM:	F&H Construction Brian Roek	EMAIL:	estimating@f-hconstruction.com
DOCUMENT/DIVISION NUMBER:	Door Schedule	DRAWING NUMBER:	A8.1

**REQUESTED CLARIFICATION:**

There are several doors on the third floor that aren't called out on the door schedule please advise.

**RESPONSE TO CLARIFICATION:**

**THIRD FLOOR DOOR SCHEDULE WAS CUT OFF IN DRAWING SHEET.  
REVISED SHEETS A8.1 & A8.2 FOR 3RD FLOOR DOOR SCHEDULE - ATTACHED.**

**LEILANI GREGORY - ARCHITECHNICA 12-04-24**

Attach additional numbered sheets as necessary; however, only one (1) request shall be contained on each submitted form.

DOOR SCHEDULE

DOOR TAG MARK	DOOR TYPE MARK	FROM ROOM: NAME	FROM ROOM: NUMBER	TO ROOM: NAME	TO ROOM: NUMBER	WIDTH	HEIGHT	FRAME MATERIAL	DOOR MATERIAL	FIRE RATING ASSEMBLY	HARDWARE SET TYPE	ACCESS CONTROL	PANIC HARDWARE	WEATHER STRIP	DOOR CLOSER	KICKPLATE	THRESHOLD	SIGNAGE	COMMENTS
BASEMENT SLAB																			
001	D3	CORRIDOR 00	0-02	STAIR -02	0-00	3'-0"	8'-0"	METAL	SC WOOD	1.5 HRS	PASSAGE	Yes	Yes	Yes	Yes	Yes	4 / A9.8.1	STAIR 02 TO EXIT	SOFT CLOSE, LHR FROM LOCKABLE SIDE, TEMPERED GLASS WINDOW
002	D5	CORRIDOR 00	0-02	WOMENS RESTROOM	0-03	3'-0"	7'-0"	METAL	SC WOOD		RESTROOM - MULTI						13C / A9.8.1	WOMEN	
003	D5	CORRIDOR 00	0-02	MEN'S RESTROOM	0-04	3'-0"	7'-0"	METAL	SC WOOD		RESTROOM - MULTI						13C / A9.8.1	MEN	
004	D9	CORRIDOR 00	0-02	IT TEAM ROOM	0-07	6'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY	Yes					13D / A9.8.1		
004.1	D4	IT TEAM ROOM	0-07	SERVER ROOM	0-05	6'-0"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD	Yes					13A / A9.8.1		NO LOUVERS
004.2	SL8	IT TEAM ROOM	0-07	TYLER'S OFFICE	0-06	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								
005	D4	CORRIDOR 00	0-02	MEDIA STUDIO	0-08	6'-0"	7'-0"	METAL	SC WOOD		OFFICE PRIVACY	Yes					13D / A9.8.1		ACOUSTIC RATED (STC 45), NO LOUVERS
006	D7	CORRIDOR 00	0-02	MEDIA OFFICE	0-09	3'-0"	7'-0"	STOREFRONT	GLASS		OFFICE PRIVACY	Yes					13D / A9.8.1		
006.1	D6	MEDIA OFFICE	0-09	STORAGE	0-10	3'-0"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD	Yes					13A / A9.8.1		
006.2	GL8	MEDIA STUDIO	0-08	MEDIA OFFICE	0-09	3'-0"	6'-11"	STOREFRONT	GLASS		OFFICE PRIVACY								
007	D3	CORRIDOR 00	0-02	STAIR-01	0-11	3'-0"	7'-0"	STOREFRONT	GLASS	1.5 HRS	PASSAGE	Yes	Yes	Yes	Yes	Yes	4 / A9.8.1	STAIR 01 TO EXIT	SOFT CLOSE, LHR FROM LOCKABLE SIDE
008	D3	CORRIDOR 00	0-02	ELECTRICAL SWITCH GEAR ROOM	0-12	3'-0"	7'-0"	METAL	H.METAL	60 MIN.	STOREROOM - STANDARD	Yes	Yes	Yes	Yes	Yes	4 / A9.8.1	ELECTRICAL ROOM	
009	D6	CORRIDOR 00	0-02	EDITING ROOM	0-13	3'-0"	7'-0"	METAL	SC WOOD		OFFICE PRIVACY						13D / A9.8.1	EDITING ROOM	
011	D4	CORRIDOR 00	0-02	STORAGE ROOM	0-14	6'-0"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD	Yes					13A / A9.8.1		
017	D6	CORRIDOR 00	0-02	BOOSTER/ PUMP	0-15	3'-0"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD						13A / A9.8.1	BOOSTER PUMP	
018	D4	CORRIDOR 00	0-02	BOOSTER/ PUMP	0-15	6'-0"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD						13A / A9.8.1	WATER HEATER	
FIRST FLOOR																			
100	D9	SERVICE LOBBY-01	1-00	EXTERIOR		6'-0"	8'-0"	STOREFRONT	GLASS		ENTRY	Yes	Yes	Yes	Yes	Yes	3/A9.8.2	EXIT	REMOVEABLE ASTRIGAL
101	GL8	CORRIDOR-101	1-02	EXTERIOR		3'-1"	8'-0"	STOREFRONT	GLASS		ENTRY	Yes	Yes	Yes	Yes	Yes	3/A9.8.2		
102	D9	ENTRY LOBBY	1-09	EXTERIOR		6'-0"	7'-11"	STOREFRONT	GLASS		ENTRY	Yes	Yes	Yes	Yes	Yes	3/A9.8.2	EXIT	
103	D10	STAIR-01	1-14	EXTERIOR		3'-0"	8'-0"	METAL	GLASS		ENTRY	Yes	Yes	Yes	Yes	Yes	2/A9.8.1	EXIT	
104	D5	EXTERIOR		FIRE ALARM	1-15	3'-0"	8'-0"	METAL	FIBERGLASS		STOREROOM - STANDARD						2/A9.8.1	FIRE ALARM	
105	D10	SERVICE LOBBY-01	1-00	STAIR 02	1-01	3'-3"	8'-1"	STOREFRONT	GLASS	1.5 HRS	STAIRWELL - SECURITY	Yes	Yes	Yes	Yes	Yes	9/A9.8.3	TO LOBBY / STAIR 02	
106	D7	CORRIDOR-101	1-02	CLASSROOM 4	1-19	3'-0"	8'-0"	STOREFRONT	GLASS		CLASSROOM - SECURITY LOCK		Yes				13C / A9.8.1	CLASSROOM	
107	D5	CORRIDOR-101	1-02	WOMENS RESTROOM	1-03	3'-0"	8'-0"	METAL	SC WOOD		RESTROOM - MULTI						13B / A9.8.1	WOMEN	
108	D5	CORRIDOR-101	1-02	MENS RESTROOM	1-04	3'-0"	8'-0"	METAL	SC WOOD		RESTROOM - MULTI						13B / A9.8.1	MEN	
109	D6	CORRIDOR-101	1-02	IT CLOSET	1-05	3'-0"	8'-0"	METAL	SC WOOD		STOREROOM - STANDARD	Yes					13A / A9.8.1	IT CLOSET	VENTILATED DOOR
111	D7	CORRIDOR 103	1-12	CLASSROOM 1	1-16	3'-0"	8'-0"	STOREFRONT	GLASS		CLASSROOM - SECURITY LOCK		Yes				13C / A9.8.1	CLASSROOM	
113	GL8	ENTRY LOBBY	1-09	OFFICE	1-10	3'-0"	8'-0"	METAL	GLASS		OFFICE PRIVACY						13D / A9.8.1		
114	D7	CORRIDOR 103	1-12	TEACHERS OFFICE	1-11	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY						13D / A9.8.1		
116	D6	CORRIDOR 103	1-12	CLASSROOM STORAGE	1-13	3'-0"	8'-0"	METAL	SC WOOD		STOREROOM - STANDARD						13C / A9.8.1	STORAGE	
117	D10	CORRIDOR 103	1-12	STAIR-01	1-14	3'-3"	8'-1"	STOREFRONT	GLASS	1.5 HRS	STAIRWELL - SECURITY	Yes	Yes	Yes	Yes	Yes	9/A9.8.3	EXIT AND STAIR 01	SOFT CLOSE, LHR FROM LOCKABLE SIDE
118	D4	MENS RESTROOM	1-04	JANITOR	1-06	5'-0"	8'-0"	METAL	SC WOOD		STOREROOM - STANDARD						13B / A9.8.1	JANITOR	WITH LOUVER
119	GL8	CORRIDOR-101	1-02	ACCOUNTING	1-07	3'-0"	8'-0"	METAL	GLASS		OFFICE PRIVACY						13D / A9.8.1		
120	GL8	CORRIDOR-102	1-08	CLASSROOM 3	1-18	3'-0"	8'-0"	STOREFRONT	GLASS		CLASSROOM - SECURITY LOCK						13C / A9.8.1	CLASSROOM	
121	GL8	CORRIDOR-102	1-08	CLASSROOM 2	1-17	3'-0"	8'-0"	STOREFRONT	GLASS		CLASSROOM - SECURITY LOCK						13C / A9.8.1	CLASSROOM	
122	D8	STAIR 02	1-01	EXTERIOR		3'-0"	8'-0"	STOREFRONT	GLASS		EXIT ONLY	Yes	Yes	Yes	Yes	Yes	2 / A9.8.1	EXIT	SOFT CLOSE
123	D29	EXTERIOR		PG&E METER	1-20	4'-0"	8'-0"	STOREFRONT	GLASS		STOREROOM - STANDARD		Yes	Yes	Yes	Yes	2 / A9.8.1	PG&E METER	PG&E TO PROVIDE LOCK
SECOND FLOOR																			
201	D10	ELEVATOR LOBBY 02	2-01	STAIR-02	2-00	3'-3"	9'-1"	STOREFRONT	GLASS	1.5 HRS	STAIRWELL - SECURITY	Yes	Yes	Yes	Yes	Yes	9/A9.8.3	STAIR 02 TO EXIT	SOFT CLOSE, LHR FROM LOCKABLE SIDE
202	D6	ELEVATOR LOBBY 02	2-01	SUPPLIES	2-02	2'-6"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD						13A / A9.8.1	STORAGE	
203	D6	RESTROOM HALL 02	2-03	WELLNESS ROOM	2-04	3'-0"	7'-0"	METAL	SC WOOD		RESTROOM - PRIVATE						13B / A9.8.1	ACCESSIBLE GENDER NEUTRAL RESTROOM	OCCUPANCY INDICATOR
204	D6	RESTROOM HALL 02	2-03	ACCESSIBLE RESTROOM	2-05	3'-0"	7'-0"	METAL	SC WOOD		RESTROOM - PRIVATE						13C / A9.8.1	ACCESSIBLE GENDER NEUTRAL RESTROOM	OCCUPANCY INDICATOR
205	D5	RESTROOM HALL 02	2-03	IT CLOSET	2-19	3'-0"	7'-0"	METAL	SC WOOD		STOREROOM - STANDARD	Yes					13A / A9.8.1	IT CLOSET	VENTILATED DOOR
206	D6	RESTROOM HALL 02	2-03	RESTROOM	2-07	3'-0"	7'-0"	METAL	SC WOOD		RESTROOM - PRIVATE						13C / A9.8.1	GENDER NEUTRAL RESTROOM	OCCUPANCY INDICATOR
207	D6	RESTROOM HALL 02	2-03	ACCESSIBLE RESTROOM	2-06	3'-0"	7'-0"	METAL	SC WOOD		RESTROOM - PRIVATE						13C / A9.8.1	WELLNESS ROOM	OCCUPANCY INDICATOR
208	D21	CORRIDOR 201	2-20	BREAK ROOM	2-08	6'-0"	8'-0"	STOREFRONT	GLASS		PASSAGE						Yes	13B / A9.8.1	
209	GL8	CALL CENTER MAIN	2-18	OFFICE	2-09	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
210	SL8	CALL CENTER MAIN	2-18	OFFICE	2-10	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
211	SL8	CALL CENTER MAIN	2-18	OFFICE	2-11	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
212	SL8	CALL CENTER MAIN	2-18	OFFICE	2-12	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
213	D7	CALL CENTER MAIN	2-18	SHARED OFFICE	2-13	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
214	SL8	CALL CENTER MAIN	2-18	CHAT ROOM	2-14	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
215	SL8	CALL CENTER MAIN	2-18	OFFICE	2-15	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
216	D7	CALL CENTER MAIN	2-18	OFFICE	2-16	3'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY
217	D10	STAIR-01	2-17	CALL CENTER MAIN	2-18	3'-3"	8'-8"	STOREFRONT	GLASS	1.5 HRS	STAIRWELL - SECURITY	Yes	Yes	Yes	Yes	Yes	9/A9.8.3	STAIR 01 TO EXIT	
218	SL2	SHARED OFFICE	2-21	CALL CENTER MAIN	2-18	6'-0"	8'-0"	STOREFRONT	GLASS		OFFICE PRIVACY								N.I.C. FURNITURE ASSEMBLY

DOOR SCHEDULE NOTES

- ALL RESTROOM AND IT CLOSET DOORS SHALL HAVE MINIMUM 1" UNDERCUTS FOR AIR CIRCULATION.
- WHERE KICK PLATE NOTED, PROVIDE ON "PUSH" SIDE OF DOOR ONLY. (TRIMCO K0050, U.O.N.) 10", .050" ALUMINUM.
- ACCESS CONTROLLED DOORS SHALL PROVIDE ELECTRONIC CONTROLLED ACCESS/ DOOR HARDWARE.
- FOR GLAZING TYPE DEFINITIONS, SEE SPECIFICATION BOOK, SECTION 08 80 00
- DOORS MARKED AS "N.I.C. FURNITURE ASSEMBLY" ARE PART OF THE MOVEABLE WALL FURNITURE ASSEMBLY PROVIDED BY OWNER.

**DOOR CLOSER (TYP. U.O.N.):**  
NORTON, PR7500DA, BARRIER FREE, 90 DEGREE OPENING WITH DELAYED CLOSING, PAINTED ALUMINUM

**LOCK GUARD (TYP. U.O.N.):**  
CYLINDRICAL LOCK GUARD, DON-JO #BLP-107-630, STAINLESS STEEL

**DOOR HINGES (TYP. U.O.N.):**  
HAGER, BB1199 STAINLESS 4 1/2 X 4 1/2, HEAVY WEIGHT, HIGH FREQUENCY, FIVE KNUCKLE, FOUR BEARING, NRP, FULL-MORTISE BUTTS, 1 1/2 PAIR

**SELF CLOSING HINGE (WHERE OCCURS):**  
LOCINOX, MAMMOTH 180, (1 PAIR TYP.)

**UNDER DOOR SWEEP (TYP. U.O.N.):**  
PEMCO 217AV

**WEATHER STRIPPING (TYP. U.O.N.):**  
PEMCO 319CR

**DOOR STOP (TYP. U.O.N.):**  
INTERIOR: 1. TRIMCO #1270 CVSV  
2. TRIMCO #1260 W

EXTERIOR: TRIMCO #1209 HA

**DOOR THRESHOLD (TYP. U.O.N.):**  
1. PEMKO, 158A (1/2" OFFSET)  
2. PEMKO, 272A (SADDLE, 6" X 1/4")



ARCHITECHNICA

555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net


www.architechnica.net



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CONSULTANT



CODESTACK ACADEMY

201 N CALIFORNIA ST.  
STOCKTON, CA 95202

SAN JOAQUIN COUNTY  
OFFICE OF EDUCATION

REVISIONS	
A	ADDENDA #1 11.05.24

PROJECT NO: 2023-04  
ISSUE SET: BID SET  
ISSUE DATE: 12.04.24  
DRAWN BY: LCG

DOOR SCHEDULE



DOOR SCHEDULE

DOOR TAG MARK	DOOR TYPE MARK	FROM ROOM: NAME	FROM ROOM: NUMBER	TO ROOM: NAME	TO ROOM: NUMBER	WIDTH	HEIGHT	FRAME MATERIAL	DOOR MATERIAL	FIRE RATING ASSEMBLY	HARDWARE SET TYPE	ACCESS CONTROL	PANIC HARDWARE	WEATHER STRIP	DOOR CLOSER	KICKPLATE	THRESHOLD	SIGNAGE	COMMENTS
SIDEWALK																			
401	G1	TRASH ENCLOSURE	T-01	EXTERIOR		3' - 0"	7' - 0"	ORNAMENTAL IRON			PADLOCK						4/A9.1.3	TRASH	TRASH ENCLOSURE MAN GATE
402	G2	TRASH ENCLOSURE	T-01	EXTERIOR		15' - 4"	6' - 8"	METAL	METAL		PADLOCK						18/A9.1.2		TRASH ENCLOSURE
403	G3	PARKING LOT		CALIFORNIA STREET		3' - 0"	7' - 0"	ORNAMENTAL IRON	WROUGHT IRON		ACCESS CONTROL	Yes	Yes	Yes	Yes		10/A9.1.3		PEDESTRIAN GATE
404	G3	PARKING LOT		CHANNEL STREET		3' - 6"	7' - 0"	ORNAMENTAL IRON			ACCESS CONTROL	Yes	Yes	Yes	Yes		10/A9.1.3		PEDESTRIAN GATE
405	G4	PARKING LOT		CALIFORNIA STREET		26' - 0"	7' - 0"	ORNAMENTAL IRON			AUTO INFRARED	Yes					3/A9.1.3		(2) PANEL SLIDE
406	G4	PARKING LOT		CHANNEL STREET		26' - 0"	7' - 0"	ORNAMENTAL IRON			AUTO INFRARED	Yes					3/A9.1.3		(2) PANEL SLIDE
407	G5	PARKING LOT	T-05	SUTTER STREET		4' - 0"	7' - 0"	ORNAMENTAL IRON	WROUGHT IRON		PADLOCK						12/A9.1.2	FIRE DEPARTMENT USE ONLY	FIREMAN ACCESS
THIRD FLOOR																			
301	D10	ELEVATOR LOBBY 3	3-01	STAIR 02	3-00	3' - 3"	7' - 8"	STOREFRONT	GLASS	1.5 HRS	STAIRWELL - SECURITY	Yes	Yes	Yes	Yes	Yes	9/A9.8.3	STAIR 02 TO EXIT	SOFT CLOSE, LHR FROM LOCKABLE SIDE
302	D6	ELEVATOR LOBBY 3	3-01	ELEVATOR ROOM	3-02	4' - 0"	7' - 0"	METAL	SC WOOD		STOREROOM - STANDARD			Yes	Yes	Yes	4/A9.8.1	ELEVATOR EQUIPMENT	CONTROL ROOM DOOR TO BE SELF-CLOSING, SELF-LOCKING, REQUIRE A KEY TO OPEN FROM OUTSIDE. ROOM TO BE TEMPERATURE CONTROLLED
303	D6	RESTROOM HALL 03	3-03	ACCESSIBLE RESTROOM	3-05	3' - 0"	7' - 0"	METAL	SC WOOD		RESTROOM - PRIVATE					Yes	13C / A9.8.1	ACCESSIBLE GENDER NEUTRAL RESTROOM	OCCUPANCY INDICATOR
303.1	D6	ACCESSIBLE RESTROOM	3-05	SUPPLY	3-04	2' - 6"	7' - 0"	METAL	SC WOOD		STOREROOM - STANDARD						13B / A9.8.1	STORAGE	
304	D6	RESTROOM HALL 03	3-03	ACCESSIBLE RESTROOM	3-06	3' - 0"	7' - 0"	METAL	SC WOOD		RESTROOM - PRIVATE					Yes	13C / A9.8.1	ACCESSIBLE GENDER NEUTRAL RESTROOM	
305	D5	RESTROOM HALL 03	3-03	IT CLOSET	3-07	3' - 0"	7' - 0"	METAL	SC WOOD		STOREROOM - STANDARD	Yes					13A / A9.8.1	IT CLOSET	VENTILATED DOOR
306	D5	RESTROOM HALL 03	3-03	JANITOR	3-09	3' - 0"	7' - 0"	METAL	SC WOOD		STOREROOM - STANDARD					Yes	13C / A9.8.1	JANITOR	
307	D6	RESTROOM HALL 03	3-03	RESTROOM	3-08	3' - 0"	7' - 0"	METAL	SC WOOD		RESTROOM - PRIVATE				Yes	13C / A9.8.1	GENDER NEUTRAL RESTROOM	OCCUPANCY INDICATOR	
308	D21	KITCHENETTE	3-16	LARGE CONFERENCE	3-10	6' - 0"	8' - 0"	STOREFRONT	GLASS		OFFICE PRIVACY						13D / A9.8.1	CONFERENCE ROOM	STC 40 PREFERRED
309	SL2	CODE SPACE	3-15	JOHNNY	3-11	6' - 0"	8' - 0"	STOREFRONT	GLASS		OFFICE PRIVACY						13D / A9.8.1		N.I.C. FURNITURE ASSEMBLY
310	SL8	CODE SPACE	3-15	SHARED OFFICE	3-12	3' - 0"	8' - 0"	STOREFRONT	GLASS		OFFICE PRIVACY						13D / A9.8.1		N.I.C. FURNITURE ASSEMBLY
311	D10	BREAK OUT AREA	3-13	STAIR 01	3-14	3' - 3"	8' - 2"	STOREFRONT	GLASS	1.5 HRS	STAIRWELL - SECURITY	Yes	Yes	Yes	Yes	Yes	9/A9.8.3	STAIR 01 TO EXIT	

DOOR SCHEDULE NOTES

- ALL RESTROOM AND IT CLOSET DOORS SHALL HAVE MINIMUM 1" UNDERCUTS FOR AIR CIRCULATION.
- WHERE KICK PLATE NOTED, PROVIDE ON "PUSH" SIDE OF DOOR ONLY. (TRIMCO K0050, U.O.N.) 10", .050" ALUMINUM.
- ACCESS CONTROLLED DOORS SHALL PROVIDE ELECTRONIC CONTROLLED ACCESS/ DOOR HARDWARE.
- FOR GLAZING TYPE DEFINITIONS, SEE SPECIFICATION BOOK, SECTION 08 80 00
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NORTON, PR7500DA, BARRIER FREE, 90 DEGREE OPENING WITH DELAYED CLOSING, PAINTED ALUMINUM

**LOCK GUARD (TYP. U.O.N.):**  
CYLINDRICAL LOCK GUARD, DON-JO #BLP-107-630, STAINLESS STEEL

**DOOR HINGES (TYP. U.O.N.):**  
HAGER, BB1199 STAINLESS 4 1/2 X 4 1/2, HEAVY WEIGHT, HIGH FREQUENCY, FIVE KNUCKLE, FOUR BEARING, NRP, FULL-MORTISE BUTTS, 1 1/2 PAIR

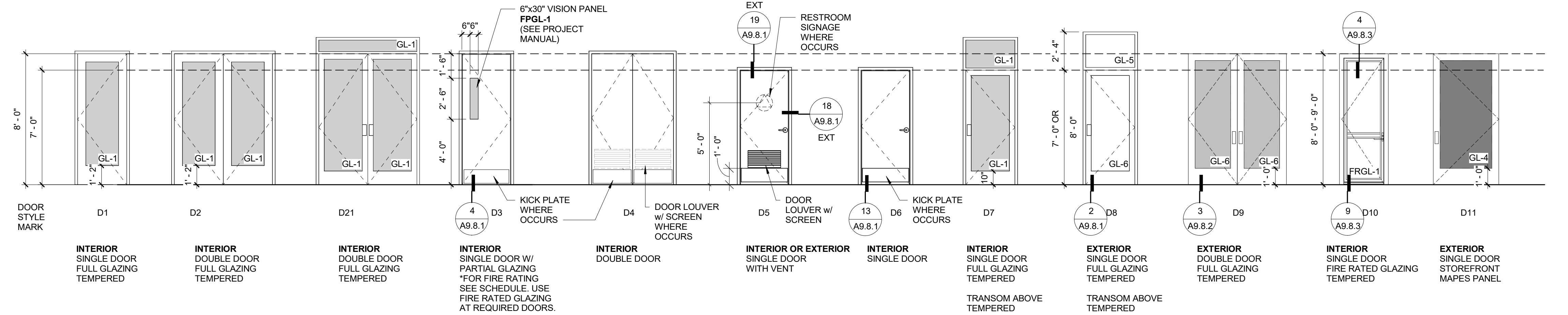
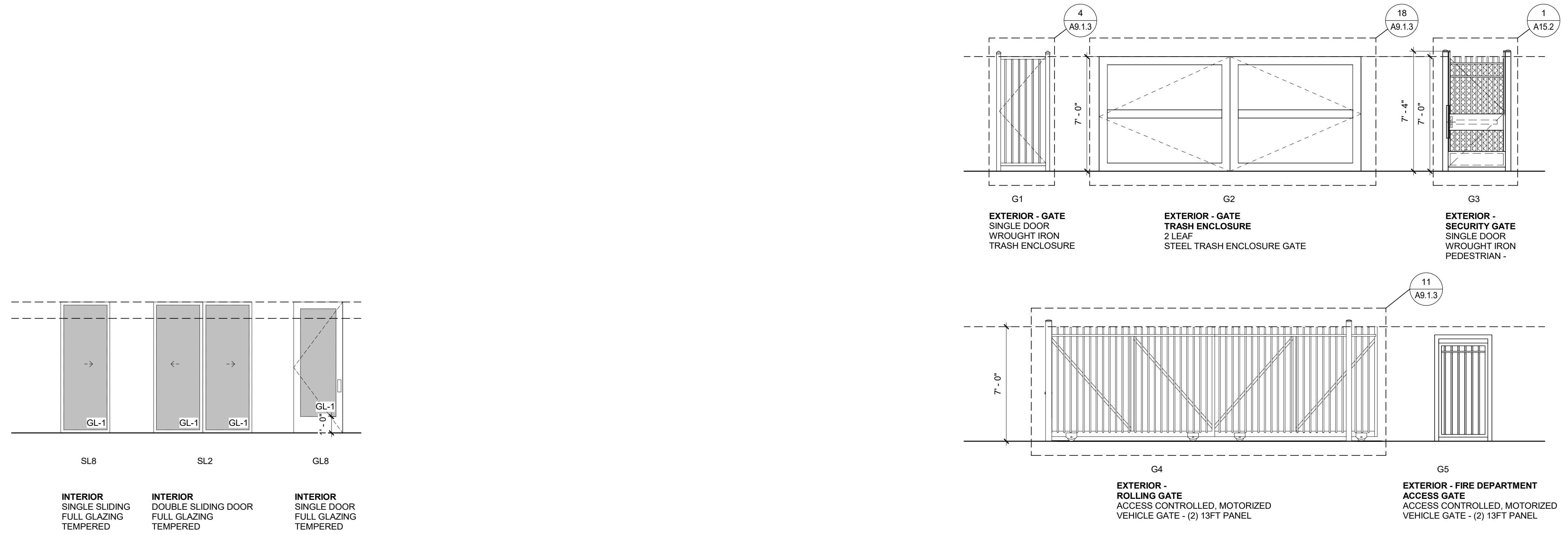
**SELF CLOSING HINGE (WHERE OCCURS):**  
LOCINOX; MAMMOTH 180, (1 PAIR TYP.)

**UNDER DOOR SWEEP (TYP. U.O.N.):**  
PEMCO Z17AV

**WEATHER STRIPPING (TYP. U.O.N.):**  
PEMCO 319CR

**DOOR STOP (TYP. U.O.N.):**  
INTERIOR: 1. TRIMCO #1270 CVSV  
2. TRIMCO #1260 W  
EXTERIOR: TRIMCO #1209 HA

**DOOR THRESHOLD (TYP. U.O.N.):**  
1. PEMCO, 158A (1/2" OFFSET)  
2. PEMCO, 272A (SADDLE, 6" X 1/4")



**DOOR TYPE LEGEND** REFER TO DOOR SCHEDULE FOR DOOR SIZE, AND ADDITIONAL INFORMATION.  
1/4" = 1'-0"

**ARCHITECHNICA**  
555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
www.architechnica.net

**CONSULTANT**

**CODESTACK ACADEMY**  
201 N CALIFORNIA ST.  
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REVISIONS  
A ADDENDA #1 11.05.24

PROJECT NO: 2023-04  
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ISSUE DATE: 12.04.24  
DRAWN BY: LCG

**DOOR SCHEDULE & TYPE LEGEND**

**A8.2**

PRE-BID CLARIFICATION FORM (For Contractor's Use)

RFI #06

PROJECT NAME:	CodeStack Academy – 201 N. California Street		
PROJECT NUMBER:	010-233-233000		
TO:	Tim Dearborn & Tim Sutton	EMAIL:	tim@architechnica.net & tisutton@sjcoe.net

DATE:			
FROM:	F&H Construction Brian Roek	EMAIL:	estimating@f-hconstruction.com
DOCUMENT/DIVISION NUMBER:	Folding Wall System	DRAWING NUMBER:	A2.2 & A7.1.7

REQUESTED CLARIFICATION:

Please provide a specification for the folding wall system

RESPONSE TO CLARIFICATION:

**NEW SPECIFICATION SECTION 10 22 39 FOR SKYFOLD WALL PROVIDED.**

**TIM DEARBORN, ARCHITECHNICA 12-06-24**

Attach additional numbered sheets as necessary; however, only one (1) request shall be contained on each submitted form.

**SECTION 10 22 39 – AUTOMATIC VERTICAL FOLDING PANEL PARTITIONS**

**PART 1 – GENERAL**

**1.1 Work Included**

- A. Supply and installation of Automatic Vertically Retractable Acoustical Wall(s) as shown on the architectural drawings. All necessary hardware, seals, lifting machinery, electrical controls are included.

**1.2 Related Work**

- A. The main support steel beam for the wall, as well as the miscellaneous support steel for the lifting machinery for the Automatic Vertically Retractable Acoustical Wall – Section 05 12 00 – Structural Steel Framing.
- B. Ceiling storage pockets along axis of Automatic Vertically Retractable Acoustical Wall – Sections 05 40 00 – Cold-formed Metal Framing and 09 29 00 Gypsum Board.
- C. Bulkheads and sound insulation above, below, and in the fixed walls at both ends of the Automatic Vertically Retractable Acoustical Wall, as per ASTM E557 - Section 07 21 00 – Thermal Insulation.
- D. All site wiring and connections for main power, including disconnect switches at each motor location. All site wiring and connections for control, including installation of key switches – Division 26 – Electrical.

**1.3 System Description**

A. Definition

- 1. Automatic Vertically Retractable Acoustical Wall (from here on called Operable Wall) shall refer specifically to acoustical partitions that, when in the down position (closed) are hard, rigid, flat, plumb walls, made of a grid of rectangular acoustical panels, and when are lifted (opened), fold upward (vertically) without the use of any manual labor, in a manner similar to an accordion, into a pocket in the ceiling, between roof joists, or up between built in bulkheads. In the down (closed) position, the operable wall shall be comprised of two vertical planes of acoustical panels, separated by an acoustical air space.

The operable wall shall open and close in a manner similar to an accordion, in that all wall panels fold and unfold sequentially in an accordion fashion.

Standard Drive System:

The motor drive assembly is mounted directly above the centre line of the operable wall. Support steel is only required at one location.

Minimum wall length without modifying our system is 9'-0". Used for walls up to 12'-0" finished ceiling heights:

- 2. The operable wall shall be opened and closed using two touch screen operator stations. Pressing and holding the up or down directional arrow symbol on one touch screen while simultaneously pressing and holding the button symbol on the second screen shall cause the wall to move in the selected direction. When hand pressure is removed, the wall shall immediately stop. The operable wall shall stop in a quick and positive fashion without coasting. As a normal part of the operation, it shall be possible to partially open (or close) the wall, stop it and then reverse the operation. There shall be two (2) switches per operable wall, located on opposite sides of the wall at opposite ends of the wall, wired in series. The screens will display faults in case of a failure with the electrical system.

3. From a fully open position, the operable wall shall be able to go through its entire cycle of closing and/or opening without any manual intervention.
4. When the operable wall is being lowered (closed) it shall come automatically to rest once it has reached the fully down (closed) position.
5. When the operable wall is being lifted (opened) it shall come automatically to rest once it has reached the fully up (open) position.
6. The operable wall shall automatically and acoustically seal against the floor without the need for any manual intervention. The floor seals shall leave a joint between the floor and the bottom acoustical panels of not more than approximately 2".
7. The operable wall shall automatically and acoustically seal against the two end walls without the need for any manual intervention. The end seals shall act in such a way as not to come into contact with the end walls while the operable wall is in motion. The end seals shall leave a joint between the acoustical panels and the end walls of no more than approximately 1". Seals that rub or brush against the end walls are not acceptable. Once the operable wall reaches the full down position, the end seals shall activate automatically. The touch screen button and directional symbol do not need to be held during the deployment of the end seals.
8. The operable wall shall automatically and acoustically seal against the ceiling without any manual intervention. The top seals shall leave a joint between the top acoustical panels and the ceiling of the pocket of not more than approximately 2".
9. The operable wall shall open and close at an average speed of approximately 5 to 10 vertical feet per minute.
10. When the operable wall is being lowered (closed), it shall stop if the leading (bottom) edge comes into firm contact with any object between it and the floor. The operable wall will then automatically reverse its direction and ascend for approximately 3 seconds to clear the object. The regular operation of the operable wall can resume once the obstruction has been removed.
11. The operable wall shall be visibly flat and rigid in the down (closed) position.
12. There shall be no exposed hinges, brackets, screws, and no part of the mechanical system shall be visible when the operable wall is in the down (closed) position.
13. All of the panel edges shall be right angled, with a minimum radius not more than 1/16".
14. All of the panels shall be rectangular, nominally of the same size, unless requested otherwise by the architect.
15. Joints between panel, vertical and horizontal, shall be no more than approximately 1/2" wide.
16. The operable wall shall stack in the up (open) position into a space no greater than 71 1/4" (1810mm) wide. The operable wall shall have a stacking height ratio in the range of 1:5 to 1:10, depending on the height of the wall.
17. Each acoustical panel shall be individually removable using only a screw driver. No special tools or equipment shall be required. The removal of a single acoustical panel shall not affect, dislocate or cause the removal of any adjacent panels or other acoustical panels.

18. The operable wall shall not weigh more than the following:
  - a. Skyfold Zenith® Premium 51 ~6.2 lbs per ft<sup>2</sup>
  - b. The preceding weights do not include the motor drive or the architectural finish on the acoustical panels and are based on 24'-0" long x 12'-0" high operable wall.
19. A completely functioning operable wall, tested in full accordance and compliance with ASTM E90 (ISO 140-3), shall achieve, from an independent laboratory, a Sound Transmission Class (STC) rating (Rw value) of not less than the following:
  - a. Skyfold Zenith® Premium 51
    - 1) System STC 51 (Rw 51), Panel Construction STC 61(Rw 60)
20. The operable wall shall be designed to have a design life of at least 10,000 complete closed to opened to closed cycles.

#### **1.4. Quality Assurance**

- A. The products herein specified established the standard of quality for the operable wall based on the following Skyfold Zenith® Premium Automatic Vertically Retractable Acoustic Walls by Skyfold Inc. of Baie d'Urfe (Montréal), Québec, Canada:

Skyfold Zenith® Premium 51: System STC 51 (Rw 51), Panel Construction STC 61 (Rw 60)
- B. All work and materials specified herein, shall be installed only by qualified representatives and/or installers and/or distributors of the manufacturer, according to the manufacturers written instructions.
- C. The operable wall must be manufactured by a certified ISO-9001-2008 company or an equivalent quality control system.

#### **1.5 References**

- A. ASTM E90, Method for Laboratory Measurement of Airborne Sound Transmission Loss of Building Partitions.
  1. Annex A1.15 Operable (Folding or Sliding Walls)
    - a. Annex A1.15.3 Operation – “The specimen shall not be designated an operable wall unless it opens and closes in a normal manner. It shall be fully opened and closed at least five times after installation is completed and tested without further adjustments.”
- B. ASTM E413, Classification for Rating Sound Insulation.
- C. ASTM E557, Standard Practice for Architectural Application and Insulation of Operable Partitions.
- D. ISO 354, Measurement of Sound Absorption
- E. ISO 140-3, Measurement of Airborne Sound Insulation
- F. ASTM C423, Measurement of Sound Absorption

#### **1.6 Submittals**

- A. Submit manufacturers' technical data for each type of operable wall specified herein.



- B. Submit shop drawings showing complete layout of operable wall system based on field verified dimensions. The drawings shall include dimensional relationship to adjoining work. Include details indicating materials, finishes, tolerances, and methods of attachment to building steel and electrical requirements.
- C. Submit certified test reports evidencing compliance to acoustical STC (Rw) requirements as specified in paragraph 1.3.A.20 and in accordance to references listed in paragraphs 1.5.A and 1.5.E.

### **1.7 Site Conditions**

- A. The floor underneath the operable wall along its axis, shall be flat to within +/- 1/4" over the entire length of an operable wall. The peak to valley undulation of +/- 1/4" shall not be closer together than 24" and a peak to valley undulation of +/- 1/8" shall not be closer than 12".
- B. Support steel above the operable wall along its axis shall be parallel to the floor within +/- 1/2" for the entire length of the operable wall. This includes loaded deflection. The beam must also be parallel to the centre line of the wall within  $\pm 1/8"$ , left to right.
- C. The fixed walls at either end of the operable wall shall be within +1/4" - 0", from plumb vertical.
- D. The fixed walls at either end of the operable wall shall be flat to within +0", -1/4".

### **1.8 Warranty**

- A. Basic Warranty: The operable wall shall be warranted free from defects in material and workmanship for a period of two (2) years or five thousand (5,000) cycles, whichever occurs first, from the date of shipment.
- B. Extended Parts Warranty: An extended warranty on parts (excluding touch screen operator stations) shall be provided. It includes coverage on all parts for a period of ten (10) years or five thousand (5,000) cycles, whichever occurs first from date of shipment. Refer to Owner's manual for full warranty details.
- C. Acoustical Performance: The operable wall shall retain its acoustical properties for 10 years from the date of shipment providing proper maintenance has been performed on the partition.
- D. Parts and labor required to maintain the operable wall and part subject to normal wear and tear are not covered under the warranty and are the owner's responsibility. (Refer to Maintenance Program).

## **PART 2 – PRODUCTS**

### **2.1. Basis of Design Manufacturer**

- A. Skyfold Zenith® Premium Automatic Vertically Retractable Acoustic Walls as manufactured by Skyfold Inc. of Baie d'Urfé (Montréal), Québec, Canada (514) 457-4767.  
E-mail: [skyfold@skyfold.com](mailto:skyfold@skyfold.com)  
Web-site: [www.skyfold.com](http://www.skyfold.com)  
Skyfold Zenith® Premium 51 System STC 51 (Rw 51), Panel Construction STC 61 (Rw 60)
- B. Substitution of products or techniques not conforming to these specifications shall be per Section 01 60 00 – Product Requirements. Independent test reports which meet the requirements and design specified herein must be submitted to obtain approval.

### **2.2 Materials**

- A. Acoustical Panels

1. Acoustical panels shall be faced with steel with top and bottom rows + pocket in standard HPL from Pionte (Little Black Dress) with black edges. Center rows in Lampre laminate white marking steel.
2. Acoustical panels, together with all of the sound insulation, shall be, as much as possible, made of non-combustible or fire-treated materials.
3. Acoustical panels shall be fabricated to be as stiff as possible in order to satisfy the rigid criteria when the operable wall is down (closed) and to ensure that there is no interference between panels when the wall is in motion.
4. Acoustical panels shall be architecturally flat with no bowing, oil canning, warping, waviness or any other surface deformation and discontinuity.
5. Acoustical panels shall have the finish of the architect's choice, provided that the finish has been approved by the operable wall manufacturer to ensure compatibility with the wall panels. The following criteria must be met:
  - a. Maximum weight of material: 0.111 lbs/ft<sup>2</sup>
  - b. Maximum thickness of material: 1/8"
  - c. No brittle materials.
  - d. Finishes are railroaded onto the panels, applied horizontally along the panel length. Pricing will vary depending on finish selection.
  - e. Only acoustically transparent fabric finish for Skyfold Zenith® Premium NRC
6. Acoustical panels shall meet the following STC ratings in accordance with ASTM E90 (ISO 140-3) specification as reported by an independent laboratory.

Skyfold Product	Panel Construction	Fully Automatic Operable wall
Skyfold Zenith® Premium 51	61 STC (60 Rw)	51 STC (51 Rw)

**B. Folding Mechanism**

1. The hanging, folding and extension mechanism shall be, as much as possible, made from structural grade aluminum extrusions and structural shapes, in order to minimize the weight of the system.
2. All wear surfaces, such as bushings, spacers, pins, discs, bearings, and sleeves shall be designed to function quietly and with minimum wear, over the 10,000 cycle design life of the operable wall.
3. The hangers, which fasten the lifting mechanism to the support steel, shall be fabricated from steel and shall be welded or bolted to the support steel supplied by others.

**C. Motor Drive**

1. The motor drive shall be sized properly so that it can open and close the operable wall effectively over the 10,000 cycle design life of the wall, at the minimum design speed specified in paragraph 1.3.A.9.
2. The folding mechanism shall be designed to function as smoothly, quietly and safely as possible. Wherever possible, ball bearings shall be used instead of bushings and wear surfaces. In no circumstance shall chain or belt drive systems be acceptable.

3. There shall be a wire rope cable for every set of folding mechanism. This cable shall be of 6 x 31 construction aircraft cable and shall be made of galvanized steel. The diameter of the cables shall be sized so that they shall be able to hold the entire weight of the operable wall, with the appropriate safety factor.
  4. The cable wraps on yoyo drums with 2 safety wraps and multiple layers of cable.
  5. The line shaft, sized to deliver the required torque with minimum deflection, shall support and rotate the cable drums.
  6. Flange bearings shall be used for the drive system, located immediately on both sides of the drum assembly.
  7. The motor drive shall be sized to deliver sufficient amount of torque to safely and effectively raise and lower the operable wall over its design life.
  8. The motor drive shall use the latest in industry standards in thermal protection, overload protection, quick acting fuses, etc., in order to ensure the safety and reliability of the system.
- D. Safety Equipment
1. The operable wall shall employ an electromagnetic type of brake which shall activate firmly, without hesitation, when power is lost to the system. This brake shall have a minimum retarding torque rating equal to 200% of the motor drive's full load torque. The drive system shall be equipped with a manual override and a brake release lever.
  2. The operable wall shall employ a dynamic brake, distinct and separate from the brake in 2.2.D.1, in order to lower the operable wall at a controlled speed of no more than approximately 150% of the normal down speed, in the case of a catastrophic failure in the motor drive's power train. Alternately, the operable wall shall employ a brake, distinct and separate from the brake in 2.2.D.1, in order to completely halt the downward motion of the wall in the case of a catastrophic failure in the power train.
  3. The operable wall shall employ electrical or other limit switches in order to stop the wall at it's up and down travel limits.
  4. The operable wall shall employ an over torque detector in order to sense a jam in the system and to act as an over travel limit in the up direction should the primary limit switch fail to act. This over torque sensor shall be mechanical, using the motor's torque arm in it's over torque detection
  5. The entire length of the bottom edge of the operable wall shall be equipped with a continuous pressure sensing strip which shall cut power to the motor drive and shall activate the brake outlined in 2.2.D.1, if the sensing edge comes in firm contact with an object, before the operable wall is in the full down (closed) position. The operable wall will automatically reverse direction and ascend for approximately 3 seconds to clear the obstruction. The power shall remain cut to the motor drive until the switches have been released. The operation of the operable wall can resume once the obstruction is removed.
  6. Supply and install manual brake release for maintenance and incase or failure.
- E. Electrical
1. The operable wall shall be equipped for a three-phase power supply to the electrical control box.
  2. Standard electrical control box will be NEMA 1.
  3. Low voltage wiring (by others). 18-gauge wiring from the switches to the control box.

4. Touch Screen Operator Stations: Two (2), 4.3” resistive LCD touch screens, wired in series with multilingual capabilities and 4-digit adjustable user pin. The screens will display faults in case of a failure with the electrical system. (wiring by others)

### **2.3. Fabrication**

- A. Factory assemble all components, assemblies and systems into the largest possible assemblies in order to minimize the amount of assembly on site.

## **PART 3 – EXECUTION**

### **3.1 Inspection**

- A. Inspect the relevant aspects of the site such as the evenness of the floor, walls, structural steel, etc., and ensure that these are within the tolerances stated in Part – 1 of this specification.
- B. Confirm in writing to the General Contractor or contract manager any deviations from these tolerances. Do not proceed until these conditions are made good.
- C. Carry out all appropriate field measurements before manufacturing any components or assemblies.

### **3.2 Installation**

- A. Install operable walls in accordance with the manufacturer’s printed instructions.
- B. The operable wall supplier shall not deliver or install this product until the General Contractor can ensure in writing safe storage and protection for the operable wall for the duration of the project.

### **3.3 Adjusting and Cleaning**

- A. Adjust and fine-tune the operable walls to ensure that all seals are operating and sealing properly and that the operable walls are in correct and smooth operation.
- B. Clean up any dirt, oil, grime, etc., that may have found its way onto the acoustical panels. Leave the operable wall in a state of architectural cleanliness.

### **3.4. Closeout Activities**

- A. Demonstrate operation of partition and identify potential operational problems.
- B. Train maintenance staff on troubleshooting and servicing the partitions.

## **END OF SECTION**

**SECTION 07 21 00 - THERMAL INSULATION**

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 01 Specification Sections, apply to this Section.

1.2 SUMMARY

A. Section Includes:

1. Glass-fiber blanket insulation.
2. Mineral-wool blanket insulation.
3. Loose-fill insulation.

B. Related Sections:

1. Section 03 52 16 "Lightweight Insulating Concrete Systems (LWIC)" for insulation specified as part of roofing construction.
2. Section 07 84 46 "Fire-Resistive Joint Systems" for insulation installed as part of a perimeter fire-resistive joint system.

1.3 SUBMITTALS

- A. Product Data: For each type of product indicated.
- B. Product Test Reports: Based on evaluation of comprehensive tests performed by a qualified testing agency, for each product.
- C. Research/Evaluation Reports: For foam-plastic insulation, from ICC-ES and listed in TER 1303-04 – Attachment of Exterior Wall Coverings Through Foam Plastic Insulating Sheathing (FBIS) to Wood or Steel Wall Framing.

1.4 QUALITY ASSURANCE

- A. Surface-Burning Characteristics: As determined by testing identical products according to ASTM E 84 by a qualified testing agency. Identify products with appropriate markings of applicable testing agency.

1.5 DELIVERY, STORAGE, AND HANDLING

- A. Protect insulation materials from physical damage and from deterioration due to moisture, soiling, and other sources. Store inside and in a dry location. Comply with manufacturer's written instructions for handling, storing, and protecting during installation.
- B. Protect foam-plastic board insulation as follows:
  1. Do not expose to sunlight except as necessary for period of installation and concealment.
  2. Protect against ignition at all times. Do not deliver foam-plastic board materials to Project site before installation time.
  3. Quickly complete installation and concealment of foam-plastic board insulation in each area of construction.

PART 2 - PRODUCTS

2.1 GLASS-FIBER BLANKET INSULATION

- A. Manufacturers: Subject to compliance with requirements, available manufacturers offering products that may be incorporated into the Work include, but are not limited to, the following:
1. CertainTeed Corporation.
  2. Guardian Building Products, Inc.
  3. Johns Manville.
  4. Knauf Insulation.
  5. Owens Corning.
- B. Unfaced, Glass-Fiber Blanket Insulation: ASTM C 665, Type I; with maximum flame-spread and smoke-developed indexes of 25 and 50, respectively, per ASTM E 84; passing ASTM E 136 for combustion characteristics.

2.2 MINERAL-WOOL BLANKET INSULATION

- A. Manufacturers: Subject to compliance with requirements, available manufacturers offering products that may be incorporated into the Work include, but are not limited to, the following:
1. Fibrex Insulations Inc.
  2. Owens Corning.
  3. Roxul Inc.
  4. Thermafiber.
- B. Unfaced, Mineral-Wool Blanket Insulation: ASTM C 665, Type I (blankets without membrane facing); consisting of fibers; with maximum flame-spread and smoke-developed indexes of 25 and 50, respectively, per ASTM E 84; passing ASTM E 136 for combustion characteristics.

2.3 LOOSE-FILL INSULATION

- A. Glass-Fiber Loose-Fill Insulation: ASTM C 764, Type I for pneumatic application or Type II for poured application; with maximum flame-spread and smoke-developed indexes of 5, per ASTM E 84.

2.4 INSULATION FASTENERS

- A. Adhesively Attached, Spindle-Type Anchors: Plate welded to projecting spindle; capable of holding insulation of specified thickness securely in position indicated with self-locking washer in place.
1. Products: Subject to compliance with requirements, provide one of the following available products that may be incorporated, but are not limited to, into the Work:
    - a. AGM Industries, Inc.; Series T TACTOO Insul-Hangers.
    - b. Gemco; Spindle Type.
  2. Plate: Perforated, galvanized carbon-steel sheet, 0.030 inch thick by 2 inches square.
  3. Spindle: Copper-coated, low-carbon steel; fully annealed; 0.105 inch in diameter; length to suit depth of insulation indicated.

- B. Insulation-Retaining Washers: Self-locking washers formed from 0.016-inch- thick galvanized-steel sheet, with beveled edge for increased stiffness, sized as required to hold insulation securely in place, but not less than 1-1/2 inches square or in diameter.
  - 1. Products: Subject to compliance with requirements, provide one of the following available products that may be incorporated, but are not limited to, into the Work:
    - a. AGM Industries, Inc.; RC150.
    - b. Gemco; R-150.
  - 2. Protect ends with capped self-locking washers incorporating a spring steel insert to ensure permanent retention of cap in the following locations:
    - a. Ceiling plenums.
    - b. Attic spaces.
- C. Anchor Adhesive: Product with demonstrated capability to bond insulation anchors securely to substrates indicated without damaging insulation, fasteners, and substrates.
  - 1. Products: Subject to compliance with requirements, provide one of the following available products that may be incorporated, but are not limited to, into the Work:
    - a. AGM Industries, Inc.; TACTOO Adhesive.
    - b. Gemco; Tuff Bond Hanger Adhesive.

### PART 3 - EXECUTION

#### 3.1 PREPARATION

- A. Clean substrates of substances that are harmful to insulation or that interfere with insulation attachment.

#### 3.2 INSTALLATION, GENERAL

- A. Comply with insulation manufacturer's written instructions applicable to products and applications indicated.
- B. Install insulation that is undamaged, dry, and unsoiled and that has not been left exposed to ice, rain, or snow at any time.
- C. Extend insulation to envelop entire area to be insulated. Cut and fit tightly around obstructions and fill voids with insulation. Remove projections that interfere with placement.
- D. Provide sizes to fit applications indicated and selected from manufacturer's standard thicknesses, widths, and lengths. Apply single layer of insulation units to produce thickness indicated unless multiple layers are otherwise shown or required to make up total thickness.

#### 3.3 INSTALLATION OF INSULATION FOR FRAMED CONSTRUCTION

- A. Apply insulation units to substrates by method indicated, complying with manufacturer's written instructions. If no specific method is indicated, bond units to substrate with adhesive or use mechanical anchorage to provide permanent placement and support of units.
- B. Glass-Fiber or Mineral-Wool Blanket Insulation: Install in cavities formed by framing members according to the following requirements:

1. Use insulation widths and lengths that fill the cavities formed by framing members. If more than one length is required to fill the cavities, provide lengths that will produce a snug fit between ends.
  2. Place insulation in cavities formed by framing members to produce a friction fit between edges of insulation and adjoining framing members.
  3. Maintain 3-inch clearance of insulation around recessed lighting fixtures not rated for or protected from contact with insulation.
  4. For metal-framed wall cavities where cavity heights exceed 96 inches, support unfaced blankets mechanically and support faced blankets by taping flanges of insulation to flanges of metal studs.
- C. Loose-Fill Insulation: Apply according to ASTM C 1015 and manufacturer's written instructions. Level horizontal applications to uniform thickness as indicated, lightly settle to uniform density, but do not compact excessively.
- D. Miscellaneous Voids: Install insulation in miscellaneous voids and cavity spaces where required to prevent gaps in insulation using the following materials:
1. Loose-Fill Insulation: Compact to approximately 40 percent of normal maximum volume equaling a density of approximately 2.5 lb/cu. ft.
  2. Spray Polyurethane Insulation: See Section 07 21 19 "Foamed-In-Place Insulation."
- 3.4 INSTALLATION OF INSULATION IN CEILINGS FOR SOUND ATTENUATION
- A. Where glass-fiber blankets are indicated for sound attenuation above ceilings, install blanket insulation over entire ceiling area in thicknesses indicated. Extend insulation 48 inches up either side of partitions.
- 3.5 PROTECTION
- A. Protect installed insulation and vapor retarders from damage due to harmful weather exposures, physical abuse, and other causes. Provide temporary coverings or enclosures where insulation is subject to abuse and cannot be concealed and protected by permanent construction immediately after installation.

**END OF SECTION 07 21 00**



## General

- Interpretation of drawings & specifications
  - For convenience, specifications have been prepared for this project and are arranged in several sections, but such separation shall not be considered as the limits of the work required by any separate trade. The terms and conditions of such limitations are wholly between the contractor and his subcontractors.
  - In general, the working details will indicate dimensions, positions and kind of construction, and the specifications will indicate quantities and methods. Any work indicated on the working details mentioned but not in the specifications, or vice versa, shall be furnished as though fully set forth in both. Work not particularly detailed, marked, or specified, shall be the same as similar parts that are detailed, marked, or specified. If conflicts occur between drawings and specifications, the most expensive materials or methods will prevail.
  - Should an error appear in the working details or specifications or in work done by others affecting this work, the contractor shall notify the architect at once and in writing. If the contractor proceeds with the work so affected without having given such written notice and without receiving the necessary approval, decision or instruction in writing from the owner, then he shall have no valid claim against the owner, for the cost of so proceeding and shall make good any resulting damage or defect. No verbal approval, decision, or instruction shall be valid or be the basis for any claim against the owner, its officers, employees or agents. The foregoing includes typical errors in the specifications or notational errors in the working details where the interpretation is doubtful or where the error is sufficiently apparent as to place a reasonably prudent contractor on notice that, should he elect to proceed, he is doing so at his own risk.
- Construction shall conform to all applicable codes and regulations.
- Shop Drawing Note:
  - Shop drawings shall be submitted in the form of one reproducible and two copies of each sheet.
  - The purpose of shop drawing submittals by the Contractor is to demonstrate to the Structural Engineer that he understands the design concept by indicating which materials he intends to furnish and install, and by detailing the fabrication and installation methods he intends to use.
  - Prior to fabrication, shop drawings shall be submitted for review to the Structural Engineer. Shop drawing submittals shall include, but are not necessarily limited to structural steel, reinforcing steel, glued laminated beams, and pre-fabricated wood roof framing items such as I-joists and trusses.
  - Prior to submission the contractor shall review all submittals for conformance with the contract documents and shall stamp submittals as being "Reviewed for Conformance."
  - Shop drawing submittals processed by the Structural Engineer are not change orders.
  - Any detail on the shop drawing that deviates from the contract documents shall clearly be marked with the note "This is a Change."
  - Shop drawings or calculations submitted for review that require resubmittal for re-review shall be billed hourly for such time to the General Contractor. Re-review will not proceed without written approval from the General Contractor for additional engineering review services.
- Safety Note:
  - It is the Contractor's responsibility to comply with the pertinent sections, as they apply to this project, of the "Construction Safety Orders" issued by the State of California, latest edition, and all OSHA requirements.
  - The contractor and Structural Engineer do not accept any responsibility for the Contractor's failure to comply with these requirements.
  - The Contractor shall be responsible for adequate design and construction of all forms and shoring required.
- The Contractor shall notify the Architect and Structural Engineer where a conflict or a discrepancy occurs between the structural drawings and any other portion of the contract documents or existing field conditions. Such notification shall be given in due time to affect the construction schedule. In case of a conflict between structural drawings and specifications, the more restrictive condition shall take precedence unless written approval has been given for the least restrictive. Contractor shall verify all dimensions with architectural and structural drawings prior to commencing any work.
- Where no specific detail is shown, the construction shall be identical or similar to that indicated for like cases of construction on this project. Should there be any question contact the Architect and Structural Engineer prior to proceeding.
- When construction attaches to an existing building, a complete set of drawings of the existing building shall be kept on the job site. Contractor to obtain these drawings from the owner.
- Contractor shall provide an allowance equal to 2% of the bid for structural steel, misc. iron, light gauge framing, and reinforcing steel to be used at the discretion of the structural engineer. Unused amount to revert to the owner upon completion of the job.
- Any substitutions for structural members, hardware, or details shall be reviewed by the Architect and Structural Engineer. Such review will be billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed.
- Do not scale drawings. Contact the Architect or Structural Engineer for any dimensions not shown.
- These drawings are not complete until reviewed and accepted by the local building official and signed by the owner and the Structural Engineer.
- All drawings and written material appearing herein constitutes the original and unpublished work of the Structural Engineer and the same may not be duplicated, used or disclosed without written consent of the Structural Engineer.
- The structure shown on these drawings is structurally sound only in its completed form. The stability of this structure depends on the diaphragms and the bracing members shown. The Contractor is to provide for the design and construction of shoring for all earth, forms, concrete, steel, wood, and masonry to resist gravity, earth, wind, seismic, and construction loads. Shoring shall remain in place until all diaphragms and lateral resisting elements are in place in their entirety. Construction materials shall be spread out if placed on framed floors or roofs. Load shall not exceed the design live load per square foot.

- ## Lightgauge Metal Framing
- All metal framing shall be formed from corrosion resistant steel conforming to ASTM A653 or ASTM A1011 with minimum yield strength of 33 ksi for 18 ga and lighter and 50 ksi for 16 ga and heavier.
  - Metal framing shown on the structural drawings shall have channel type sections with stiffened flanges having the minimum properties as shown in the light gauge metal framing schedule.
  - Metal tracks shall be the same gauge as framing which it supports, unless noted otherwise, with minimum flange width of 1/4" and minimum properties as shown in the light gauge metal framing schedule.
  - Galvanized coating must meet the ASTM G455 specification.
  - Factory punch-outs to be located along the centerline of the webs of the members and have a minimum center-to-center spacing of 24". Punch-outs to have a maximum width=1/2", and a maximum length=4". Lightgauge framing members shall be cut such that the minimum distance between the end of the member and the near edge of the web punch-out=10".
  - All header members shall be un-punched.
  - Design properties of metal framing studs, channels & tracks shall conform to (or exceed) the Steel Stud Manufacturer's Association (SSMA) Product Technical Information catalog # ICC Report # ESR-3064F.

## Design Criteria

- Code: 2022 California Building Code (CBC)
  - Design Loads:
 

Area	Live Load	Remarks	Dead Load
Roof	L = 20 psf	Reducible per code	D = 32 psf
Floor	L = 100 psf	Reducible per code	D = 40 psf
  - Snow Design Parameters: N/A
  - Wind Design Parameters:
 

Basic Design Wind Speed (3-sec gust)	V = 45 mph
Nominal Design Wind speed (3-sec gust)	V <sub>nom</sub> = 74 mph
Risk Category	II
Exposure Category	C
Internal Pressure Coefficient	0.18
Analysis Method	Directional Procedure
- Roof Pressures for Components & Cladding:**
- Wind Uplift loads (zones defined per ASCE T-16 fig. 30.3-2 thru 30.3-6):
 

A) Zone 1:	12 psf
B) Zone 2:	34 psf
C) Zone 3:	50 psf
  - Discontinuity Distance: a = 7.2 ft
- Wall Pressures for Components & Cladding:**
- |          |          |
|----------|----------|
| Zone 4:  | 20.8 psf |
| Zone 5:  | 23.9 psf |
| Parapet: | 54.4 psf |
- Earthquake Design Parameters:
 

5.1. Seismic Importance Factor	I <sub>e</sub> = 1.0
5.2. Risk Category	II
5.3. Soil Site Classification	D'
5.4. Seismic Design Category	D'
5.5. Mapped Spectral Response Accel	
A) Short period	S <sub>s</sub> = 0.125g
B) 1-sec period	S <sub>1</sub> = 0.283g
5.6. Design Spectral Response Accel	
A) Short Period	S <sub>ps</sub> = 0.540g
B) 1-sec period	S <sub>p1</sub> = 0.16g
5.7. Seismic Force Resisting System	Steel Special Concentric Braced Frames, Semi Rigid Diaphragm at roof, Rigid Diaphragm at floors.
5.8. Seismic Base Shear	V = 540 kips
5.9. Seismic Response Coefficient	C <sub>s</sub> = 0.048g
5.10. Response Modification Factor	R = 6.0
5.11. Component Amplification Factor	
A) Condenser	ap = 2.5
B) Generator	ap = 1.0
5.12. Component Response Modification Factor	
A) Condenser	R <sub>p</sub> = 6.0
B) Generator	R <sub>p</sub> = 2.5
5.13. Analysis Procedure	Equivalent Lateral Force

## Foundations

- Foundation design is based on the Geotechnical Engineering Report, MPE No. 06357-01 by Mid Pacific Engineering, Inc, dated April 18, 2024.
- All building pad preparation and foundation work shall be done in accordance with the contents of the geotechnical report. Copies of the report may be obtained from the engineer upon request.
- The Geotechnical Engineer shall observe all footing excavations prior to placement of reinforcing steel and concrete.
- Foundation depths indicated on plans are for estimating purposes only. Actual depths are to be determined by the Geotechnical Engineer on the jobsite.
- When structural observation is required, structural engineer shall observe footing reinforcing steel prior to concrete placement. Provide 48 hours notice to structural engineer prior to concrete placement.
- The contractor shall be solely responsible for all excavation procedures including, but not limited to, lagging, shoring and protection of adjacent property, structures, streets, and utilities in accordance with the local building department.
- Foundation type: mat slab supported by helical anchors (designed by others) coefficient of friction = 0.20 passive lateral earth pressure = 150 pcF
- New basement retaining walls shall be designed to resist "at-rest" lateral earth pressures equal to an equivalent fluid pressure of 75 psf per foot of wall backfill for conditions of walls fixed at the top.

## Metal Deck

- Provide metal decking manufactured by Verco, ASC Profiles, or approved equivalent of type and gauge as shown on plans. Decking shall have the following minimum effective section properties uncoated:
 

Location	Type	Gauge	I (in <sup>4</sup> /ft)	I (in <sup>4</sup> /ft)	S <sub>x</sub> (in <sup>3</sup> /ft)	S <sub>y</sub> (in <sup>3</sup> /ft)
Roof	3.5D	18	2.415	2.272	0.980	1.070
Floor	3.5D-FormLok	16	3.133	2.960	1.317	1.371
Stair Landing	B-36	16	0.381	0.381	0.349	0.410
- Metal floor deck shall be composite type, conforming to ASTM A-653, Grade 40 min. or equal and shall have a galvanized, G-60 grade coating. If contractor chooses to substitute decking by alternative manufacturer, contractor shall submit complete supporting data showing comparison of structural properties, gravity & shear values to that of the specified deck.
- Metal roof deck shall conform to ASTM A-653, Grade 40 min. or equal and shall have a galvanized, G-60 grade coating.
- Prior to fabrication, the Contractor shall submit shop drawings and ICC/APMO evaluation report for product used to the Architect and Structural Engineer for review. Shop drawings should indicate deck gauge, size, and layout as well as closure conditions, welds to supports and side lap details.
- Connection and welding of decking to structural supports and at deck side seams shall be as specified in the structural drawings.
- No conduit or non-structural items may be placed in concrete fill over metal deck.
- When placing concrete over metal deck, concrete should first be placed over beams and girders rather than at mid-span of the metal deck and concrete should not be piled beyond the finished depth of the slab.
- All metal deck receiving concrete fill shall have factory-punched vent tabs to provide 1% ventilation typical.

## Elevator Guide Rails & Supports

- Guide rails, guide rail supports and brackets for elevator cab or counterweight shall be designed to meet the force and displacement requirements of ASCE T-16 Section 13.6.1f.
- Provide Shop Drawings and engineering calculations for the guide rails, guide rails supports and supporting brackets signed by a Civil Engineer, registered in the state which the projects located, for review by the Architect and/or Structural Engineer, and for approval by the enforcement agency prior to fabrication.
- All loading conditions resulting in eccentricities or torsion to beams and/or columns shall be resolved by the installation of stiffeners and diagonal struts designed and installed by the supplier.
- Seismic forces shall be accounted for and braced back to the structure. For additional requirements see California Code of Regulations Title 24 part 2 and 7.
- See Architectural drawings for opening/pit dimensions, locations and details.

## Concrete

- Structural concrete shall attain 28-day compressive strength as required in note #30. Maximum slump shall not exceed 4".
- Concrete mix designs shall be prepared by a registered Civil Engineer, reviewed by Owner's testing laboratory and submitted to the Structural Engineer for review. Selection of concrete mix proportions shall be per ACI 318-19 Section 26.4.3 & 26.4.4.
- Cementitious materials:
  - Cement shall conform to ASTM C-150 type I or II.
  - Fly ash shall conform to ASTM C-618. Max quantity of fly ash shall be as given in specs (15% max u.n.o.)
- Concrete aggregates shall conform to ASTM C-33 for normal weight concrete and ASTM C-330 for light weight concrete.
- Water shall be clean and free from injurious amounts of oils, acids, alkalis, salts, organic materials or other substances deleterious to concrete or reinforcement.
- Non-shrink grout or drypack shall consist of a premixed nonmetallic formula. See note #27 for additional information.
- Reinforcing steel shall conform to ASTM A615-grade 60 for #4 and larger, and ASTM A615-grade 40 for #3 and smaller, except reinforcing steel to be welded shall conform to ASTM A706. Contractor shall submit rebar mill certificates.
- Welded wire fabric shall conform to ASTM A-1064.
- All preheating and welding of reinforcing bars shall be done in accordance with AWS D1.4 latest edition and shall be continuously inspected by a qualified laboratory. Contractor shall furnish IWFS for all rebar welding to the laboratory. Reinforcing steel shall be fabricated according to "Manual of Standard Practice for Reinforced Concrete Construction".
- Dimensions shown for location of reinforcing are to the face of bars listed and denote clear coverage. Non-prestressed, cast-in-place concrete coverage shall be as follows, u.n.o.:
 

Cast against earth (except slabs)	3"
Cast in forms and exposed to earth or weather	2"
#6 & larger	2"
#5 & smaller	1 1/2"
Beams & columns (ties)	1 1/2"
Beams & columns (main reinf)	2"
Cast-in-place walls	
(exterior face & soil side)	see above
Cast-in-place walls	
(interior face - #11 & smaller)	3/4"
Tilt-up walls	see details
Slabs (on forms)	
Slabs (on ground)	2" c/c from top u.n.o.
- Splices in continuous reinforcement shall be lapped u.n.o., lap bars per note 31 u.n.o., splices in adjacent bars shall be greater than 5'-0" apart. Splice continuous bars in soil-bearing grade beams, structural slabs on grade and mat foundations as follows u.n.o.: top bars at centerline of support; bottom bars at mid-span. Splice continuous bars in elevated slabs and beams, etc., as follows u.n.o.: top bars at mid-span bottom bars at support ends. All bars size #14 and larger shall be continuous for full length shown or spliced with mechanical couplers as noted in details. Splices in WWF shall overlap 2 squares minimum.
- The minimum clear spacing between parallel bars in a layer shall not be less than the larger of bar diameter, 1", or 33% greater than the maximum aggregate size (nominal), whichever is greatest. This requirement also applies to the clear spacing between different layers of parallel bars and to the clear distance between a contact lap splice and adjacent splices or bars.
- All hooks shall be standard hooks unless otherwise shown or noted. At walls, provide hooks at ends of all reinforcing ends, corners and intersections, u.n.o.
- Provide construction/control joints @ all slabs on grade as noted on plan. Proposed joint plan shall be submitted to the Structural Engineer for approval prior to construction. Concrete surface at construction joints shall be thoroughly cleaned and laitance removed. Where indicated on drawings, roughen concrete surface to 1/4" amplitude. Concrete may be roughened by chipping the entire surface, sand blasting, or raking the surface to provide 1/4" deep deformations.
- Remove all debris from forms before casting dry concrete.
- Reinforcing, dowels, bolts, anchors, sleeves, etc., to be embedded in concrete shall be securely positioned in forms before placing concrete.
- Pipes and electrical conduits shall not be embedded in structural concrete or concrete fill over metal decking except where specifically approved by the Structural Engineer.
- Anchor bolts (AB's) cast in concrete or masonry for wall sill and ledger/ applications shall be headed bolts with cut threads conforming to ASTM A307 or F1554 u.n.o. Refer to "Wood notes" for additional requirements for bolts in contact with pressure treated or fire retardant material. Refer to "Structural steel" note for requirements for anchor rods cast in concrete for column base plate and steel embed applications.
- Walls shall be cast in horizontal layers of 2'-0" maximum depth.
- Concrete in walls, piers or columns shall set at least 2 hours before placing concrete in beams, spandrels, or slabs supported thereon.
- Consolidate concrete placed in forms by mechanical vibrating equipment supplemented by hand-spading, rodding or tamping. Use equipment and procedures for consolidation of concrete in accordance with the recommended practices of ACI 304 to suit the type of concrete and project conditions. Concrete shall not be dropped through reinforcing steel (as in walls) so as to cause segregation of aggregates. In such cases hoppers and chutes or trunks of variable lengths shall be used so that the free unconfined fall of concrete shall not exceed 6 feet.
- Drill through steel columns, beams and plates to pass continuous reinforcing, u.n.o.
- No wood spreaders allowed. No wood stakes allowed in areas to be concreted.
- Additional reinforcing in precast or tilt-up panels required for lifting stresses shall be supplied by contractor.
- Provide #5x4'-0" diagonal reinforcing at mid-depth of slab at all re-entrant corners typical. This applies to slab on grade, concrete over metal deck, and elevated structural slab conditions.
- Place non-shrink grout under base plates, sill plates, etc as indicated on the drawings. Non-shrink grout shall be Mastertion 428 Grout by Master Builders Technologies or approved equal with a minimum f'c of 7500 psi @ 28 days.
- All saw cutting shall be done after initial set has occurred to avoid tearing or damage by the saw blade, but before initial shrinkage has occurred.
- Notify Structural Engineer a minimum of 48 hours before placing any concrete.
- Concrete strength: (max slump = 4")

Use	f'c @ 28 days	Max Aggregate Size	Density (lbs/ft <sup>3</sup> )	Max W/C Ratio
Foundations	3000 psi	1 1/2"	145	0.58
Slab-on-grade	3500 psi	1"	145	0.45
Walls and pilasters	3500 psi	1"	145	.52
Concrete fill of metal deck	3500 psi	3/8"	145	0.52
Exterior flatwork	2500 psi	1"	145	0.60

- Development lengths shall be provided per the table below unless noted otherwise.

Bar	Straight Bars		With Standard Hooks	
	f'c	f'c	f'c	f'c
	3000 psi	4000 psi	3000 psi	4000 psi
#3	15"	21"	#3	6"
#4	24"	25"	#4	11"
#5	36"	31"	#5	14"
#6	43"	37"	#6	17"
#7	63"	54"	#7	20"
#8	72"	62"	#8	22"
#9	80"	70"	#9	25"

## Test and Inspections

- Tests and Inspections shall be provided as required below and shall conform to the requirements of the 2022 CBC, Chapter 17.
- All Test and Inspections shall be performed by a certified special inspector from an established Testing & Inspection Company, unless noted otherwise. Jobsite visits by the Structural Engineer do not constitute inspections and are not a substitute for special inspection.
- The special inspector shall observe the work indicated for conformance with the approved construction documents.
- The special inspector shall furnish inspection reports to the building department, the engineer or architect of record, and other designated persons. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority and to the building department.
- The special inspector shall submit a final signed report stating whether the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved construction documents and the applicable workmanship provisions of the 2022 CBC.
- It is the contractor's sole responsibility to see that these tests and inspections are performed.
- Required Tests and Inspections are indicated below with a solid filled rectangle "■".
- Continuous notation indicates the full-time observation of work requiring special inspection by an approved special inspector who is present at the work area. Periodic notation indicates the intermittent observation of work.

- Note: Coordinate with building department, Test & Inspection firm.
- |   |  |
|---|--|
| ■ A. Compact fill   |  |
| ■ B. Concrete mix design, cement, aggregates & admixtures   |  |
| ■ C. Concrete strength f'c test   |  |
| ■ D. Reinforcing steel mill certification   |  |
| ■ E. Structural steel mill certification  |  |
| ■ F. Structural steel, cold formed steel, and anchor bolt sampling & testing (if not properly identified) |  |
| □ G. Masonry strength f'm   |  |
| □ H. Masonry mortar, grout proportion, aggregates, additives  |  |
| ■ I. Post installed anchors: Expansion / Epoxy Anchors  |  |
| ■ J. High strength bolts, nuts and washers  |  |
| ■ K. End-welded studs   |  |
| □ L. Beam to column moment connection   |  |
| □ M. Veneer bond strength test  |  |
| □ N. Shotcrete preconstruction test   |  |
| □ O. Shotcrete strength & core test   |  |
| □ P. Prefabricated items  |  |

	Verification and Inspection	Continuous	Periodic
A. STEEL			
1. Material verification of high-strength bolts, nuts & washers	○	●	●
2. Inspection of high-strength bolting, bearing & typical connections	○	●	●
3. Inspection of Welding Structural Steel: (field/shop) Complete & partial penetration groove welds	●	○	○
Multi-pass fillet welds	●	○	○
Single-pass fillet welds : 3/8"	●	○	○
Single-pass fillet welds : 5/8"	●	○	○
Floor and roof deck welds	○	●	●
4. Inspection of Steel Frame Joint Details for Compliance with Approved Construction Documents	○	●	●
5. Automatic end-weld stud shear connectors	○	●	●
B. CONCRETE			
1. Concrete Placement	●	○	○
2. Inspection of reinforcing steel & placement	○	●	●
3. Inspection of anchors cast in concrete	○	●	●
4. Precast concrete attachments & inserts	○	●	●
5. Erection of precast concrete members	○	●	●
C. WOOD			
1. Verify grade and thickness of sheathing	○	●	●
2. Verify nominal size of framing members at adjoining panel edges	○	●	●
3. Verify nail diameter and length, number of fastener lines, spacing between fasteners in each line and at edge margins	○	●	●
4. Verify positive connection of wood members supporting balcony or deck connections to exterior walls prior to concealment	○	●	●
D. MASONRY PLACEMENT & GROUTING	○	●	●
Note: refer to TMS 602-16 Tables 3 & 4			
1. Level 2 masonry inspection (Risk Categories I, II, III)	○	●	●
2. Level 3 masonry inspection (Category IV, DSA, OSHFD)	○	●	●
E. SOIL (by Geotechnical Engineer)			
1. Footing excavation	○	●	●
2. Pile/Pier foundation	○	●	●
3. Material verification below footing	○	●	●
4. Excavation verification to proper depth	○	●	●
5. Placement and compaction of controlled fill	○	●	●
6. Site preparation prior to placement of controlled fill	○	●	●
F. POST-INSTALLED ANCHORS			
1. Expansion anchor installation	○	●	●
2. Epoxy anchor installation	○	●	●

## Demolition

- Safety Notes:
  - It is the Contractor's responsibility to comply with the pertinent sections, as they apply to this project, of the "Construction Safety Orders" issued by the State of California, latest edition, and all OSHA requirements.
  - The Structural Engineer and Owner do not accept any responsibility for the Contractor's failure to comply with these requirements.
- Shore or brace trusses, beams, columns, and walls as required to maintain the stable integrity of the existing structure prior to demolition. It is the Contractor's sole responsibility to design and provide competent shoring and bracing for all loads imposed during and after demolition through completion of new construction.
- All dimensions given to and of the existing structure are approximate. Verify by field measurements or approved equal with a minimum f'c of 7500 psi @ 28 days. Where actual conditions deviate from the details shown on the drawings, notify the Structural Engineer for instructions prior to proceeding with work.
- Demolition and removal of existing construction shall be made in such a manner as to avoid or minimize damage to adjacent construction.
- Extent of demolition is to be as indicated on plans, sections and details. Demolition is to include removal and disposal construction.

## Structural Observation

- This structure requires "Structural Observation" per 2022 CBC section 1709. The Architect or Engineer of record responsible for the structural design shall perform the structural observation. Observed deficiencies shall be reported in writing. In addition, final written documentation shall be issued stating that the necessary site visits have been made and identifying any reported deficiencies that, to the best of the structural observer's knowledge, have not been resolved.
- At a minimum, structural observations are required at the following stages of construction:
  - Prior to concrete placement of first footing pour when all reinforcing steel is in place.
  - When 75% percent of the rough framing is in place.
  - When 100% of the roof framing is in place.
- It is the Contractor's responsibility to notify the Architect and Structural Engineer at least 48 hours in advance of these Structural Observations.
- Structural Observations do not constitute special inspections of any type.



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net

www.architechnica.net

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2479 Sunrise Blvd.  
Gold River, CA 95670  
Tel: (916) 631-3030  
Fax: (916) 631-8996  
Web: www.bevier.net  
Bevier Job No.23011



Electronically Signed On: 12/14/24

CONSULTANT



CODESTACK  
ACADEMY

201 N CALIFORNIA ST.  
STOCKTON, CA 95202

SAN JOAQUIN COUNTY  
OFFICE OF EDUCATION

REVISIONS
Bid Set 11/06/2024
Addendum #5 12/04/2024

PROJECT NO: BSE 23011

ISSUE SET: BID SET

ISSUE DATE: 12.04.24

DRAWN BY: TB, JRW, MC

## GENERAL NOTES

S1.1



## Structural Steel

- Fabrication, erection and materials shall conform to the specifications and standards of the AISC, as contained in the "AISC 360-16 Specification of Structural Steel Buildings" & the "AISC Manual of Steel Construction", 15th edition and California Building Code latest edition.
- Structural steel shall conform to the following specifications, u.n.o.:

Shapes	
Wide Flanges (W, WT)	ASTM A992
Wide Flanges (S, M), Angles (L)	ASTM A572
Channels (C), Misc Channels (MC)	ASTM A36 (B'), ASTM A992 (2B')
Hollow Structural Steel (HSS)	ASTM A500, Gr. C (F <sub>y</sub> = 50 ksi)
Steel Circular Pipes (P)	ASTM A53, Type E or S, Gr. B
Plates & Bars	
Column Base Plates	ASTM A36 / ASTM A572, Gr. 50
Brace Gusset Plates	ASTM A36
Beam Shear Connection Plates	ASTM A36
Column Continuity Plates	ASTM A572, Gr. 50
Beam Stiffener Plates	ASTM A36
Deck Closure Plates	ASTM A36
Stainless Steel Plates & Bars	ASTM A276
Other	ASTM A36
Nuts, Bolts, Rods, & Washers	
General Bolts	ASTM F3125, Gr A325-N
Slip Critical Bolts (see note #4 below)	ASTM F3125, Gr A325-SC
High Strength Bolts	ASTM F3125, Gr A325-N or Gr A490
Machine Bolts (general use)	ASTM A307
Bent & Headed Anchor Bolts	ASTM F1554, Gr. 36, 55, or 105
Partial & Fully Threaded Anchor Rods	ASTM F1554, Gr. 36, 55, or 105
Fully Threaded Rod (general use)	ASTM A36 (A307 Gr. A for 3/8"φ)
Welded Shear Connectors	ASTM A108, Gr. 1015 thru 1020
Welded Threaded Studs	ASTM A108, Gr. 1015 thru 1020
Nuts for Bolts & Machine Bolts	ASTM A563
Hardened Washers	ASTM F436
Unhardened Washers	ASTM F844
Plain Washers	ASTM B18.22.1
Beveled Washers	ASTM B18.23.1

- Bolted connections shall consist of unfinished bolts per the table above unless noted otherwise. Anchor bolts cast in concrete or masonry shall be headed bolts with cut thread, full diameter body style conforming to ASTM F1554 u.n.o. Unless noted otherwise, anchor bolts/rods shall be grade 36 except that welded anchor bolts shall be grade 55 per 51 Supplementary requirements. All bolted connections and base plates shall have standard cut washers unless noted otherwise. Washers at base plates shall be placed at top and bottom of plate.
- "Slip-critical" bolted connections:
  - "Slip-critical" connections (A325-SC design values with special inspection) are required at all braced frame connections, at all connections along chord lines and drag lines (as noted on plans), and u.n.o., at all bolts in oversized or slotted holes.
  - The special inspector must be present during installation and tightening operation of "slip-critical" connections.
- All structural steel shall receive minimum of one shop coat of red primer with a minimum dry film thickness of 2.0 mils. Do not shop prime or paint areas to be field welded, fireproofed, galvanized, to receive slip-critical high strength bolts, or to be embedded in concrete. Prior to priming or painting, clean structural steel in accordance with Steel Structures Painting Council (SSPC) recommendations & as required by the primer & paint manufacturer. Provide additional painting as noted in the specifications.
- All structural steel shall be erected plumb and true to line. Temporary bracing shall be installed and shall be left in place until other means are provided to adequately brace the structure. Contractor responsible for reviewing all base plate and support conditions during erection and bracing as required. See AISC and CSI requirements.
- Place non-shrink grout under all base plates before adding vertical load. See Concrete Notes for non-shrink grout requirements.
- Structural steel below grade shall have 3" minimum of concrete cover.
- Provide 1/2"Ø stitch bolts and ring fills, space at not more than 24" cc for all double angle members.
- At wood to steel parallel contact, attach with 1/2"Ø welded threaded studs at maximum 32"cc, & 6" from ends of wood member, typical unless noted otherwise.
- Holes for unfinished bolts shall be of the same nominal diameter of the bolt plus 1/16". Use standard AISC gage and pitch for bolts except as noted otherwise. Holes for anchor bolts embedded in concrete shall be of the same nominal bolt diameter plus 3/16" unless noted otherwise.
- Welding shall be done by the electric arc process in accordance with American Welding Society standards, using only certified welders. All groove welds shall have complete penetration unless noted otherwise. All exposed welds shall be ground smooth. All welding to be done using ET0xx electrodes. In addition, welding of ASTM A572 grade 50 steel and ASTM A992 steel shall be done with electrodes capable of depositing weld metal with a maximum diffusible hydrogen content of 16ml/100g (H16). Weld lengths called for on plans are the net effective lengths required.
- Minimum fillet welds:
  - 3/16" @ t < 1/2"
  - 1/4" @ t > 1/2"
  - 3/8" @ t > 3/4"
- Welding Procedure Specifications (WPS) for shop and field pre-qualified weld joints and weld joints qualified by test shall be prepared for review prior to fabrication. All welding procedures that meet these requirements of AWS D1.1 Sec. 5.1 shall be considered as pre-qualified. Qualification testing is required when the depth of a partial penetration or complete penetration weld is 2" or greater.
- Structural steel connections that are permanent to weather shall be either primed and painted or hot dipped galvanized in accordance with ASTM A123 & A153. Repair galvanizing after welding in accordance with ASTM A180.
- When structural steel & connections will be exposed to view in the completed building, they shall be fabricated, erected & finished in compliance with Architecturally Exposed Structural Steel (AESS) guidelines & Section 10 of the AISC 309-22 "Code of Standard Practice for Steel Buildings and Bridges".
- All used in members and connections is the Seismic-Force Resisting System shall be made with filler metals meeting the requirements specified in clauses 6.1, 6.1, and 6.3 of Structural Welding Code - Seismic Supplement (AWS D1.8/D1.8M).

## Powder Activated Fasteners (Shot Pins) - Hilti

- These notes govern all conditions called out on the plans as "PAF" or "shot pins", unless noted otherwise.
- Installation, testing & inspection of all PAFs shall be in accordance with the applicable evaluation report, these plans, and any project specifications.
- PAFs specified in these notes shall be used for dry, interior applications only.
- All PAFs shall be manufactured by Hilti, Inc. Tulsa, Oklahoma in accordance with the ICC evaluation report referenced below.

Connected Material	Base Material	Base Material Thickness, 'L'	Minimum Penetration Into Base Material (W)	Hilti Fastener	Evaluation Report (Issue Date)
Metal Decking	Steel (all grades)	1/8" ≤ 'L' < 3/8"	Full	X-HSN24 (W)	ESR-2147 (11/2022)
		1/4" ≤ 'L' < 1/2"	Full	X-ENP-14 L15	
Cold Formed Steel (Light Gauge) & Non-Preservative Treated Wood	Steel (all grades)	3/8" ≤ 'L' < 1/2"	Full	X-U w/ PB washer	ESR-2264 (06/2022)
		1/2" & thicker	1/2"		
		Normal Weight Concrete (Including concrete fill over metal decking) (a)	3x penetration min		
2x Preservative Treated Wood	Concrete (a)	CMU (grouted) (b) (c)		X-CP T2	ESR-2319 (08/2021)
		4 1/2" min	1 3/8"		

### Table Footnotes:

- 3" minimum edge distance & 4" minimum spacing required. Installations in concrete over metal deck may be installed either from underneath through the metal deck or from above directly into the concrete. For fasteners into the bottom of metal deck, spacing parallel to the deck flutes shall be 51" minimum.
- 4" minimum edge distance, and no more than one fastener shall be located in any given cell.
- Fasteners installed in the face of CMU shall be installed 1" minimum away from vertical mortar joints. At bed joints, fasteners shall not be spaced closer than 6"cc and must be installed a minimum of 8" from the end of the wall.
- 1 1/2" minimum concrete edge distance required. Locate fastener 6" from ends of sill plates.
- Full penetration means the entire length of the tapered tip shall penetrate completely through the base material.

## Expansion Anchors-Concrete:

- Use Hilti Knik Bolt-TZ2 Expansion Anchors as manufactured by Hilti Inc., Tulsa Oklahoma. ICC-ES Report No. ESR-4266 reissued December 2021.
- Installation of anchors shall be in accordance with the manufacturer's recommendations, ICC-ES Report, and these notes.
- Special inspection is required in accordance with the 2022 CBC Sections 1705A.11.3 and 1910A.5. Special inspector must verify product, expiration date, concrete type and strength, anchor diameter and steel grade, compliance of drill bit, hole diameter and location, cleanliness of hole and anchor, and anchor embedment.
- Each anchor type (loaded in either pullout or shear) shall be torque tested in accordance with CBC Section 1910A.5 to the appropriate test load shown in the table. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may be resumed.
- When installing anchors in existing concrete do not cut or damage existing reinforcing bars. Locate existing reinforcing bars with pachometer or x-ray if required.
- The testing of the anchors shall be done by the Testing Laboratory and a report of the test results shall be submitted to the Building Dept. and Architect/Structural Engineer.
- Anchors installed up into the bottom of metal deck with concrete fill shall be installed in the center of the low flute of the decking. The decking shall have a minimum thickness of 20 gauge. The minimum depth of embedment above the top of the deck shall be 1 1/2". The effective depth of embedment is considered to be one-third of the metal deck height plus the depth of embedment above the top of the deck. There shall be a minimum concrete cover of 1" between the top surface of the concrete and the end of the bolt.

Normal Weight Concrete Anchors			
Anchor Diameter	Embedment Effective/Nominal u.n.o.	Carbon Steel Anchors	Stainless Steel Anchors
		Installation Torque (ft-lbs)	Installation Torque (ft-lbs)
3/8"	2" / 2 1/2"	30	30
1/2"	3 1/4" / 3 3/4"	50	40
5/8"	4" / 4 1/2"	40	60
3/4"	4 3/4" / 5 1/2"	110	125

## Automatic End Welded Studs

- Automatic end welded studs shall be Tru-Weld Shear Connector Studs or approved equivalent. Studs shall be manufactured of C-1010 through C-1020 cold rolled steel which conforms to ASTM specifications A-108 or A-24.
- The studs shall be automatically end welded in accordance with the manufacturer's recommendations in such a manner as to provide complete fusion between the end of the stud and the plate. There should be no porosity or evidence of lack of fusion between the welded end of the stud and the plate. The stud shall decrease in length during welding approximately 1/8" for 3/8" diameter studs and under, and 3/16" for studs over 3/8" diameter. Welding shall be done only by qualified welders approved by a qualified welding inspector.
- Inspection of all the shop and field welding operations for automatic end welded studs shall be made by a qualified welding inspector, in accordance with the Structural Welding Code-Steel, latest edition by the American Welding Society. The type and capacity of the welding equipment shall be in accordance with manufacturer's recommendations and shall be checked and approved by the welding inspector. At the beginning of each days work, a minimum of two test stud welds shall be made, with the equipment to be used, to metal which is the same as the actual work piece. The test studs shall be subjected to a 40° bend test by striking with a heavy hammer. After the above test, the weld section shall not exhibit any tearing out or cracking.

## Adhesive Anchors-Concrete

- Where "Hilti" or "Simpson" post-installed adhesive anchors or post-installed reinforcing bars in concrete are called out on plan, the following Hilti or Simpson adhesive products shall be used, respectively. Substitutions between or for other products shall be approved by the engineer prior to use.
  - Hilti HIT-HY 200 V3 Adhesive as manufactured by Hilti, Inc. ICC Report No. ESR-4868 revised June 2023.
  - Simpson "SET-36" Adhesive as manufactured by Simpson Strong-Tie, Inc. ICC-ES Report No. ESR-4051 reissued April 2023.
- Installation, inspection & testing of anchors shall be in accordance with the manufacturer's recommendations, ICC-ES report and these notes.
- Threaded rod anchors shall be F1554, Grade 36 u.n.o.
- Continuous special inspection is required in accordance with CBC Section 1701. Special inspector must verify product, expiration date, concrete type and strength, anchor diameter and steel grade, compliance of drill bit, hole diameter and location, cleanliness of hole and anchor, adhesive application, and anchor embedment. See "Test and Inspections" section of plans for additional information. Where pull-test loads are designated on plan, each anchor type (loaded in either pullout or shear) shall have 50% of the anchors (alternate in each group arrangement) tested in tension to the tension load shown. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may be resumed. Where pull-test loads are not shown, pull-testing is not required.
- The testing of the anchors shall be done by the Testing Laboratory and a report of the test results shall be submitted to the Building Dept. and the Architect/Structural Engineer. Testing shall occur after full epoxy cure time has elapsed (24 hours min). Where the number of anchors of a specific size and type exceed 100, the following testing procedure may be used. The first 40 anchors shall be tested as specified in note 5 above, then 10% of the additional anchors shall be tested. Any failure shall be handled in the same manner as specified in note 5 above.
- When installing anchors in existing concrete do not cut or damage existing reinforcing bars. Locate existing reinforcing bars with pachometer or x-ray if required.

## Concrete Masonry

- 28-day compressive strength of concrete masonry (fm) shall be fm = 2000psi for all uses. Full masonry stresses are used in design.
- Concrete block units shall conform to ASTM C-90. Units shall be lightweight with a maximum unit weight of 105 pcf.
- Mortar shall be Type S.
- Grout shall comply with ASTM C476. All cells to fully-grouted unless specified otherwise on plan.
- Compliance with the requirements for the specified compressive strength of masonry, fm shall be in accordance with section 51.4B of the TMS402/602-16. For unit strength method see table below for required 28-day compressive strength of the concrete block units, grout, and mortar.
 

specified fm	required 28-day compressive strength		
	conc. block units (psi)	grout (psi)	mortar (psi)
2000	2000	2000 min	1800
2250	2600	2250 min	1800
2500	3250	2500 min	1800

 Unit strength method shall not be used for specified compressive strengths in excess of 2500 psi.
- Reinforcing steel shall conform to ASTM A615-grade 60 for #4 and larger, grade 40 for #3 and smaller.
- All reinforcement shall be continuous. Stagger splices where possible. Lap bars 4B diameters minimum, unless noted otherwise.
- Vertical reinforcing shall be held in position at top and bottom and at intervals not to exceed 20Ø diameters.
- Each vertical bar in walls shall lap 4B diameters with a dowel of the same size extending into the foundation. Carry each dowel to within 3" of the bottom of the foundation and terminate with 90° hook. Dowels shall be straight and plumb.
- Place all horizontal bars in bond beam units. When 2 bars are used, stagger laps a minimum of 5'-0".
- Provide 2-#5 bars (full height of wall at jamb and extending a minimum of 2'-6" past edges of openings at head and sill) east side of all openings and each end of all walls, unless noted otherwise on drawings.
- Before block is placed on concrete, thoroughly clean concrete and remove all laitance and loose material. Roughen concrete surface to 1/8" amplitude.
- Concrete block masonry shall be built to preserve the unobstructed vertical continuity of the cells. All head and bed joints shall be solidly filled with mortar for a distance in from the face of the unit not less than the thickness of the face shell. Bond shall be provided by lapping successive courses or by equivalent mechanical anchorage.
- Vertical cells shall have vertical alignment sufficient to maintain a clear unobstructed continuous vertical cell measuring not less than 2"x3".
- Low Lift (lift height up to 5'-4")
  - All cells shall be filled solidly with grout. Grout shall be placed in a continuous pour in lifts not exceeding 5'-4" where cleanouts are not provided. All grouting shall be done under the continuous observation of the owner's testing laboratory.
  - High Lift (lift height greater than 5'-4" and up to 12'-8")
    - Cleanout openings shall be provided in the bottom course of wall to be filled at each lift or pour of grout where such lift or pour of grout is in excess of 5'-4" in height. Maximum lift or pour height shall not exceed 12'-8". Cleanouts shall be provided at each cell. However, if the course at the bottom of the pour is constructed entirely of inverted open-end bond beam units, cleanout openings need only be provided at reinforced cells. Maximum cleanout spacing shall not exceed 32'cc. The cleanouts shall be sealed after inspection and before grouting.
- Thoroughly clean all cells and bond beams of mortar projections, mortar droppings, or other foreign material before grouting.
- All grout shall be thoroughly consolidated by mechanical vibration during placement in a manner to provide solidly grouted spaces.
- When grouting is stopped for one hour or longer, horizontal construction joints shall be formed by stopping the pour of grout 1/2" below the top of the uppermost unit.
- All embedded items (bolts, etc.) shall be securely positioned prior to grouting. Provide a minimum of 1" grout around all bolts in masonry. See Typical Details Sheet.
- Pipes and electrical conduits shall not be embedded in concrete masonry except where specifically approved by the structural engineer.
- Use open end block for all stack bond construction.

- Use Hilti HIT-HY 210 adhesive as manufactured by Hilti, Inc., Tulsa, Oklahoma. ICC-ES Report No. ESR-4143 issued January, 2022, u.n.o.
- Installation of anchors shall be in accordance with the manufacturer's recommendations, ESR report, and these notes.
- Special inspection is required in accordance with 2022 CBC Section 1704 & the ESR report. Special inspector must verify:
  - Anchor type, diameter & length, and adhesive product type & expiration.
  - Installation description, including verification of masonry compressive strength, anchor installation & location (spacing & edge distance) in accordance with the manufacturer's published instructions and ESR Report.
- Do not cut or damage existing reinforcing bars. Locate with pachometer or x-ray if necessary.
- Base material temperature @ time of installation shall be between 23°-104° F.
- Anchors shall be A36 threaded rod, u.n.o. Anchors exposed to exterior weathering conditions shall be Type 304 or 316 stainless steel or hot-dipped galvanized.
- Each anchor type (loaded in either pullout or shear) shall have the following percentage of anchors pull-tested to the test load shown in the table below. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may resume:
  - Structural anchorage: 100% u.n.o.
  - Anchorage of non-structural elements: 50%
  - Sill E. bolts: 10%
- Pull-testing shall be done by the Testing Laboratory in the presence of the Special inspector and a report of the test results shall be submitted to the Building Dept. and the Architect/Structural Engineer. Testing shall occur 24 hrs. minimum after anchor installation.

## Window System Design Criteria

- All mullions and their connections shall be designed to span between structural supports as shown on drawings. Verify ceiling heights with architectural drawings.
- All mullions and their connections shall allow for a relative movement between stories of not less than 1/2" due to seismic loads.
- Submit complete shop drawings and calculations signed by a Civil Engineer registered in the state in which the project is located, prior to fabrication. Details provided in these drawings are for reference only. Window system manufacturer shall design and supply all connection materials (including embedded items, diagonal bracing angles, brackets, outriggers, etc.) as required for the support of the window system. Embedded items shall be installed by the Contractor.

## Abbreviations

add..... Additional	LLH.....Long leg horizontal
alt..... Alternate	LLV.....Long leg vertical
AISC..... American Institute of Steel Construction	LVL.....Laminated Veneer Lumber
APA..... American Plywood Association	MB.....Machine bolt
ASTM..... American Society for Testing and Materials	mfr.....Manufacturer
AWS..... American Welding Society	max.....Maximum
AB..... Anchor bolt	mech.....Mechanical
¢..... And	min.....Minimum
arch..... Architect/Architectural	misc.....Miscellaneous
at..... At	mtl.....Metal
b.o..... Bottom of	N.I.C.....Not in contract
bm..... Beam	(N).....New
brg..... Bearing	nts.....Not to scale
btr..... Better	#.....Number or pounds
btwn..... Between	o'.....Over
blkg..... Blocking	oc.....On center
B.S..... Both sides	ONJ.....Open web joist
bot..... Bottom	opg.....Opening
BN..... Boundary nail	opp.....Opposite
clg..... Ceiling	O.H.....Opposite Hand
cc..... Center to center	o.d.....Outside diameter
cl..... Center line	PP.....Partial penetration
clr..... Clear	pc.....piece
col..... Column	pl.....Plate
CP..... Complete Penetration	ply, plywd.....Plywood
con..... Concrete	psf.....Pounds per cubic foot
CMU..... Concrete masonry unit	psf.....Pounds per square foot
con..... Connection	psi.....Pounds per square inch
CJ..... Construction Joint	PAF..... Powder Activated Fasteners
cont..... Continuous	PTDF..... Pressure Treated Douglas Fir
csk..... Countersink	r, rad..... Radius
CTJ..... Control Joint	RDVD..... Redwood
DL..... Dead Load	reinf.....Reinforcing
det..... Detail	req'd.....Required
diag..... Diagonal	rt.....Roof
dia..... Diameter	R.O.....Rough opening
do..... Ditto	Ø.....Round or diameter
D.F..... Douglas Fir	sched.....Schedule
dbl..... Double	S.A.D..... See architectural drawings
dn..... Down	S.E.D..... See electrical drawings
dwg..... Drawing	S.M.D..... See mechanical drawings
ea..... Each	SMS..... Sheet Metal Screws
E.F..... Each Face	SDS..... Simpson Strong-Drive Screw
embed.....Embedment	SDSTS.....Self drilling self tapping screw
EN..... Edge Nail	max.....Maximum unit weight of 105 pcf.
eq..... Equal	el.....Elevation
eq..... Equipment	eq.....Equal
ex..... Existing	eq.....Equipment
EJ..... Expansion Joint	ex.....Existing
FC..... Face of Concrete	EJ.....Expansion Joint
FB..... Face of Block	FG..... Face of Concrete
FF..... Face of Masonry	FB..... Face of Block
FS..... Face of Stud	FF..... Face of Masonry
fin..... Finish	FS..... Face of Stud
fl..... Finish floor	fin.....Finish
F.G..... Finish grade	fl.....Finish floor
flr.....Floor	F.G.....Finish grade
ftg..... Footing	flr.....Floor
ft..... Foundation	ftg.....Footing
f.o..... Face of	ft.....Foundation
frmg.....Framing	f.o.....Face of
galv.....Galvanized	frmg.....Framing
gib..... Gage	galv.....Galvanized
glb..... Glued-laminated beam	gib.....Gage
g.l..... Grid Line	glb.....Glued-laminated beam
hgr..... Hanger	g.l.....Grid Line
hd..... Header	hgr.....Hanger
ht..... Height	hd.....Header
H5B.....High strength bolt	ht.....Height
H55.....Hollow Steel Section	H5B.....High strength bolt
horiz.....Horizontal	H55.....Hollow Steel Section
i.d..... Inside diameter	horiz.....Horizontal
int..... Interior	i.d.....Inside diameter
inv..... Inverted	int.....Interior
ist..... Joist	inv.....Inverted
Jh..... Joist hanger	ist.....Joist
L5.....Lag screw	Jh.....Joist hanger
lt. wt.....Light weight	L5.....Lag screw
LL..... Live Load	lt. wt.....Light weight
	LL.....Live Load

## Adhesive Anchors-Concrete Masonry

- Use Hilti HIT-HY 210 adhesive as manufactured by Hilti, Inc., Tulsa, Oklahoma. ICC-ES Report No. ESR-4143 issued January, 2022, u.n.o.
- Installation of anchors shall be in accordance with the manufacturer's recommendations, ESR report, and these notes.
- Special inspection is required in accordance with 2022 CBC Section 1704 & the ESR report. Special inspector must verify:
  - Anchor type, diameter & length, and adhesive product type & expiration.
  - Installation description, including verification of masonry compressive strength, anchor installation & location (spacing & edge distance) in accordance with the manufacturer's published instructions and ESR Report.
- Do not cut or damage existing reinforcing bars. Locate with pachometer or x-ray if necessary.
- Base material temperature @ time of installation shall be between 23°-104° F.
- Anchors shall be A36 threaded rod, u.n.o. Anchors exposed to exterior weathering conditions shall be Type 304 or 316 stainless steel or hot-dipped galvanized.
- Each anchor type (loaded in either pullout or shear) shall have the following percentage of anchors pull-tested to the test load shown in the table below. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may resume:
  - Structural anchorage: 100% u.n.o.
  - Anchorage of non-structural elements: 50%
  - Sill E. bolts: 10%
- Pull-testing shall be done by the Testing Laboratory in the presence of the Special inspector and a report of the test results shall be submitted to the Building Dept. and the Architect/Structural Engineer. Testing shall occur 24 hrs. minimum after anchor installation.

Rod Diam.	Min Embed	Face of CMU Test Load (lbs)		Top of CMU Test Load (lbs)
		4" Edge Dist	20" Edge Dist	
3/8"	3 3/8"	2000	2500	n/a
1/2"	4 1/2"	3100	4100	2400
5/8"	5 5/8"	4100	5100	2400
3/4"	6 3/8"	5100	7600	n/a



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net

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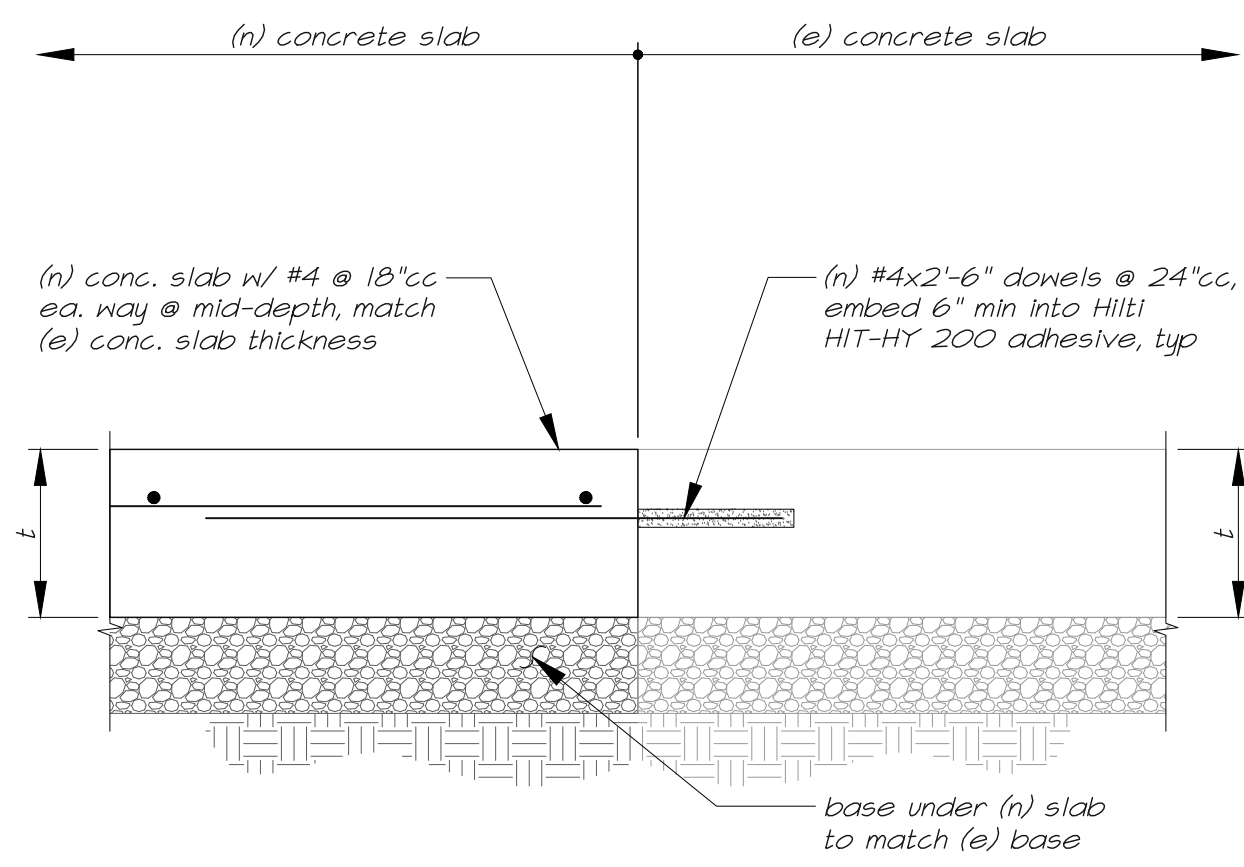
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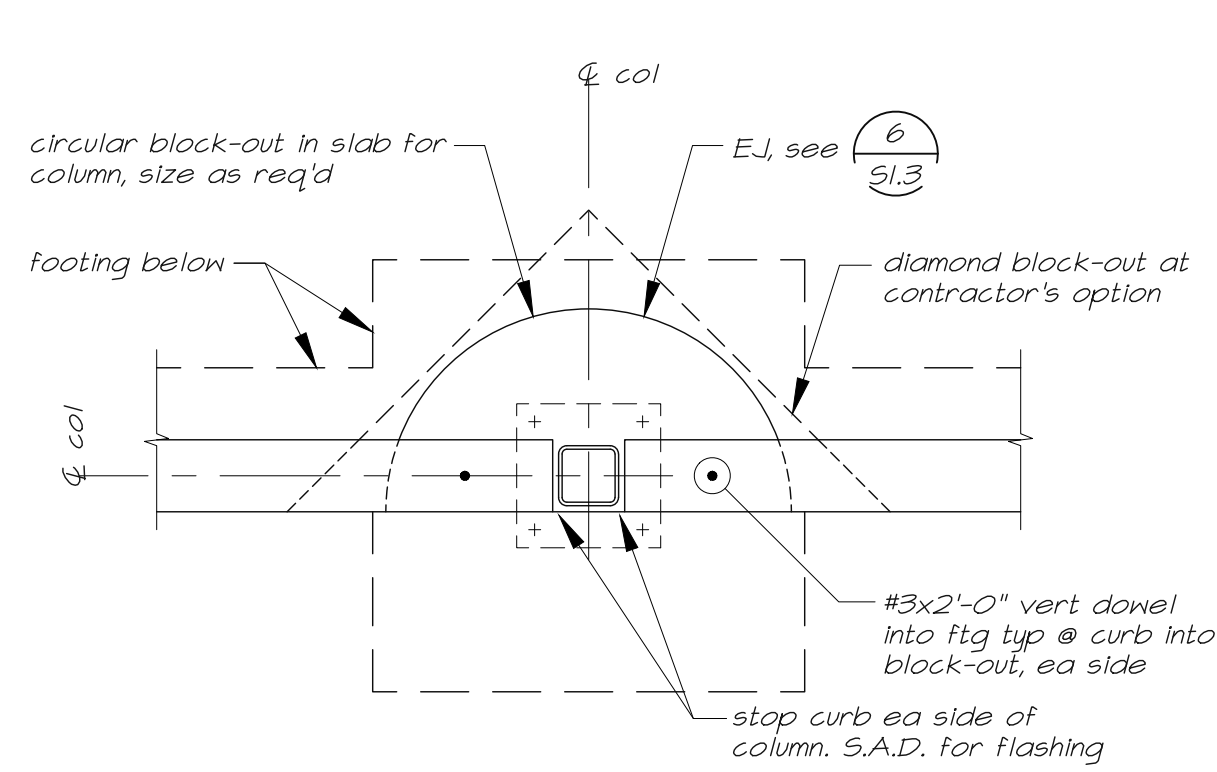
GENERAL NOTES

S1.2

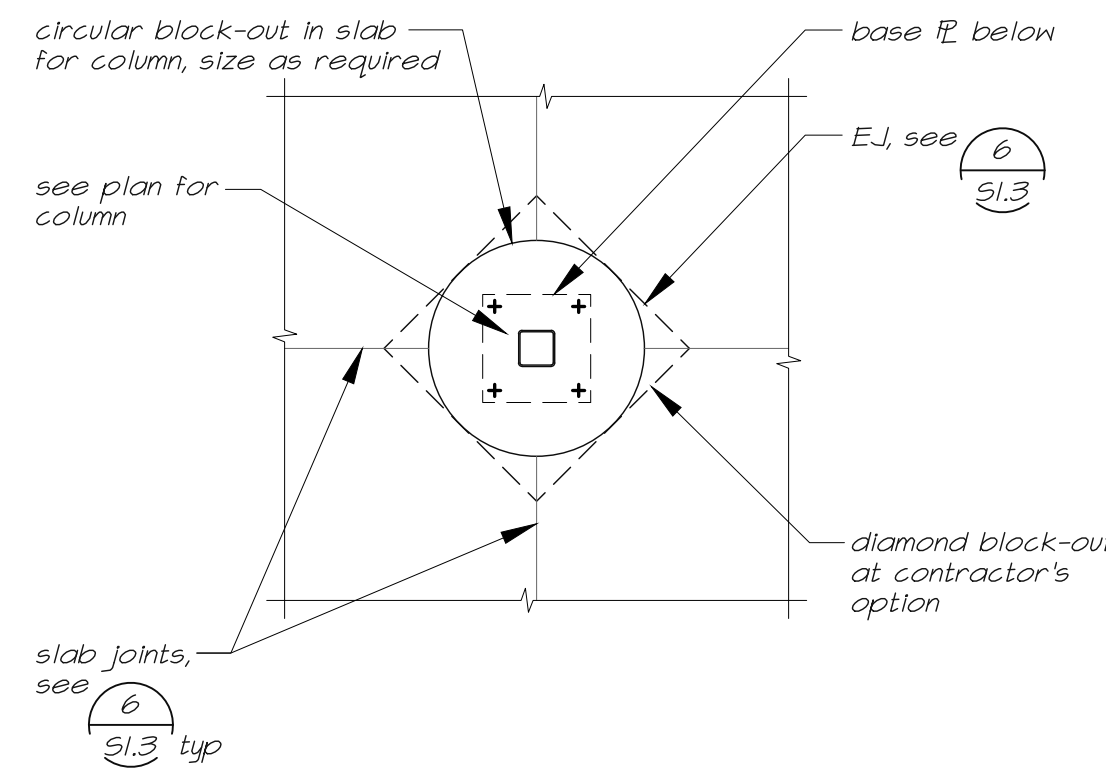




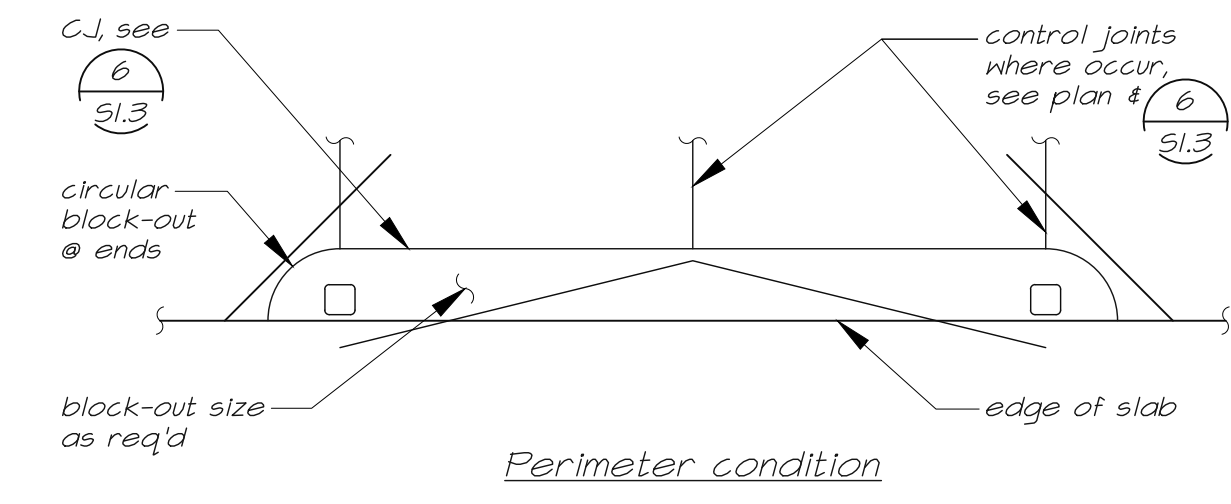
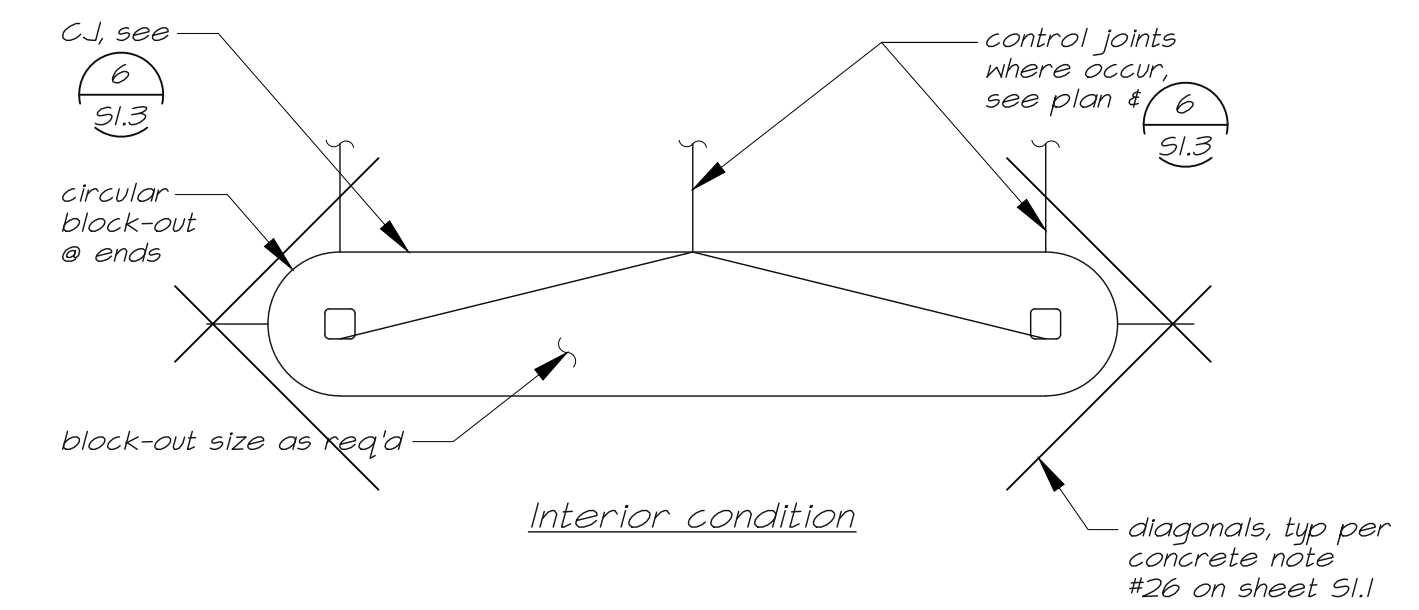
9 Typical (n) to (e) Slab  
S1.3



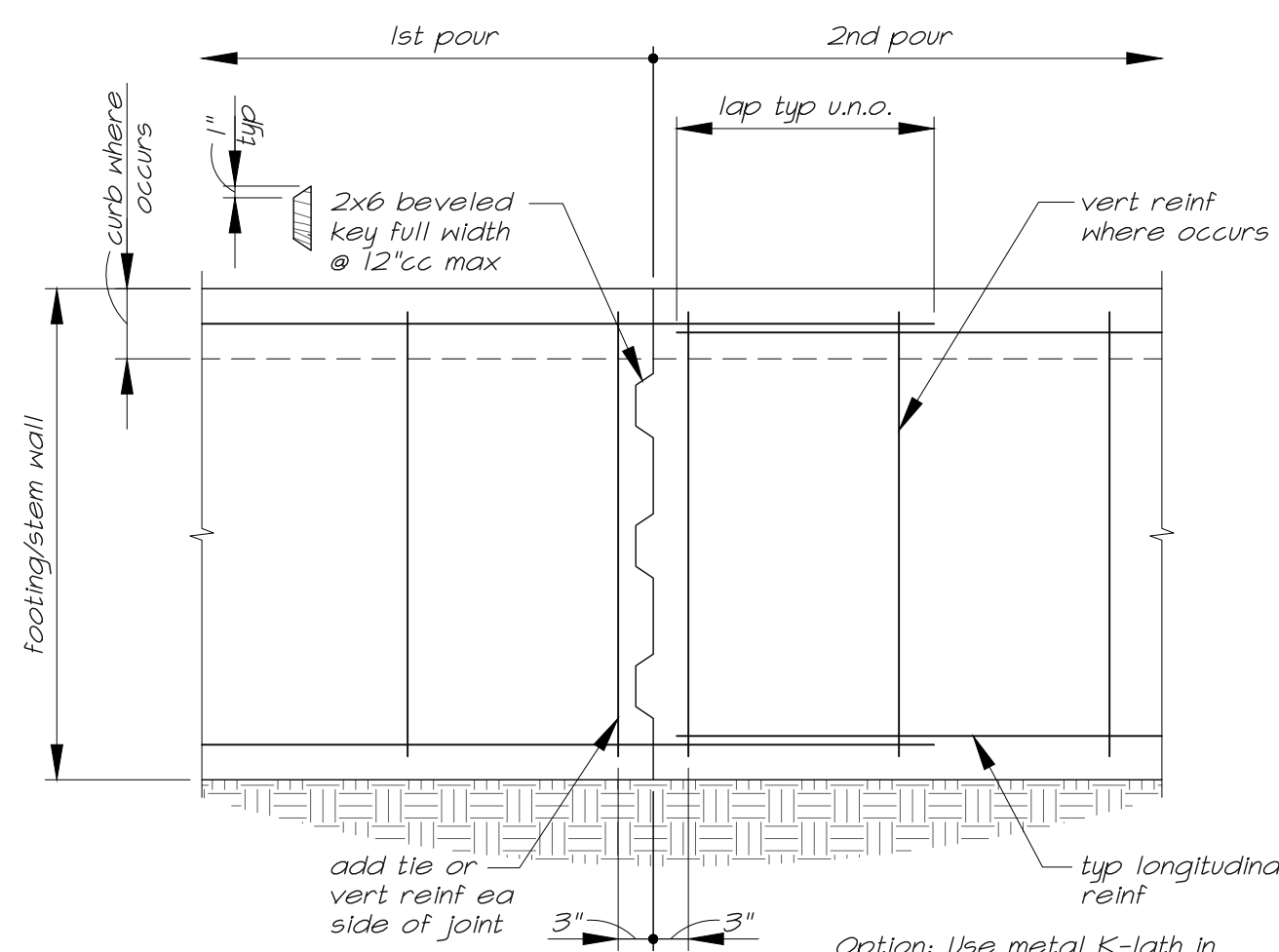
10 Typical Slab Block-out  
perimeter  
S1.3



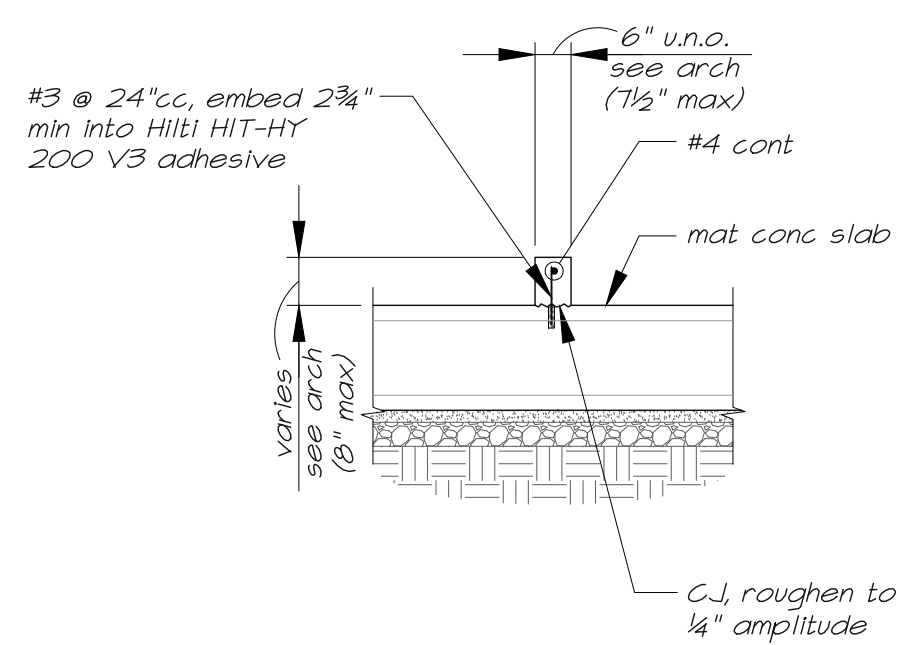
11 Typical Slab Block-out  
interior  
S1.3



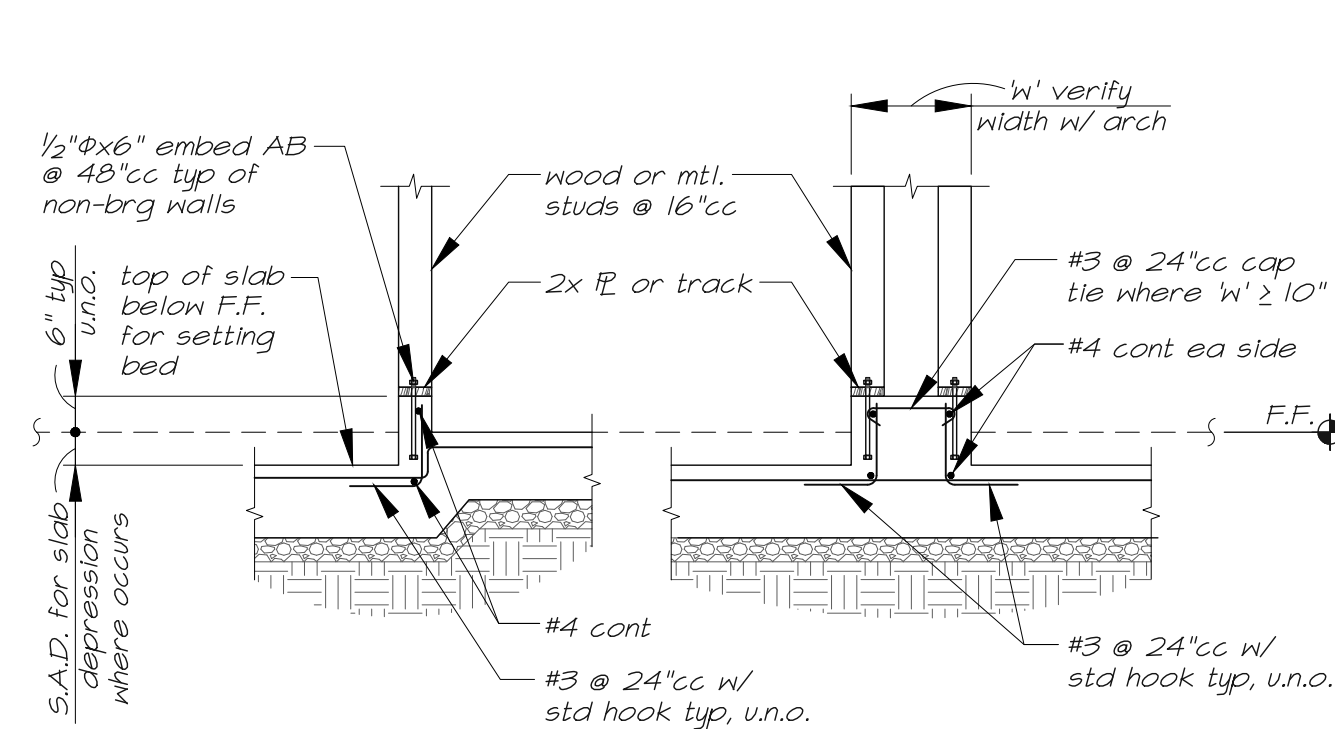
12 Typical Block-out  
@ braced frame  
S1.3



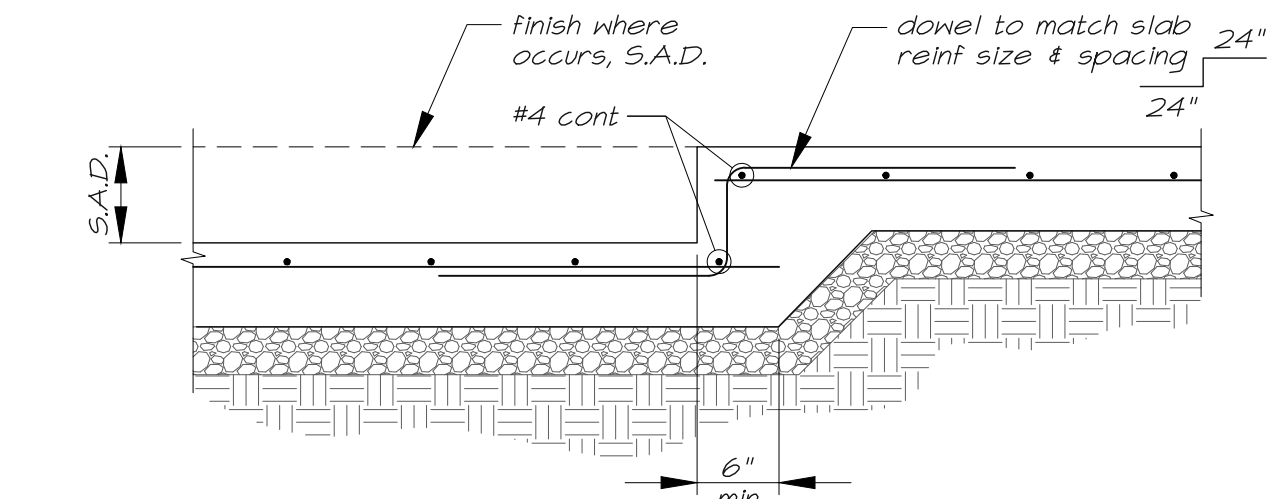
5 Typical Foundation Construction Joint  
S1.3



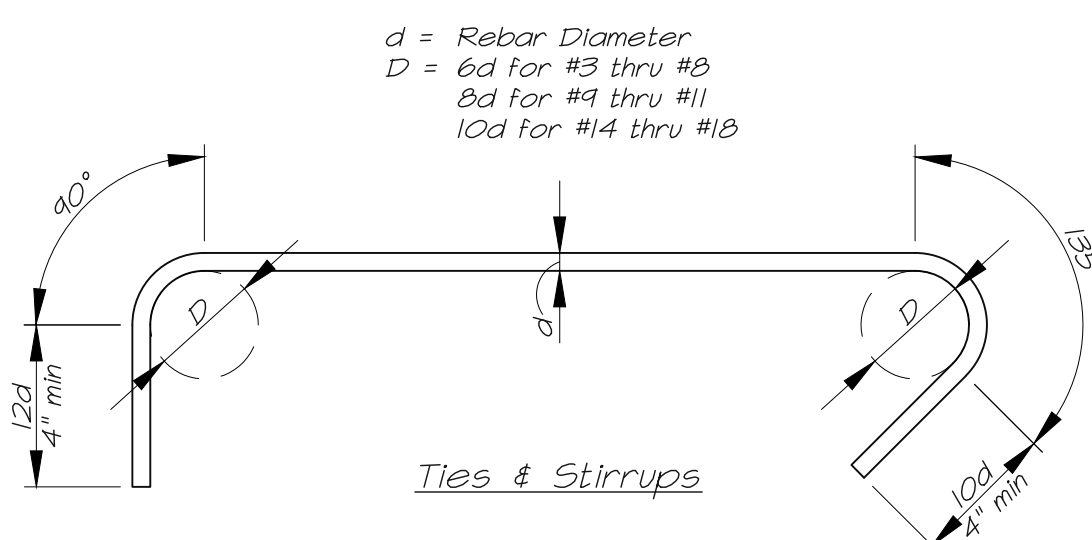
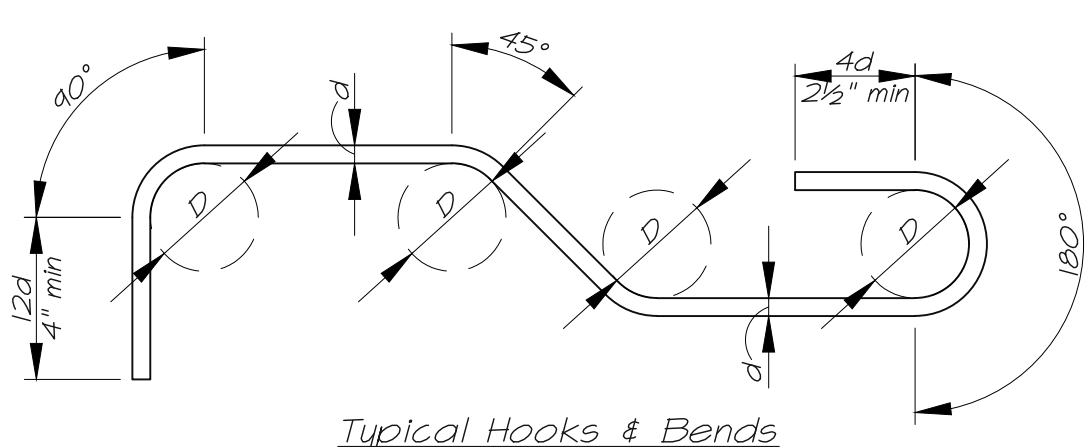
6 Typical Concrete Curb  
S1.3



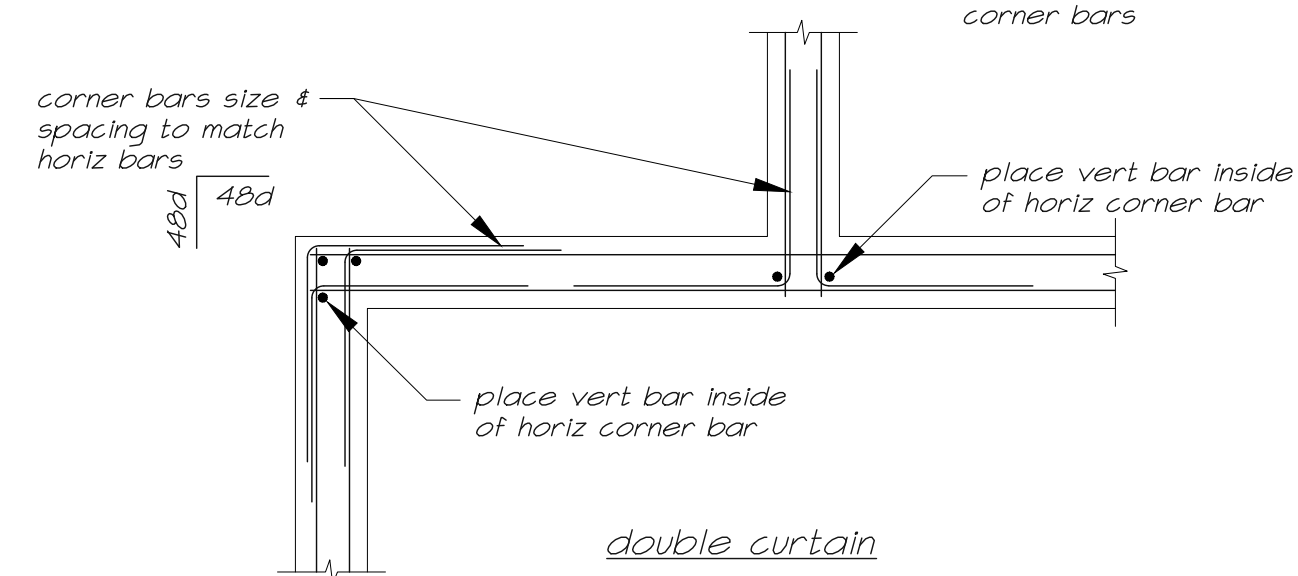
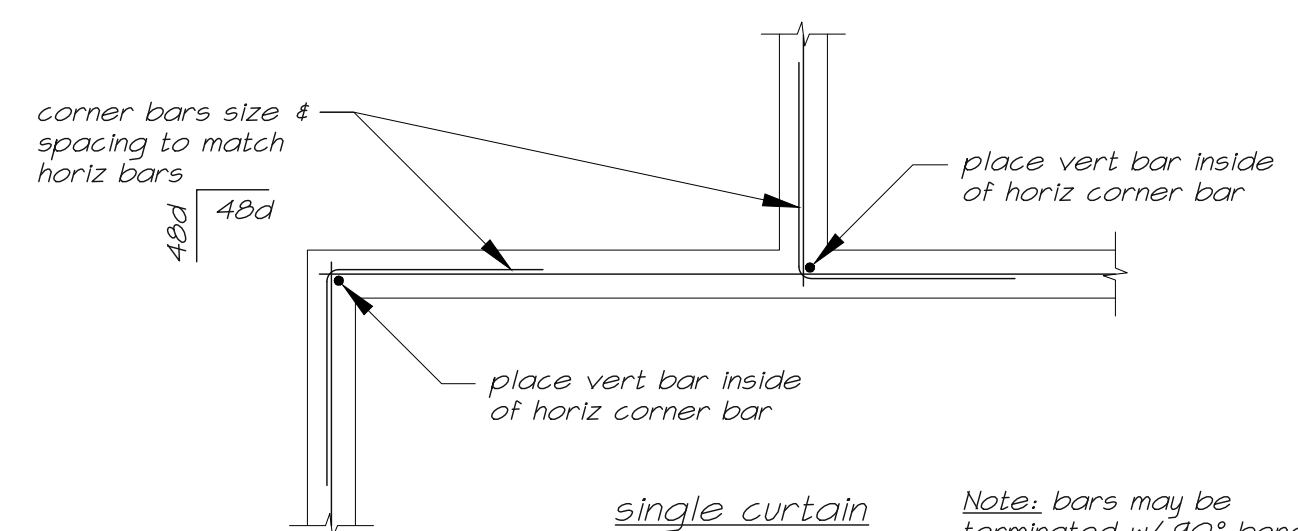
7 Typical Concrete Curb  
(at slab depression & double wall)  
S1.3



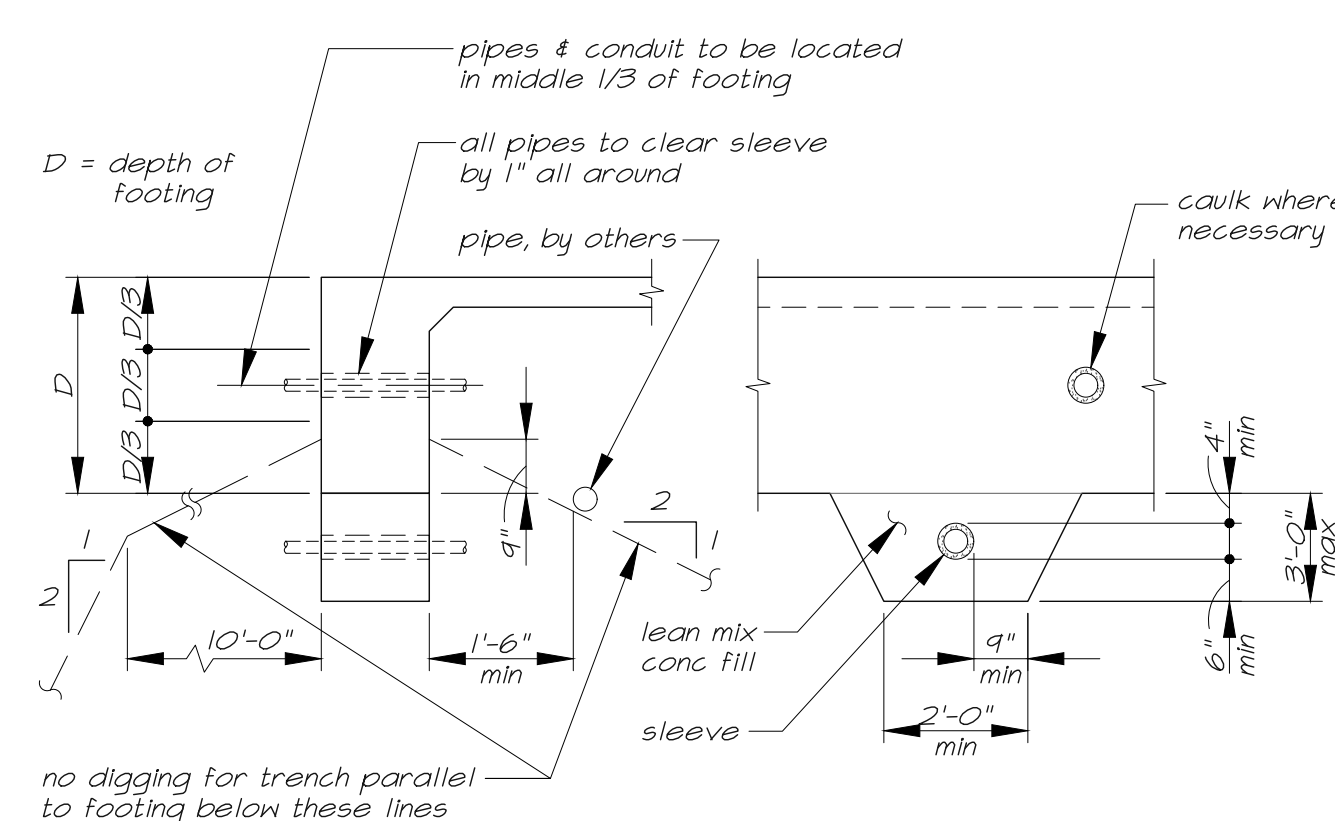
8 Typical Slab Depression  
S1.3



1 Rebar Hooks & Bends  
S1.3



2 Typical Corner Reinforcing  
S1.3



3 Concrete Footing @ Pipes & Conduits  
S1.3



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Stockton, California 95207  
P: (209) 952-5850  
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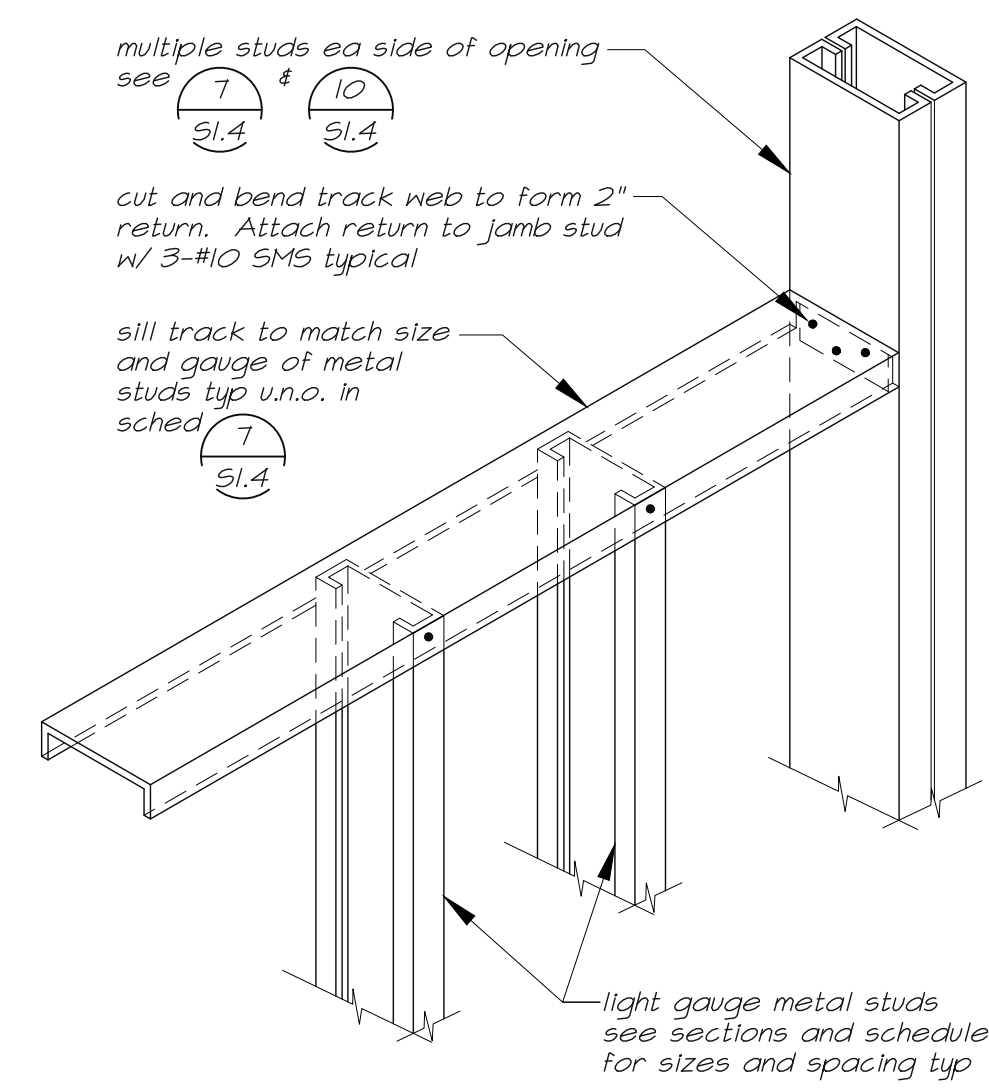
TYPICAL DETAILS



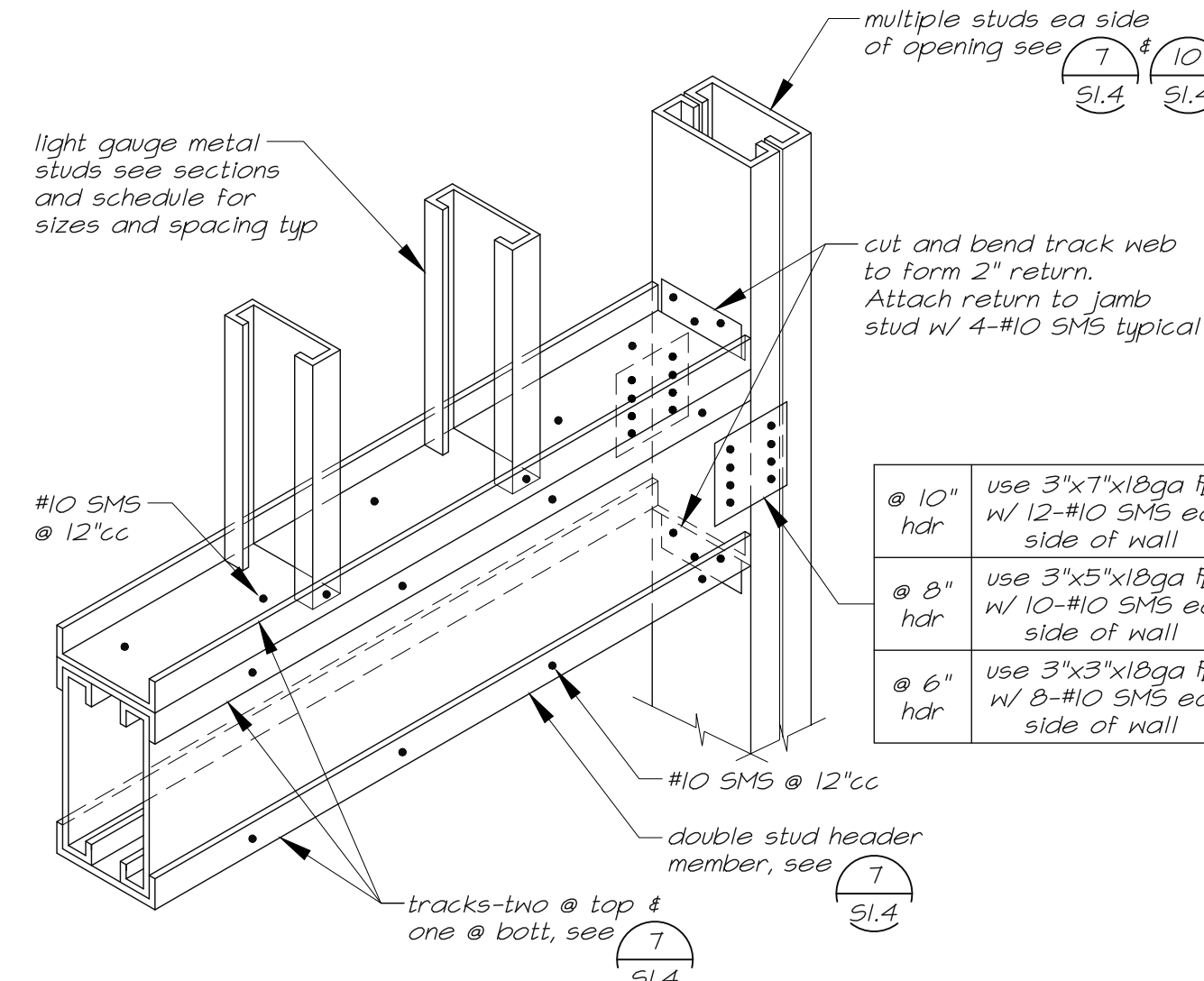
W' Opening width max	Required jamb stud	Required header section	Required sill section
4'-0" max	dbl 600S137-43	dbl 600S137-43 w/ 3-600T125-43 t&b	600T125-43
5'-0" max	dbl 600S137-43	dbl 600S137-54 w/ 600T125-43 t&b	600T125-54
12'-0" max	dbl 600S162-54	dbl 600S162-54 w/ 600T125-54 t&b	600T125-54

- Schedule Notes:**
- The requirements of this schedule shall govern unless specifically detailed or noted otherwise.
  - Metal stud section properties shall conform to the Steel Stud Manufacturer's Association Product Catalog (55MA) as specified in the Lightgauge Steel notes.
  - All header members shall be un-pinched.
  - At interior non-bearing conditions, track width @ headers and sills shall match depth of studs as specified in the architectural drawings. Flange length & ga shall match this table.
  - At spans over 12'-0", provide diagonal bracing of headers/sills to structural framing @ 4'-0"cc max per sections. ("N" = 4'-0" max @ these locations)
  - Maximum width shown in table is either the width between the jamb studs or between diagonal bracing as shown in note #5 above (where diag bracing occurs.)

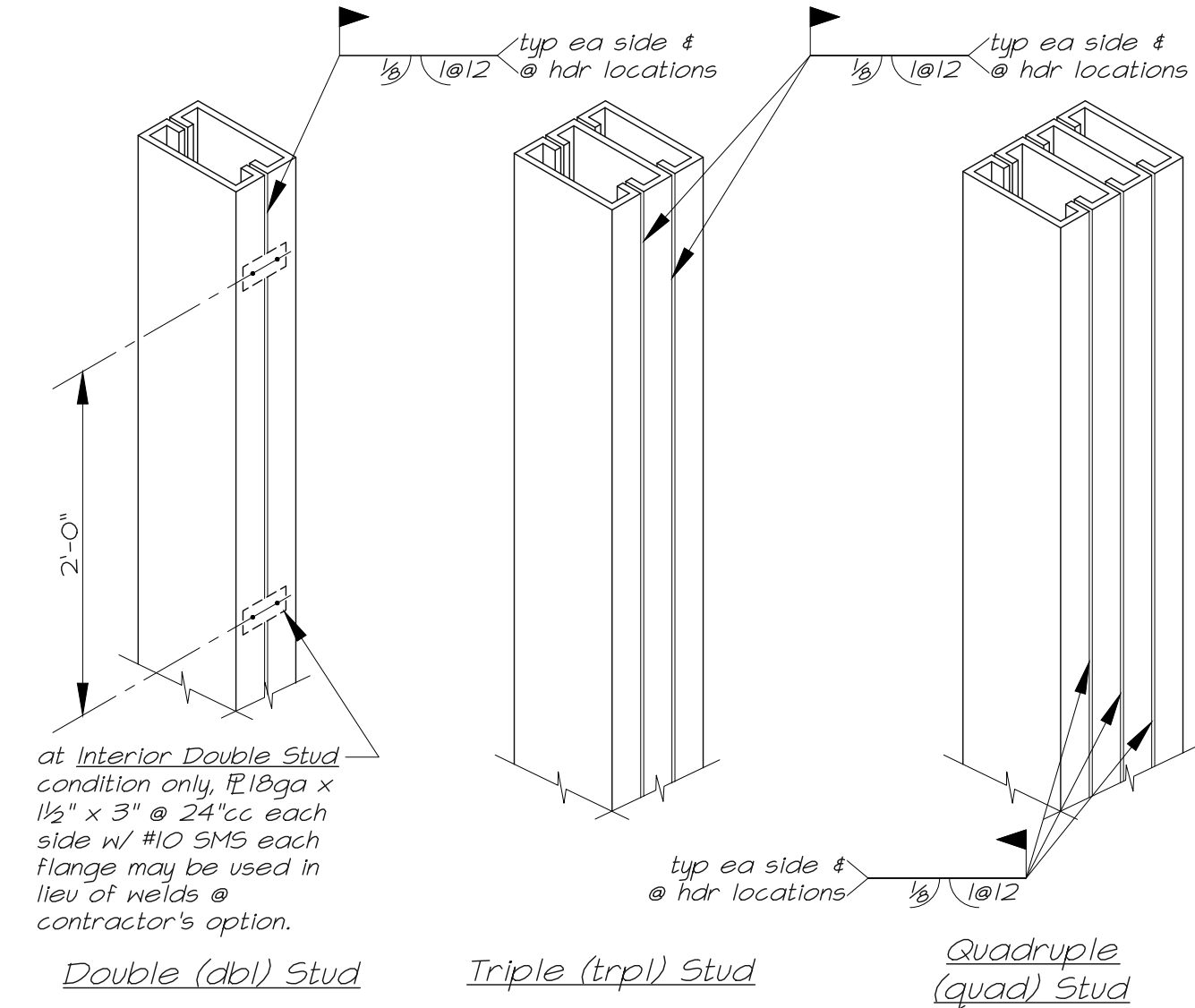
7 Exterior Wall Header Schedule



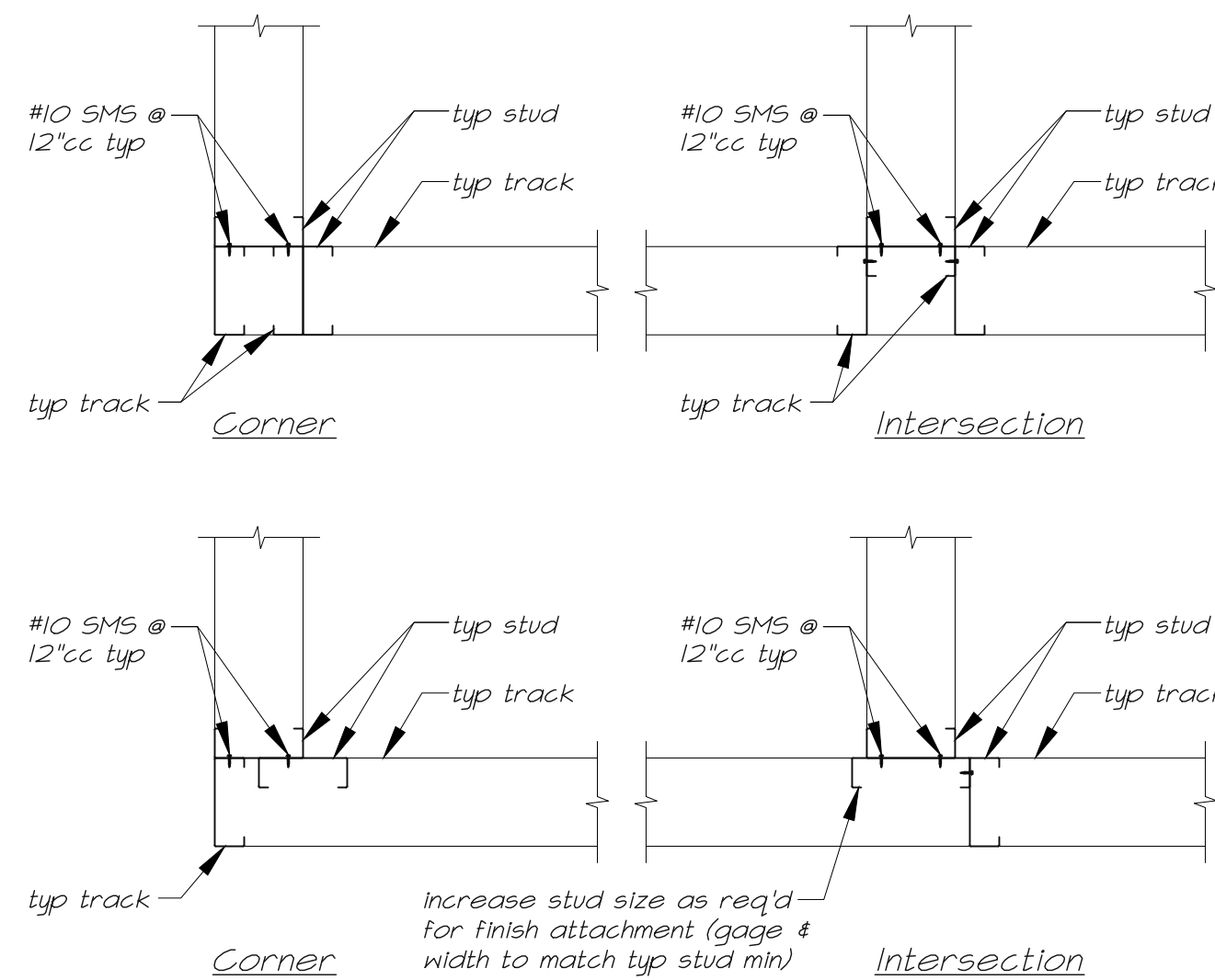
8 Typical Sill Framing



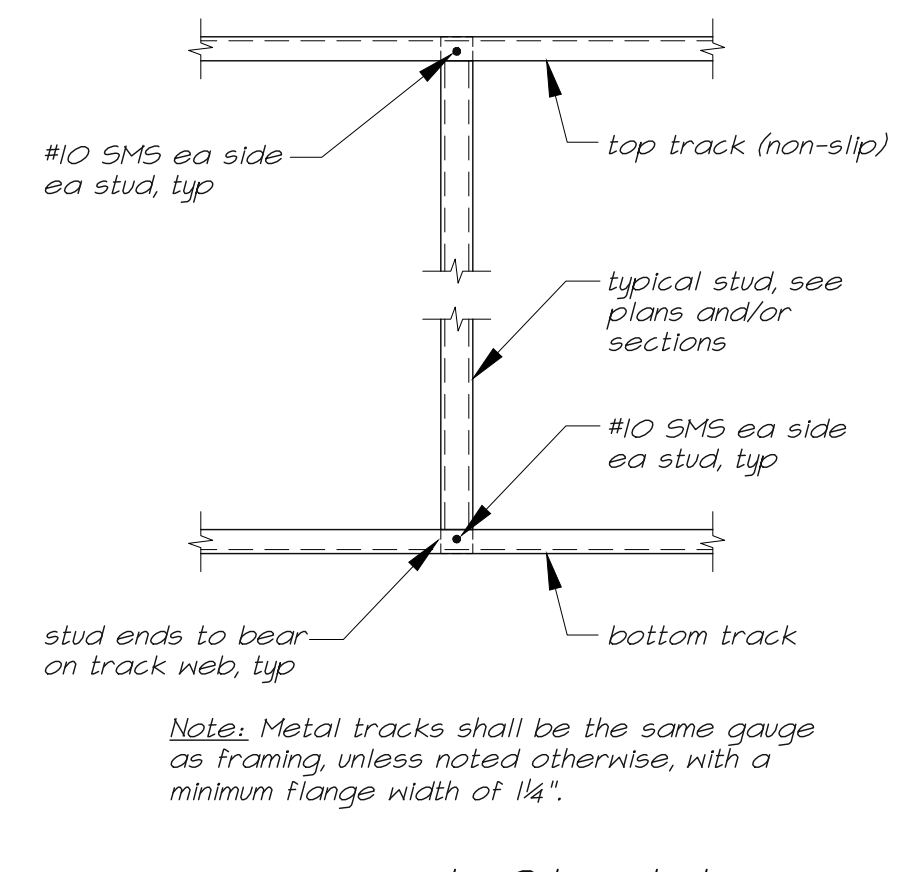
9 Typical Header Framing



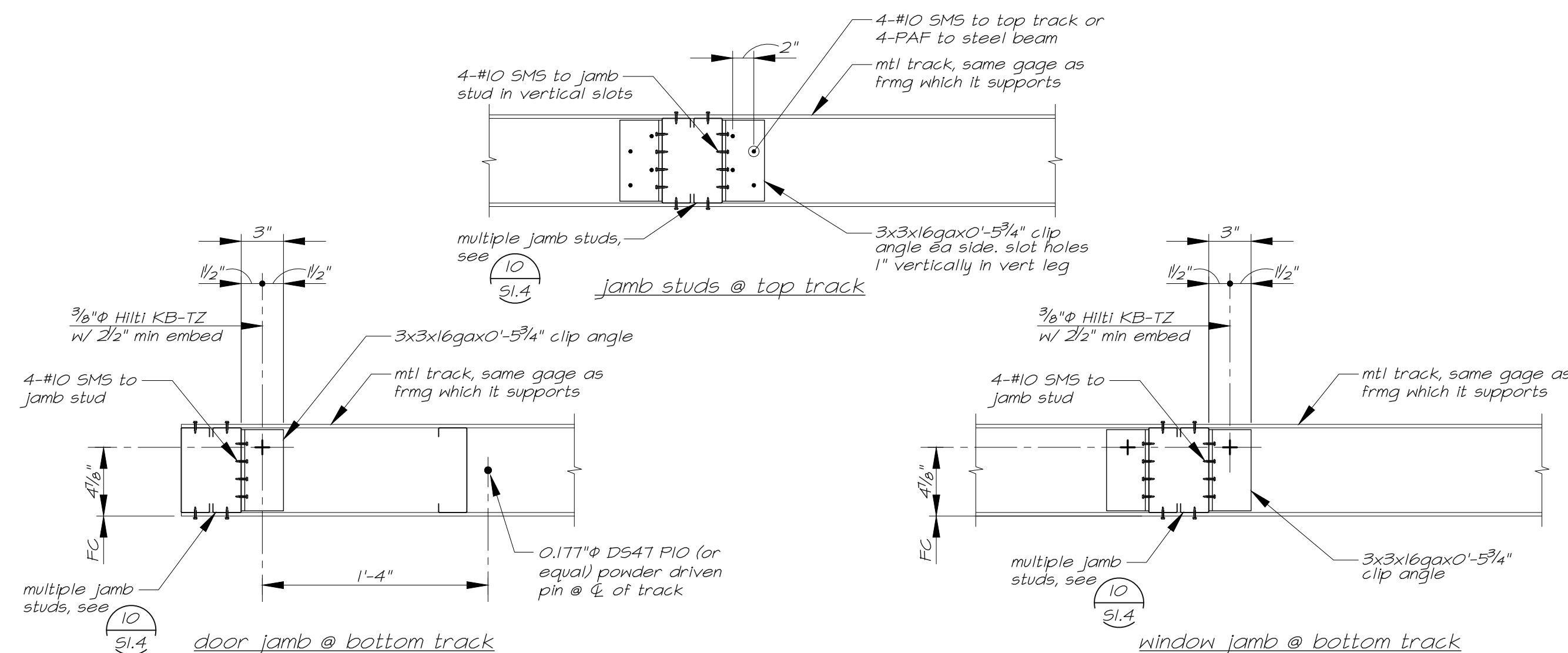
10 Typ Multiple Stud Connection



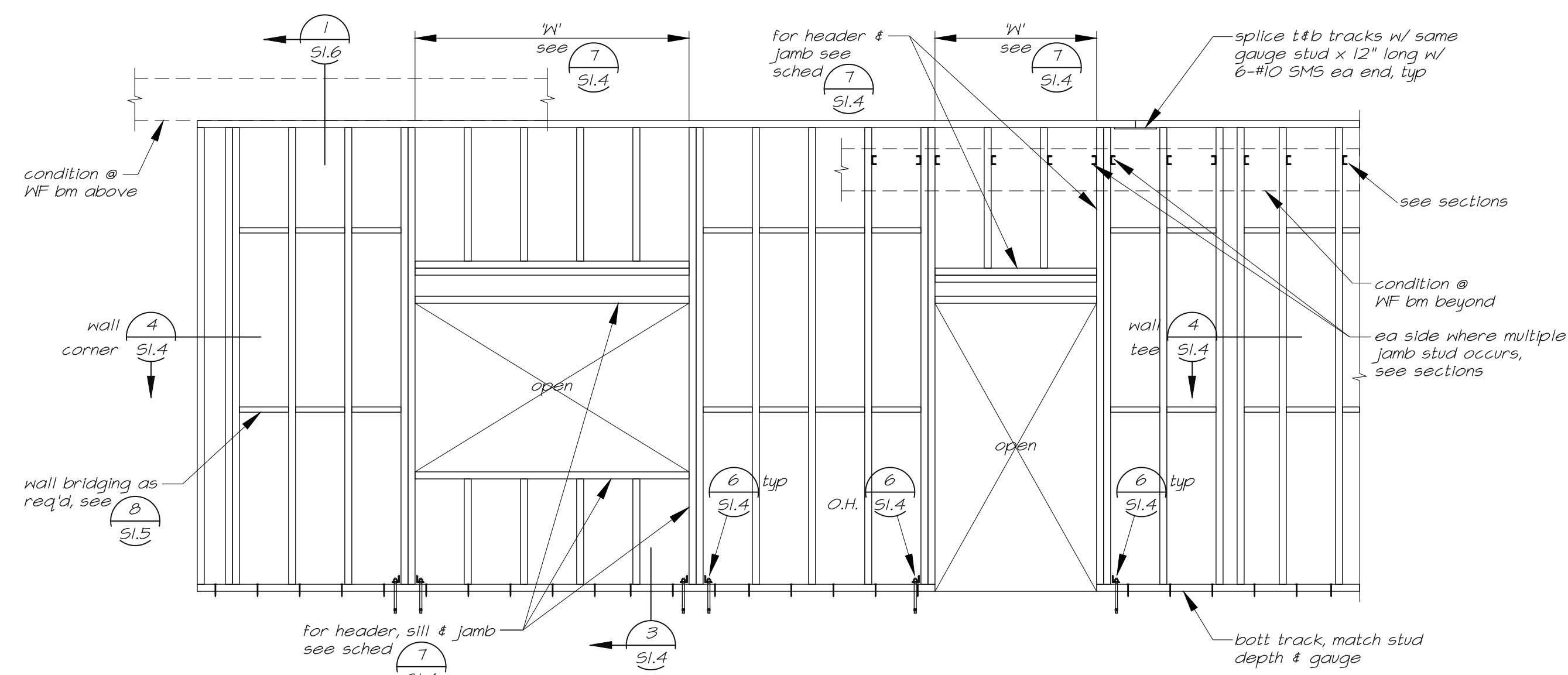
4 Typical Wall Framing Plans



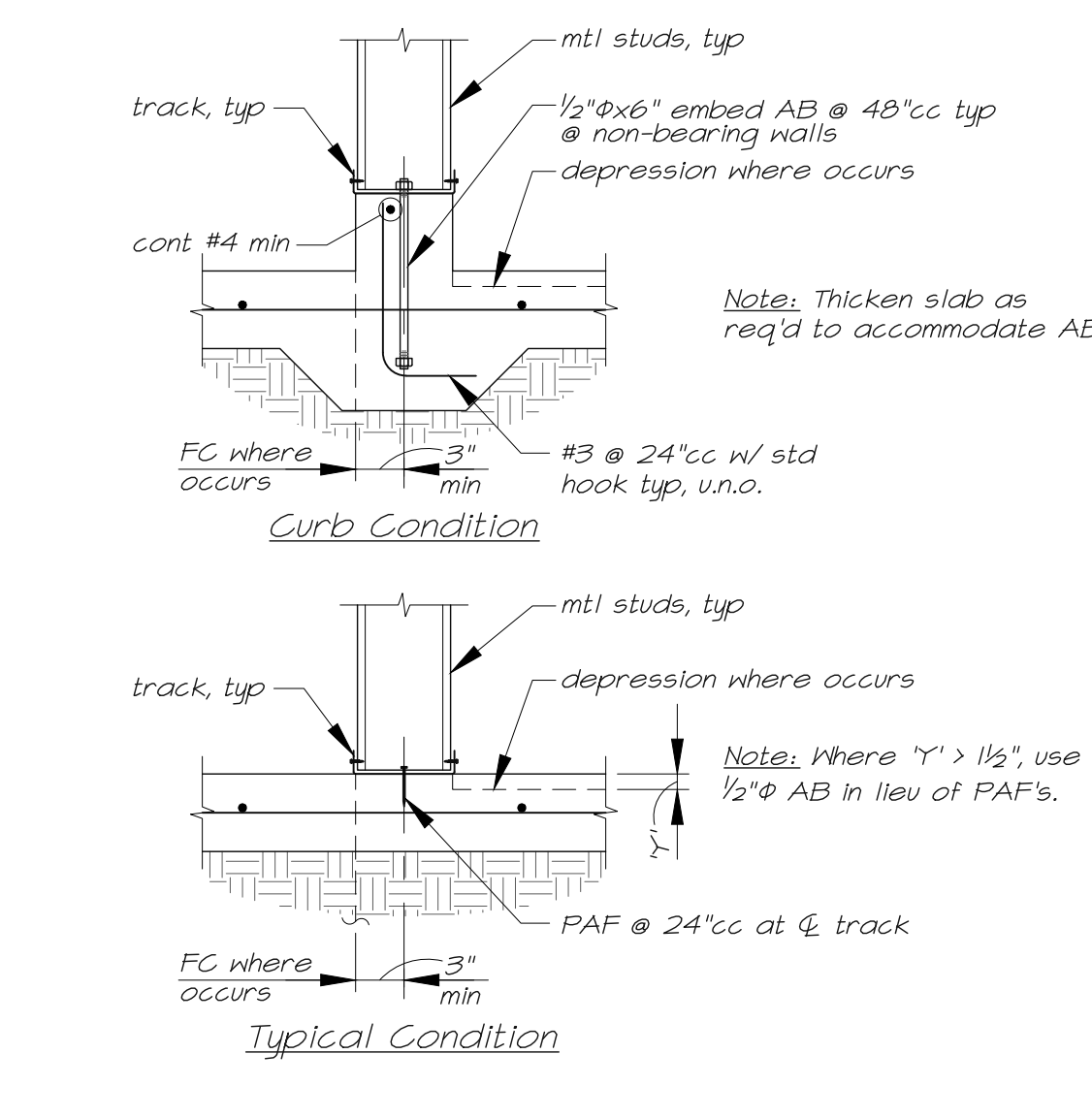
5 Typical Stud to Track Connection



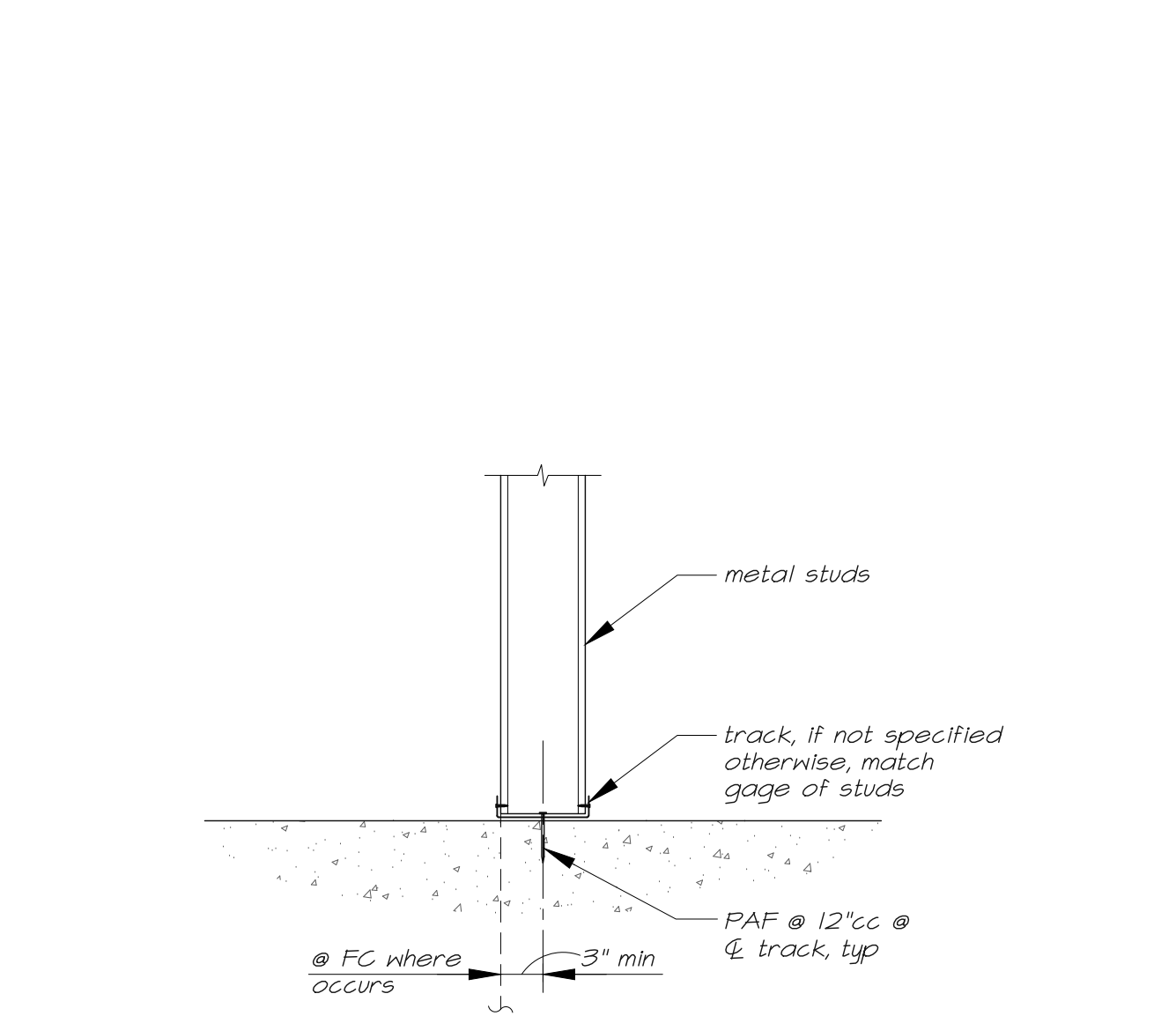
6 Typical Plan Details



1 Typical Exterior Wall Framing Elevation



2 Typ Sill Anchorage to Conc



3 Typ Anchorage to Concrete



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
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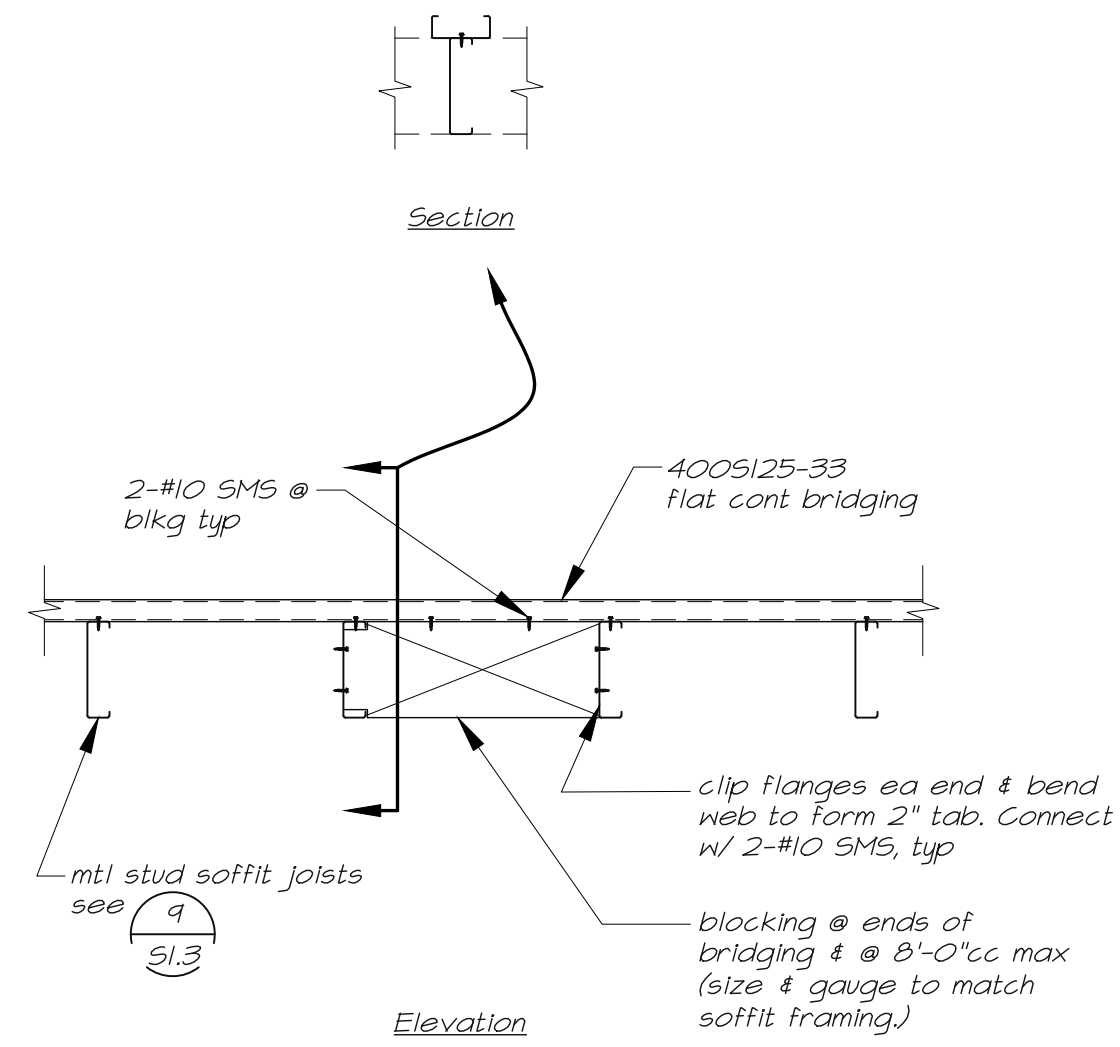
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TYPICAL DETAILS

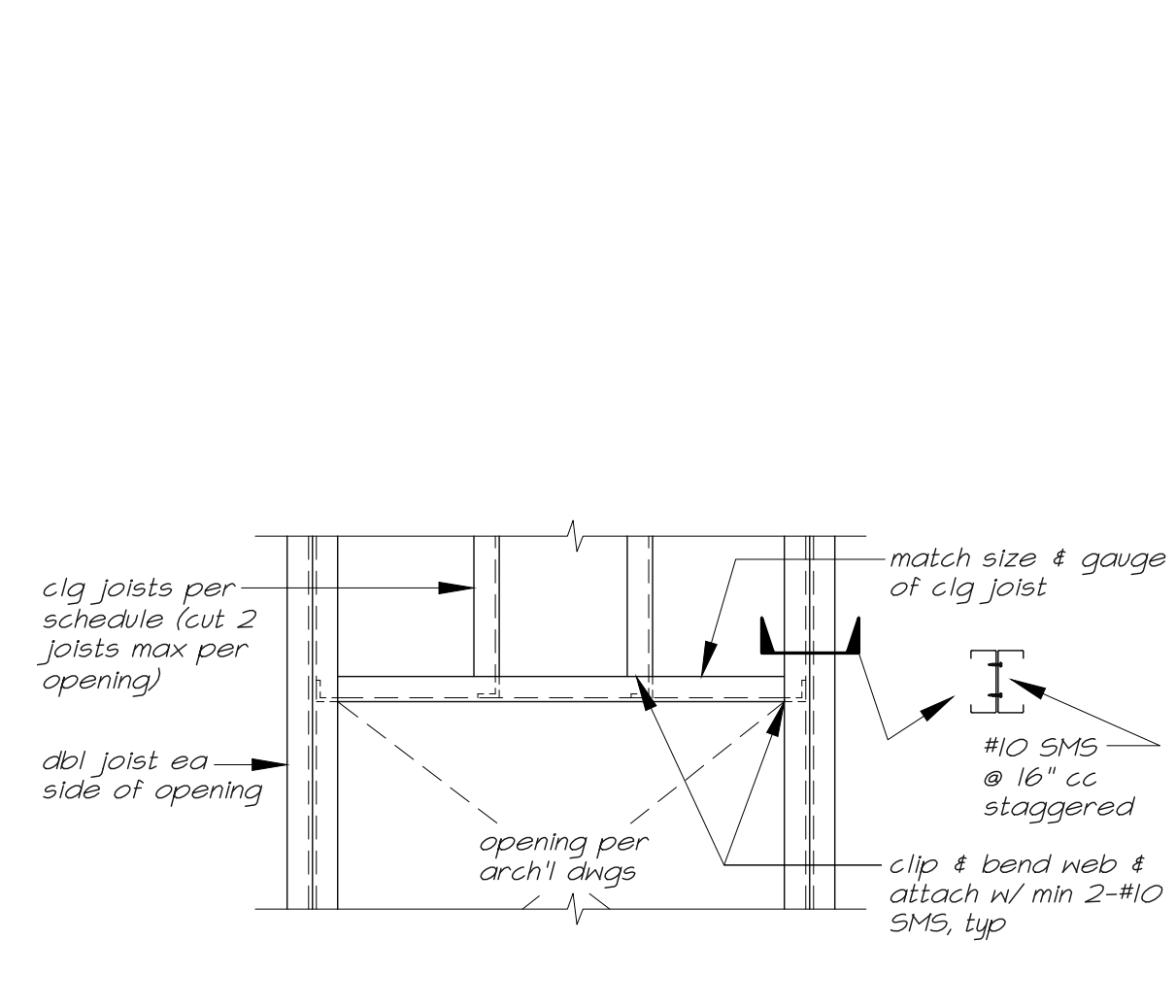




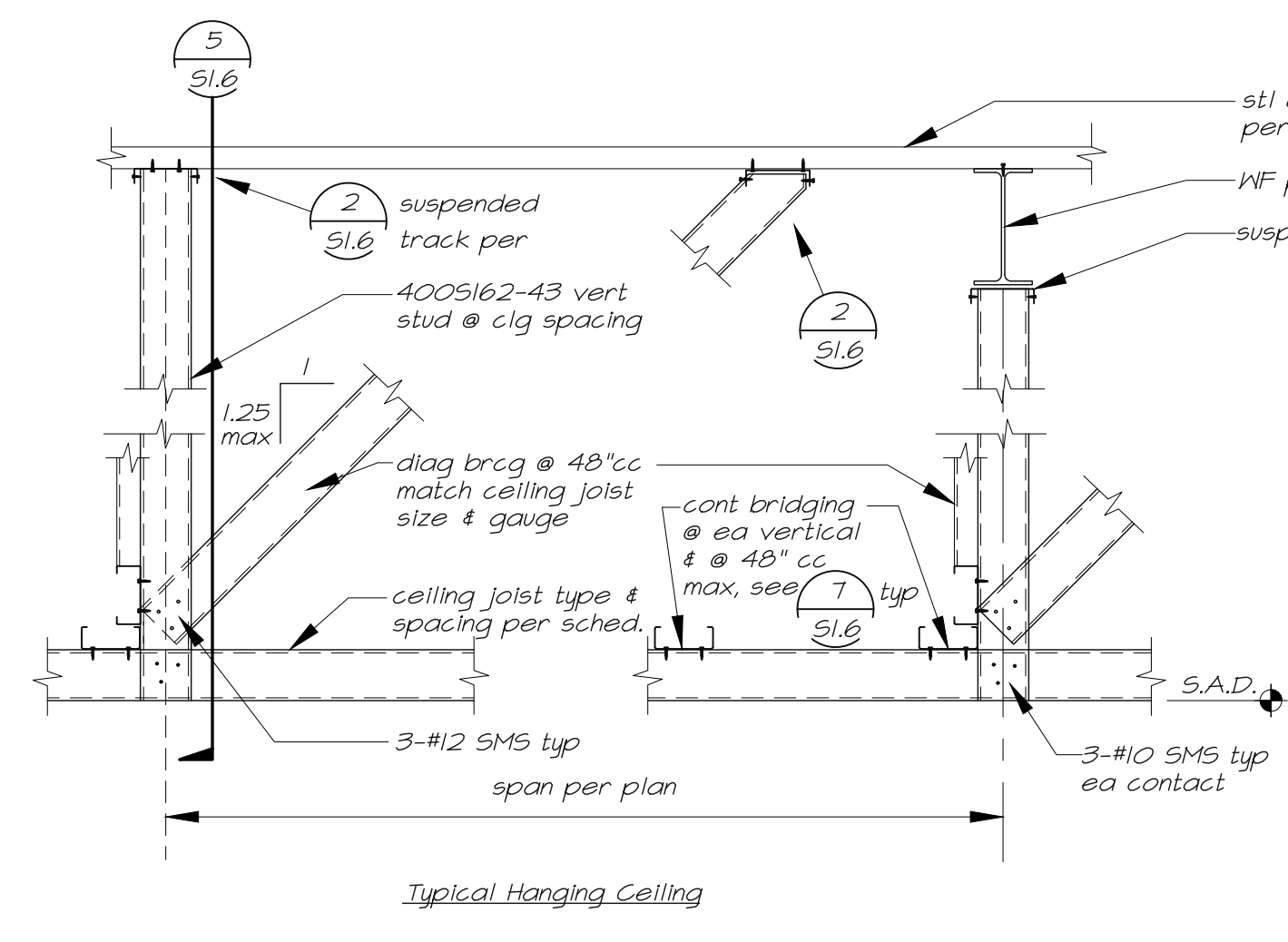




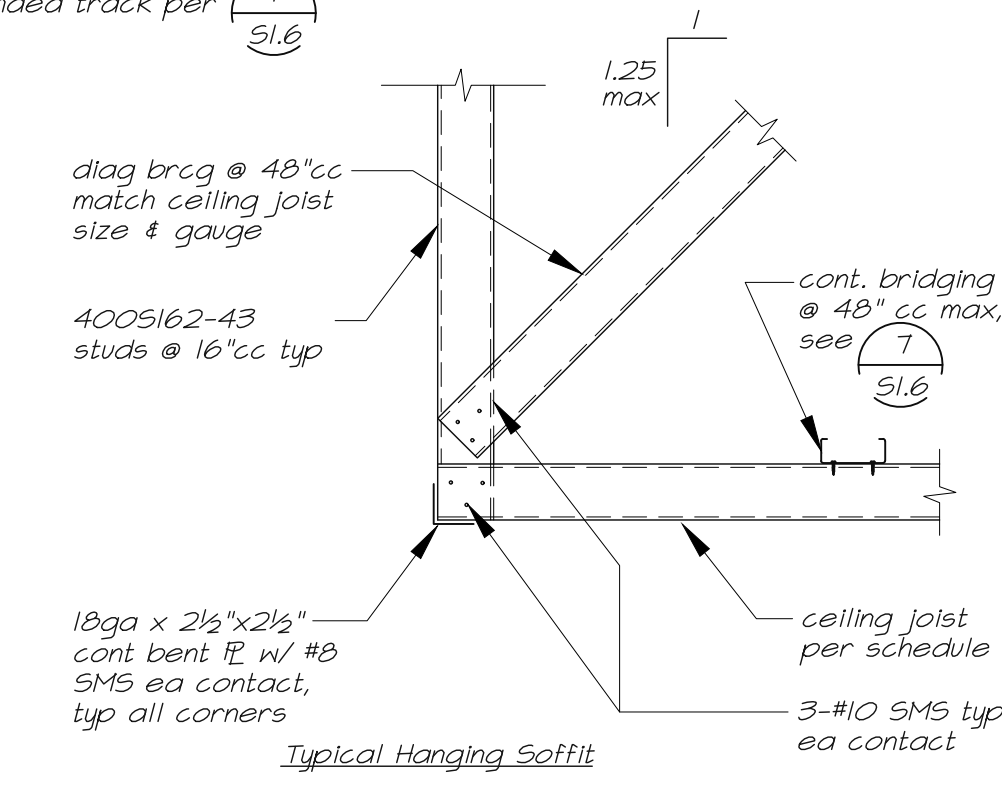
7  
S1.6  
1"=1'-0" Typ Soffit Joist Bridging



8  
S1.6  
Typical Opening in Ceiling

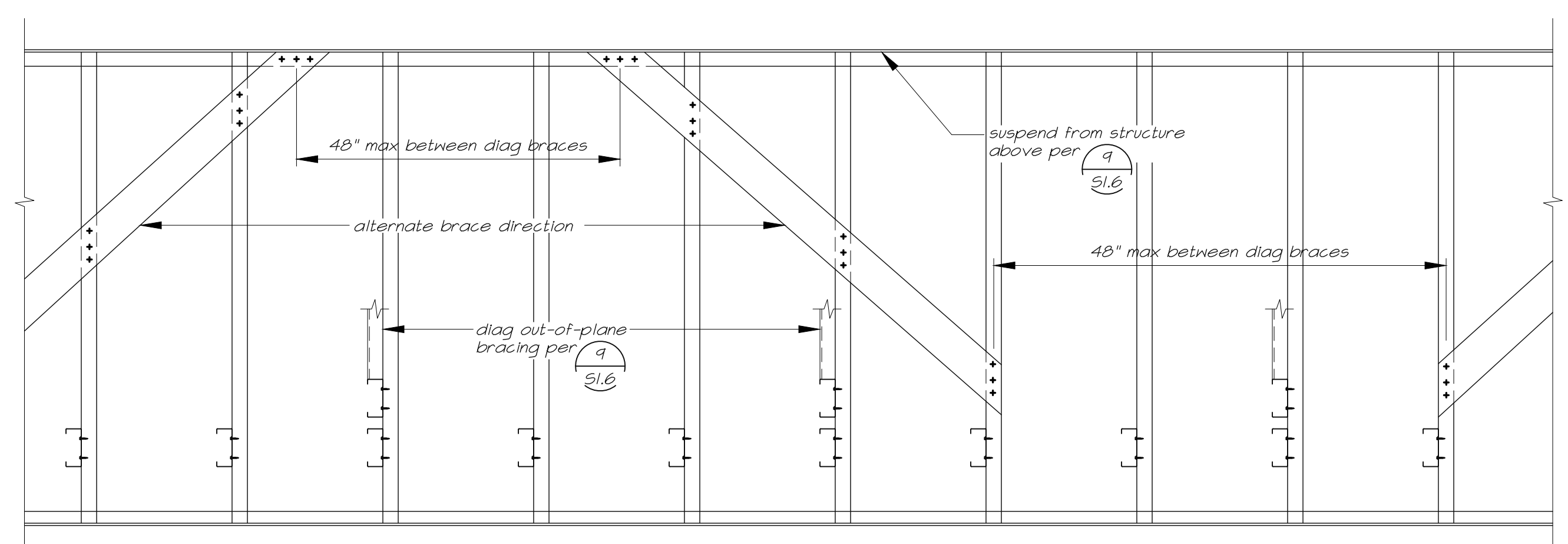


9  
S1.6  
1"=1'-0" Note: S.A.D. for rated ceiling details

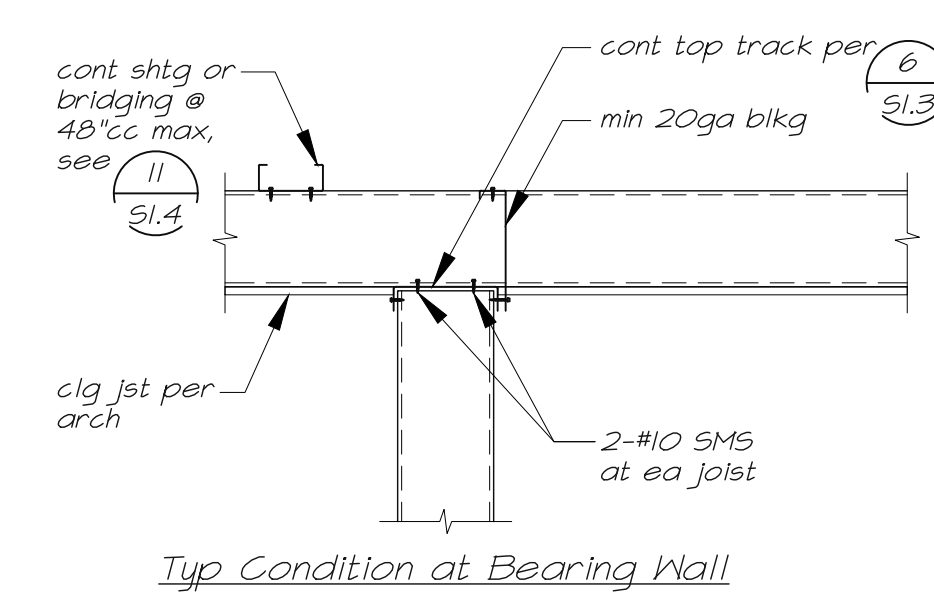


Span	Ceiling Joist
5'-0" max	400S125-33 @ 16"cc
8'-0" max	400S137-33 @ 16"cc
10'-0" max	400S137-43 @ 16"cc
12'-0" max	600S137-43 @ 24"cc
14'-0" max	600S137-43 @ 16"cc

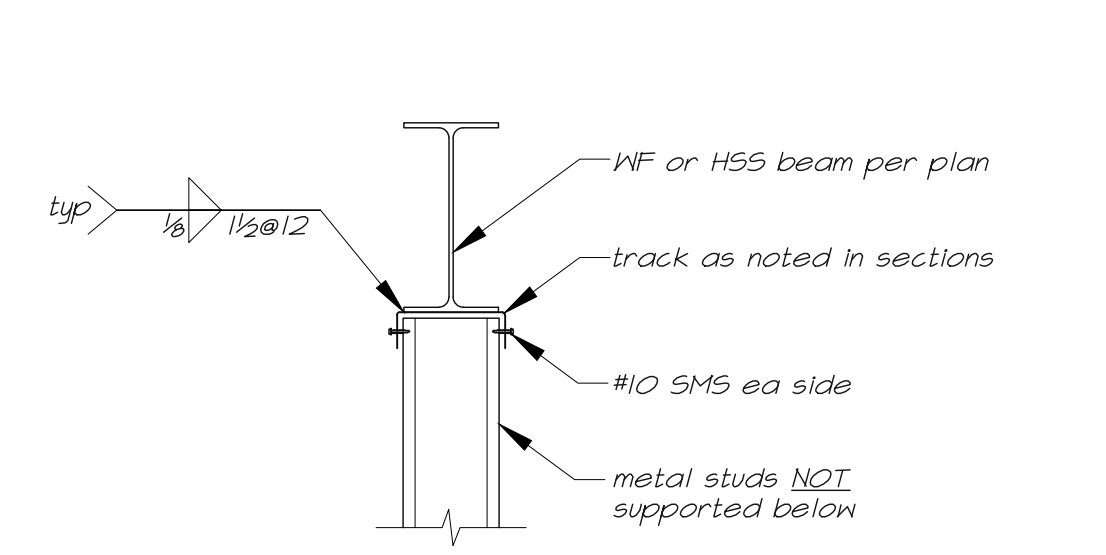
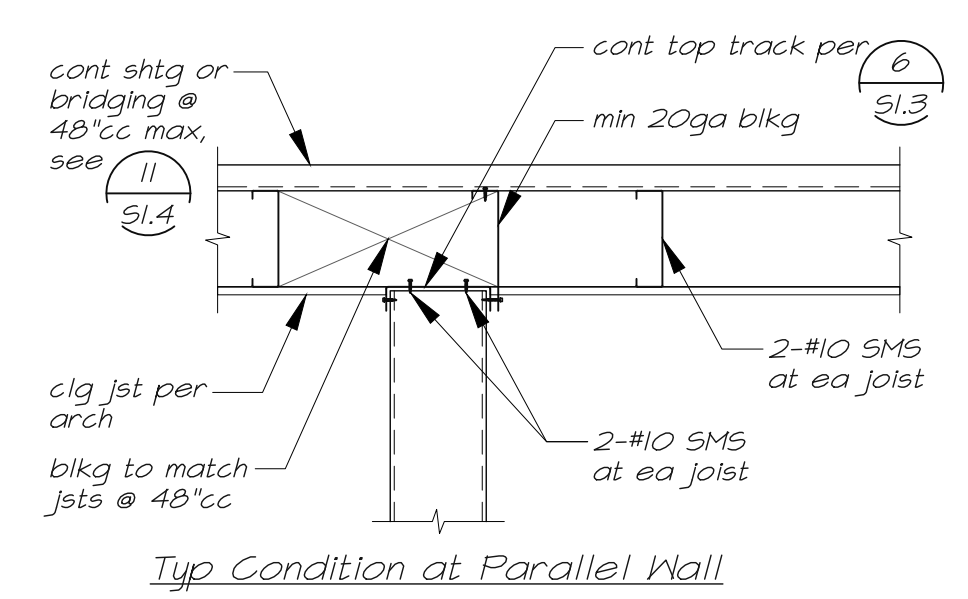
NOTE:  
1. Provide bridging @ 48" cc, see 7  
2. Maximum load on ceiling joists shall be 15 psf DL + 10 psf LL



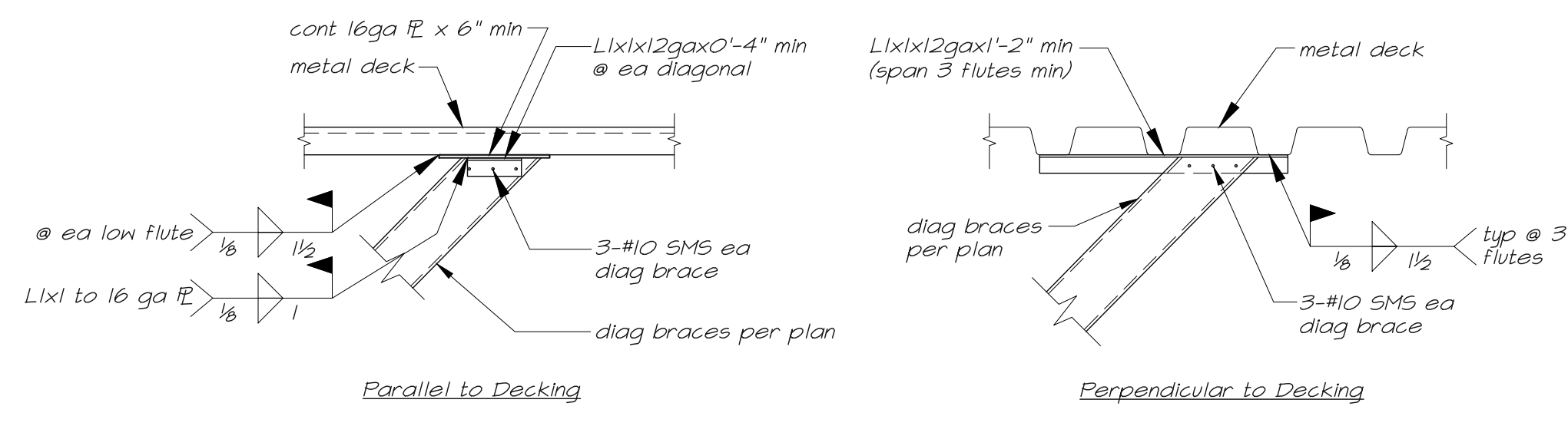
5  
S1.6  
Typical Diagonal In-Plane Bracing @ Suspended Studs



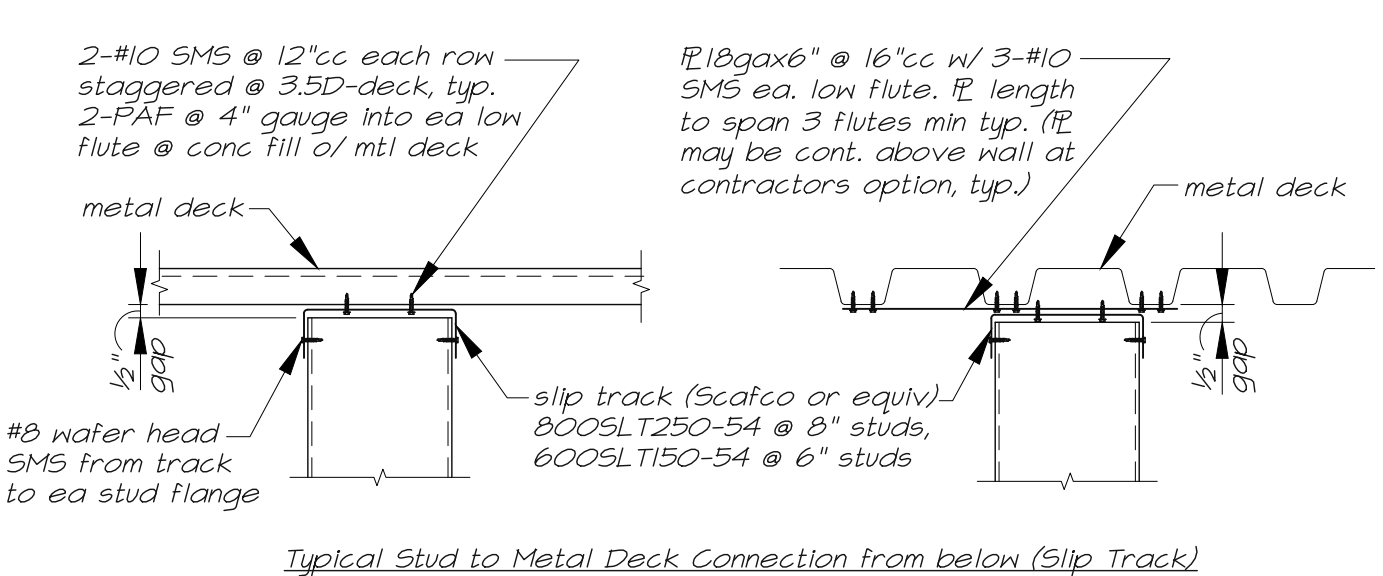
6  
S1.6  
Typical Joists Over Bearing Walls



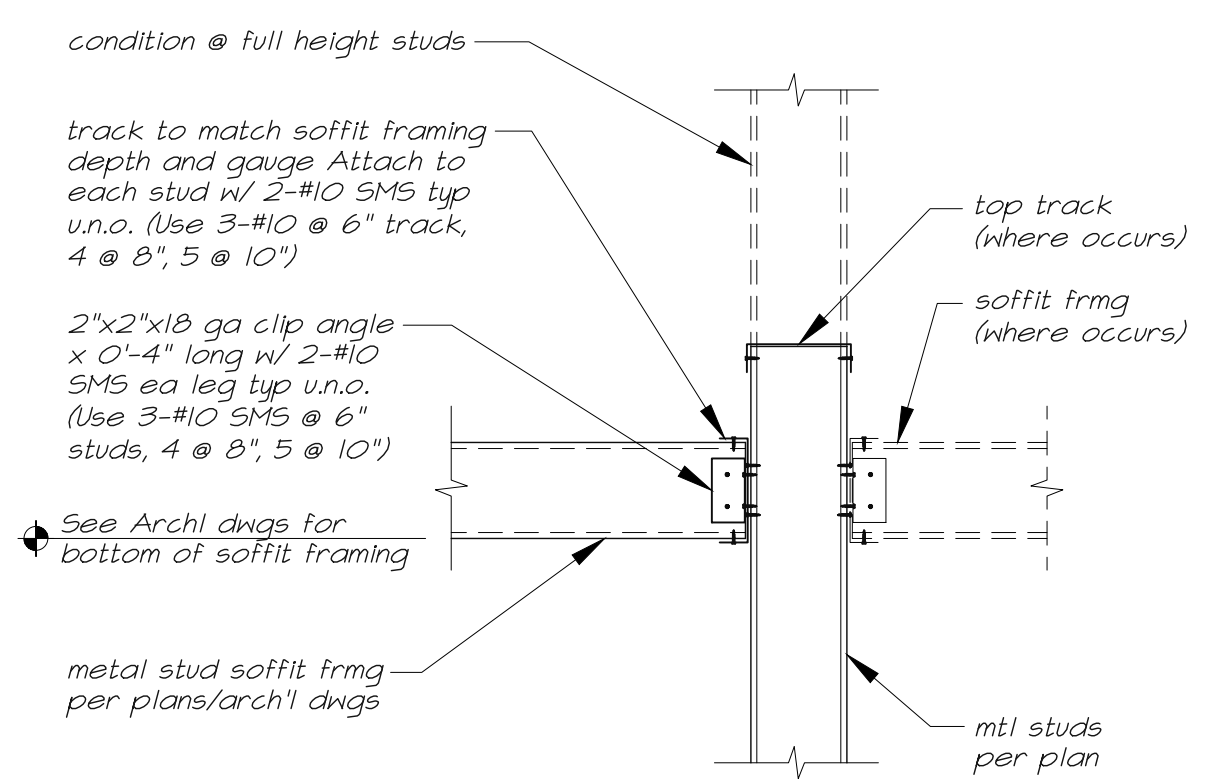
1  
S1.6  
Detail  
stud to beam



2  
S1.6  
Detail  
1"=1'-0"



3  
S1.6  
Detail  
1 1/2"=1'-0" Intr condition



4  
S1.6  
Typical Joist to Stud



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
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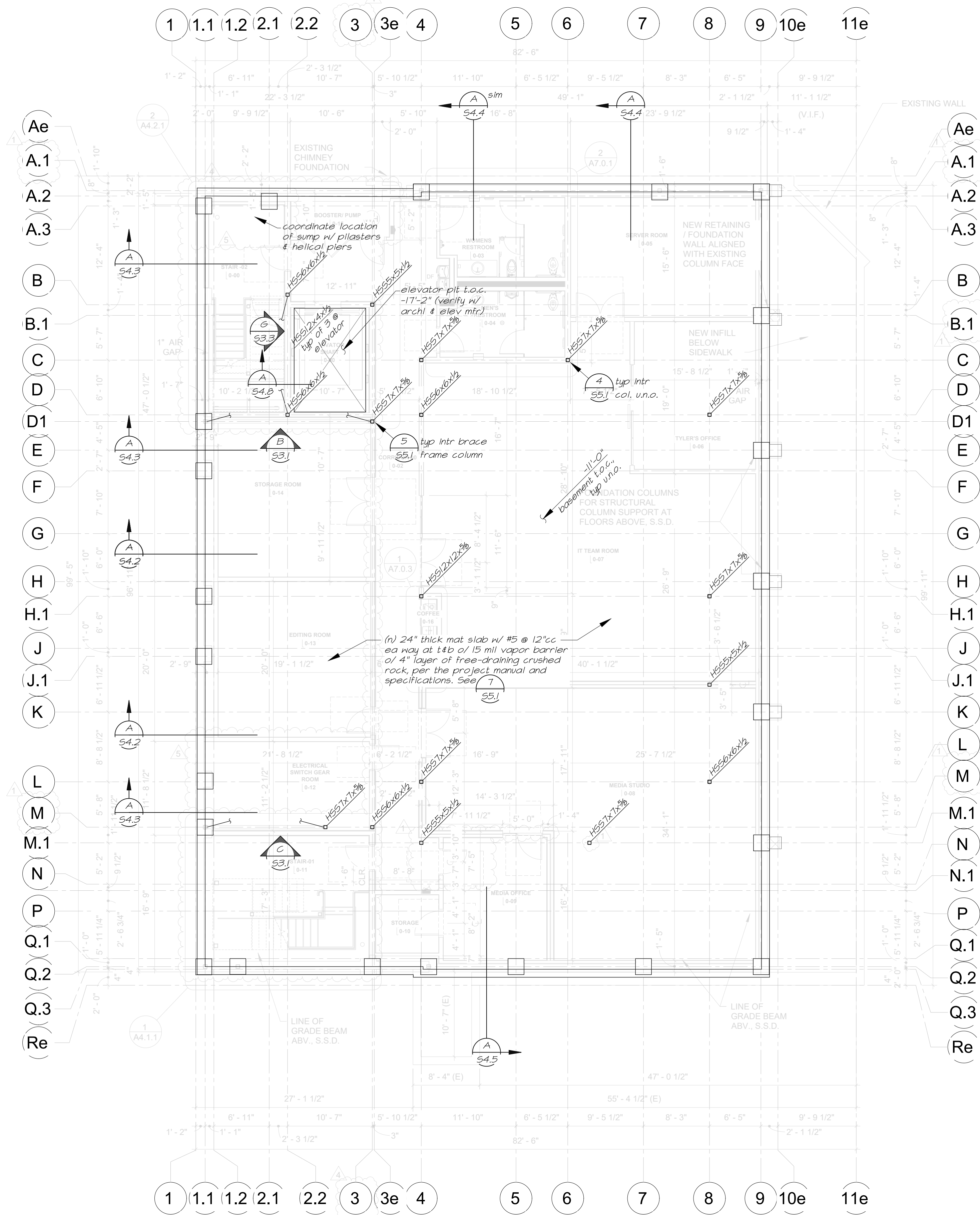
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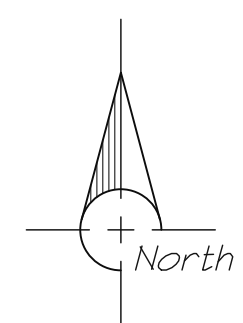


**Foundation Notes**

1. Site preparation and building pad construction shall be done in accordance with the recommendations in the Geotechnical Engineering Report by Mid Pacific Engineering, Inc. (MPE), MPE No. 0635T-01, dated April 18, 2024.
2. Verify all dimensions with architectural drawings. Notify Architect immediately of any discrepancies for resolution prior to proceeding.
3. Dimensions are to face of concrete (FC) or column centerlines, typical u.n.o.
4. Spread Footings are centered on columns and braced frames, typical u.n.o.
5. Basement top of concrete slab = elevation -11'-0" with respect to reference first floor t.o.c. elevation +0'-0".
6. HSS column base t.o.c. elevation = -11'-10" below reference first floor elevation +0'-0" typical u.n.o.
7. Slab block-outs shown at columns are diagrammatic. Actual size and configuration is to be determined by the contractor for constructability.
8. Provide 3" concrete cover minimum @ base of anchor bolts, and columns typical.
9. For Typical Framing details at exterior metal stud walls, see sheet S1.4.
10. For typical reinforcing bends and corner reinforcing, see  $\frac{1}{S1.3}$   $\frac{2}{S1.3}$
11.  $\frac{1}{S1.3}$   $\frac{2}{S1.3}$  Indicates top of concrete elevation below reference elevation +0'-0"
12.  $\square$  Indicates HSS column, size indicated on plan.
13.  $\triangle$  Indicates braced frame location. See elevations and details referenced.



**Basement/Foundation Plan**  
 $\frac{1}{8}'' = 1'-0''$



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 Stockton, California 95207  
 P: (209) 952-5850  
 F: (209) 952-2442  
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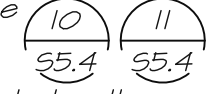

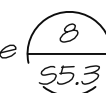




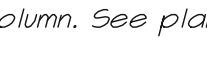
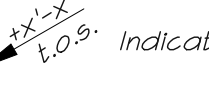
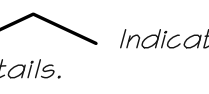

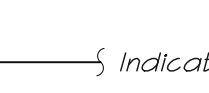

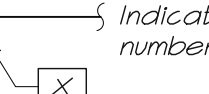
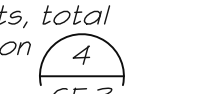

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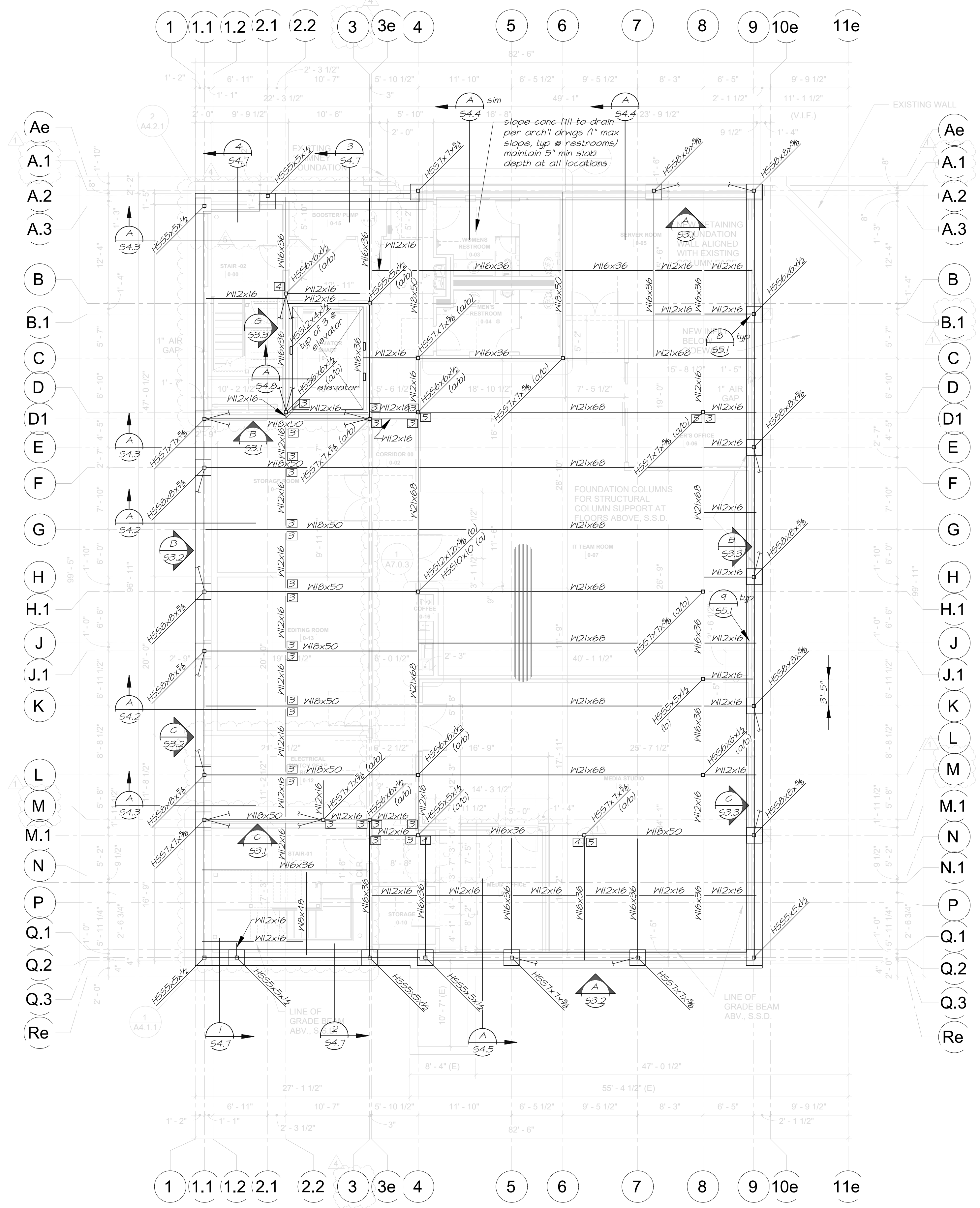
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**BASEMENT FOUNDATION PLAN**



**Floor Framing Notes**

- 3rd Floor: Top of slab elevation is +28'-0" above reference elevation +0'-0", typical u.n.o. Top of steel elevation is +27'-5 1/2" above reference elevation typical u.n.o.
- 2nd Floor: Top of slab elevation is +16'-0" above reference elevation +0'-0", typical u.n.o. Top of steel elevation is +15'-5 1/2" above reference elevation typical u.n.o.
- 1st Floor: Top of slab elevation is +0'-0" above reference elevation +0'-0", typical u.n.o. Top of steel elevation is -0'-6 1/2" with respect to reference elevation typical u.n.o.
- Dimensions are to centerline of steel or face of closure/angle at slab edge typical u.n.o.
- Verify all openings in floor with Architectural, Mechanical, and Electrical drawings. For openings at concrete fill over metal deck, see  .
- For typical framing details at exterior metal stud walls, see sheet S1.4.
- C=3/4" indicates amount of camber required at mid-span of beam or girder.
- (34) Indicates number of automatic end welded studs required at beam or girder see .
- For typical beam to beam connections, see  .
- For typical beam to column connections, see .
- Metal deck must be attached to all steel beams. At locations where low flutes do not align with beam, split deck as in .
-  Indicates HSS column. See plans for size.
-  Indicates top of steel elevation above reference elevation +0'-0".
-  Indicates braced frame location. See braced frame elevations and referenced details.
-  Indicates 3" normal weight concrete fill of metal deck. Span direction of metal deck as indicated on plan. Steel deck shall be 2 spans min u.n.o. For metal deck types and typ details, see note 18 for slab reinforcing.
-  Indicates moment connection at end of beam, see .
-  Indicates beam connection requiring A325 SC Class A bolts, total number of bolts required is shown inside box, see note #1 on .
- Exterior walls shall be 8005162-43 metal studs @ 16"cc, u.n.o. Interior metal studs shall be per architectural drawings.
- Install compressible material around braced frame gusset plates per details.
- All concrete fill of metal deck shall be reinforced w/ 6x6-W2.9 sheets of WNF (rolls not acceptable) at mid-depth of concrete topping, typ.
- "FDST" indicates full depth shear tabs per .



**First Floor Framing Plan**  
  
 1/8"=1'-0"



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**FIRST FLOOR FRAMING PLAN**

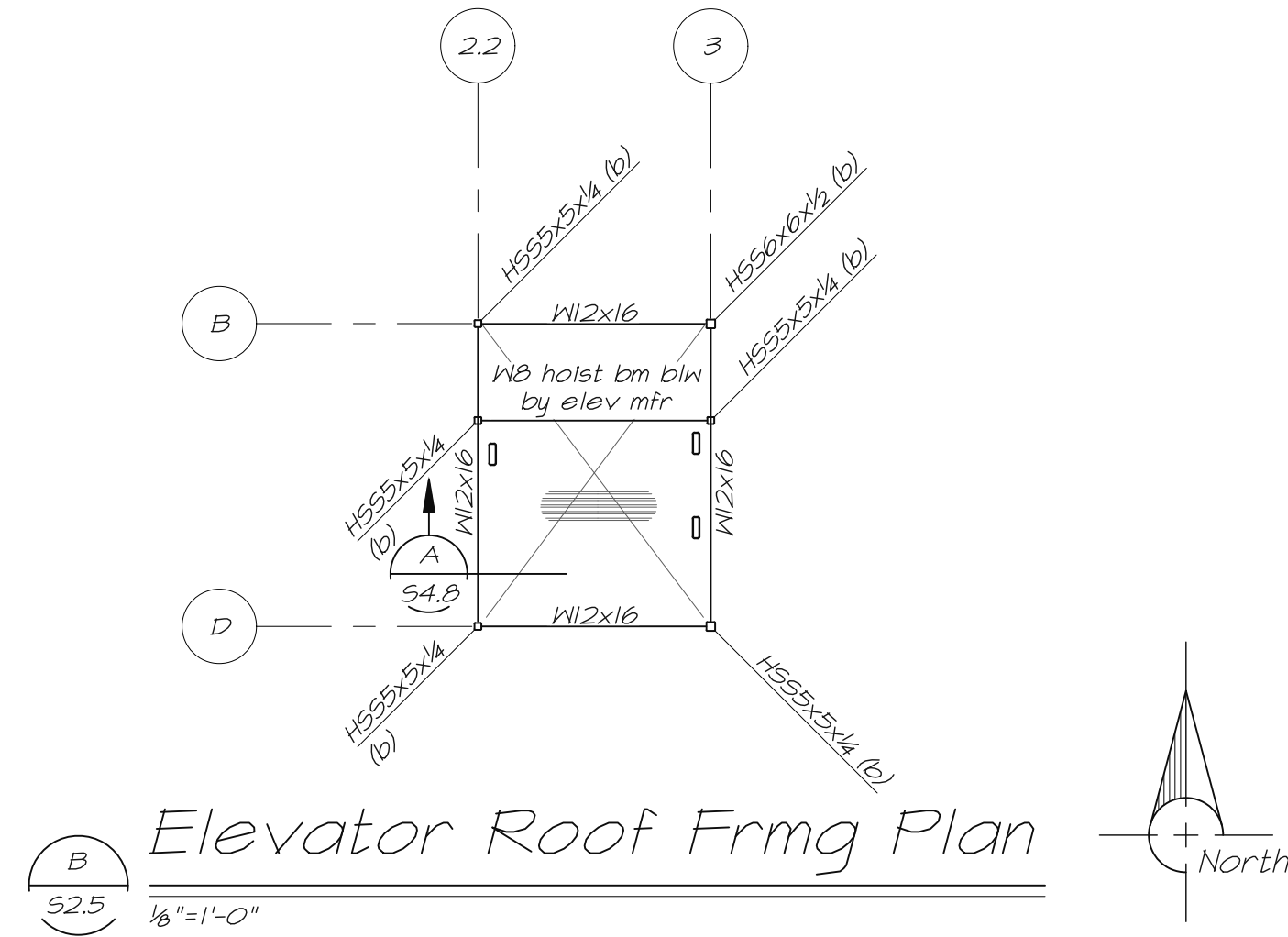






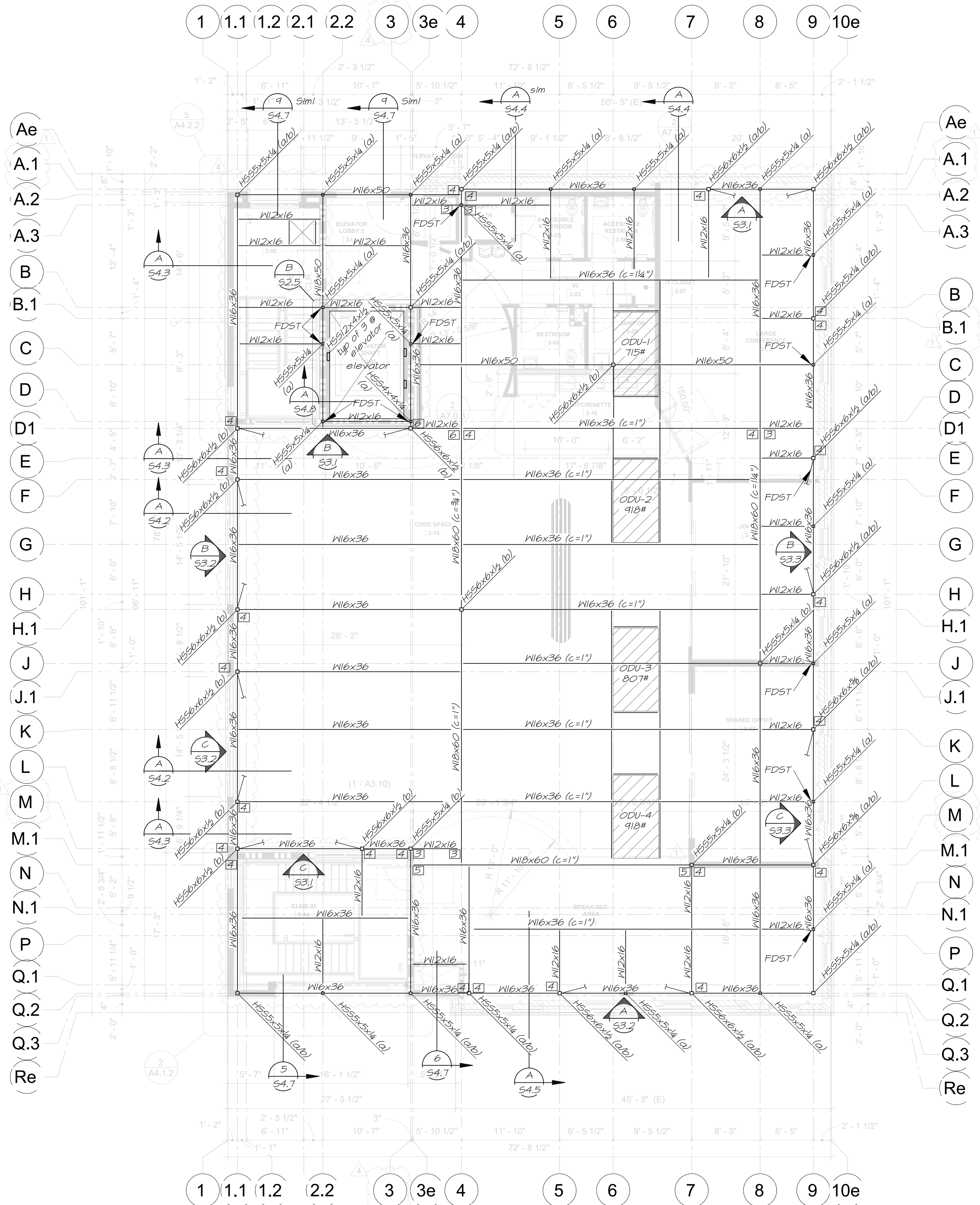






**Roof Framing Notes**

- Dimensions are to centerline of steel or edge of decking typical u.n.o.
- Verify all openings in roof with Architectural, Mechanical and Electrical drawings. For openings at metal deck, see (8) (9) (55.5) (55.5)
- For typical framing details at exterior metal stud walls, see sheet S1.4.
- C=3/4" indicates amount of camber required at mid-span of beam or girder.
- For typical beam to beam connections, see (2) (3) (55.3) (55.3)
- For typical beam to column connections, see (1) (55.3)
- Metal deck must be attached to all steel beams. At locations where low flutes do not align with beam, split deck as in (6) (55.5)
- Indicates HSS column. See Roof Framing Plan for sizes.
- ↗ 12'-2" / 1'-0" Indicates top of steel elevation above reference elevation +0'-0"
- ∧ Indicates braced frame location. See braced frame elevations and details referenced.
- ≡≡≡ Indicates span direction of metal deck. For metal deck types and typical details, see (5) (55.4)
- ↔ Indicates moment connection at end of beam, see (5) (55.4)
- ↔ Indicates beam connection requiring A325 SC Class A bolts, total number of bolts required is shown inside box, see note #1 in (4) (55.3)
- XXX# Indicates approximate location & weight of mechanical unit. Coordinate location of unit and of beams supporting unit with mechanical drawings



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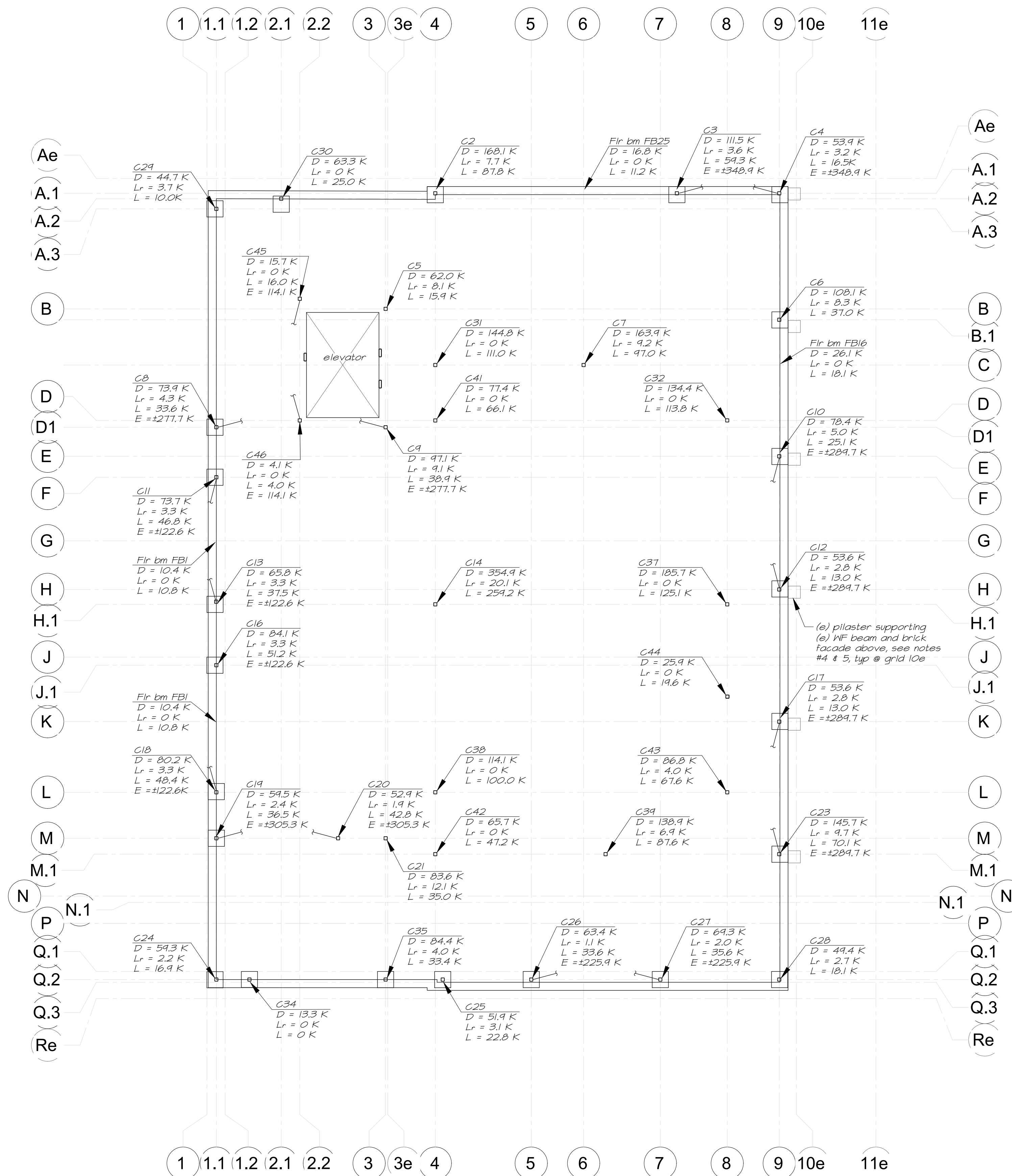
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**ROOF FRAMING PLAN**

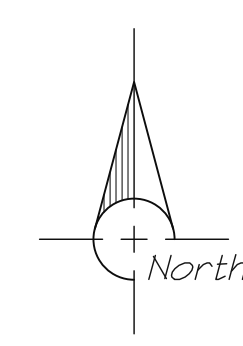




**ADDITIONAL LOADS AND NOTES:**

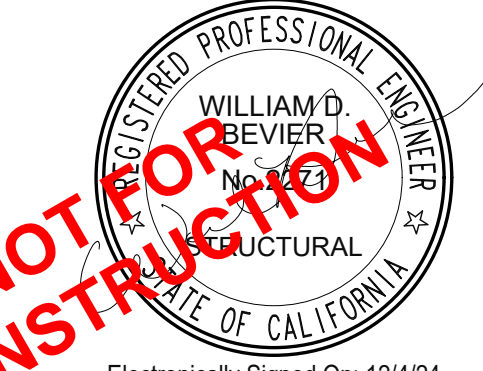
- Dead loads of (n) concrete pilasters (around perimeter), and (e) concrete pilasters at grid 10e are included in the loads noted on the plan.
- The following loads are additional and need to be included in the foundation design.
  - (n) 12" thick concrete retaining wall at basement at grid lines 1, 4, A, & Q
  - (n) 24" sq concrete grade beam at top of (n) retaining wall at grid lines 1, 4, A, & Q D=0.60 kif
  - (e) brick facade at grid lines Ae, Re, & 10e as noted on arch. drwgs. grids Ae&Re D=5.94 kif (grid line 10e supported on (e) beams/pilasters/concrete walls - loads NOT included in (n) column loads)
  - (e) concrete wall below (e) brick facade at grid lines A&Q (if 10e where occurs) D=1.575 kif (grid line 10e brick facade supported on (e) beams/pilasters/concrete walls - beam and brick facade wall loads NOT included in (n) column loads).
  - (e) footing below (e) conc walls D estimated =6.0 kif
  - Weight of (n) mat slab not included. Thickness = 24".
- Loads are unfactored:
  - D = dead load
  - L = floor live load
  - Lr = roof live load
  - E = seismic load

Col Load Summary at Foundation  
 1/8"=1'-0"



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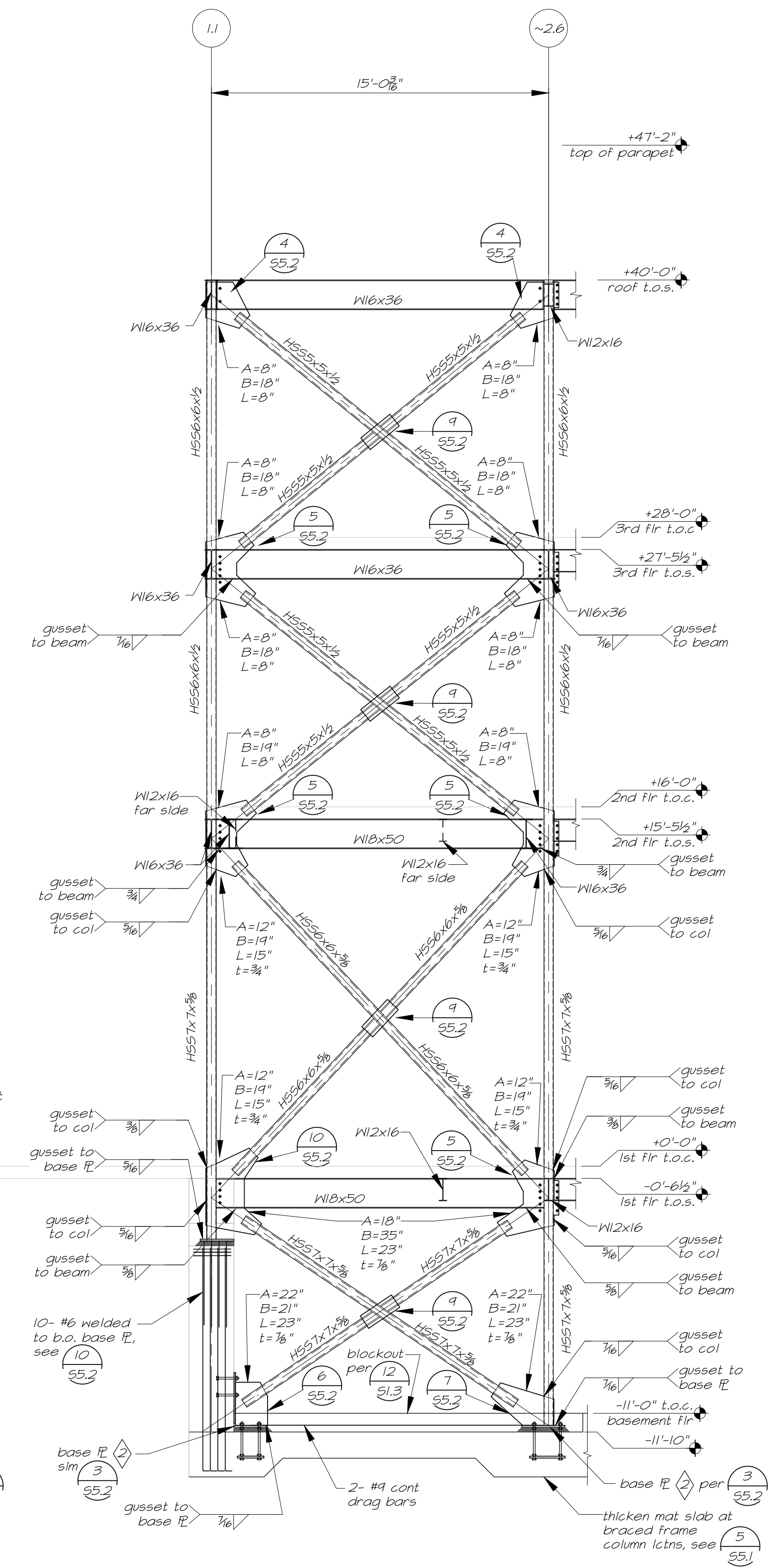
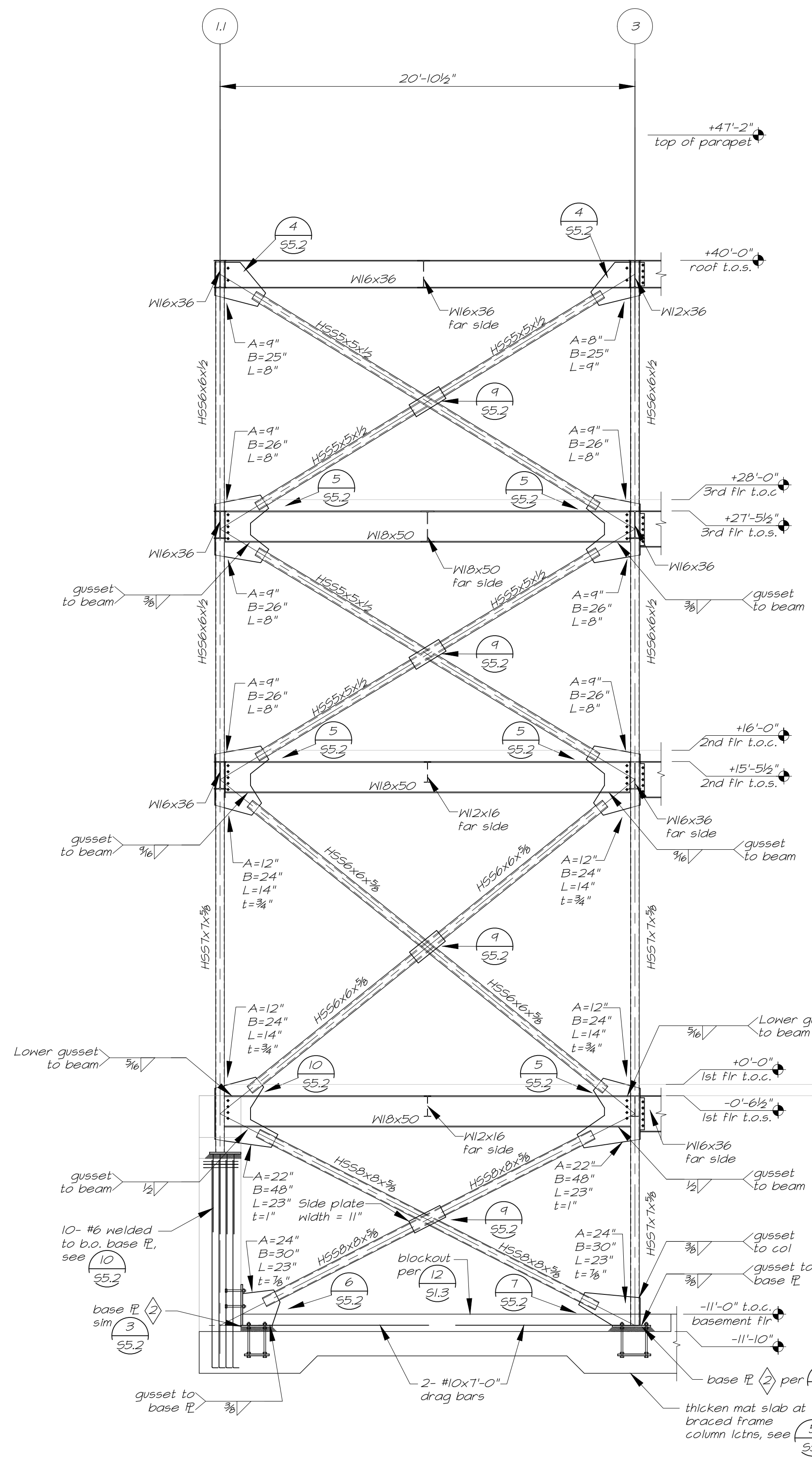
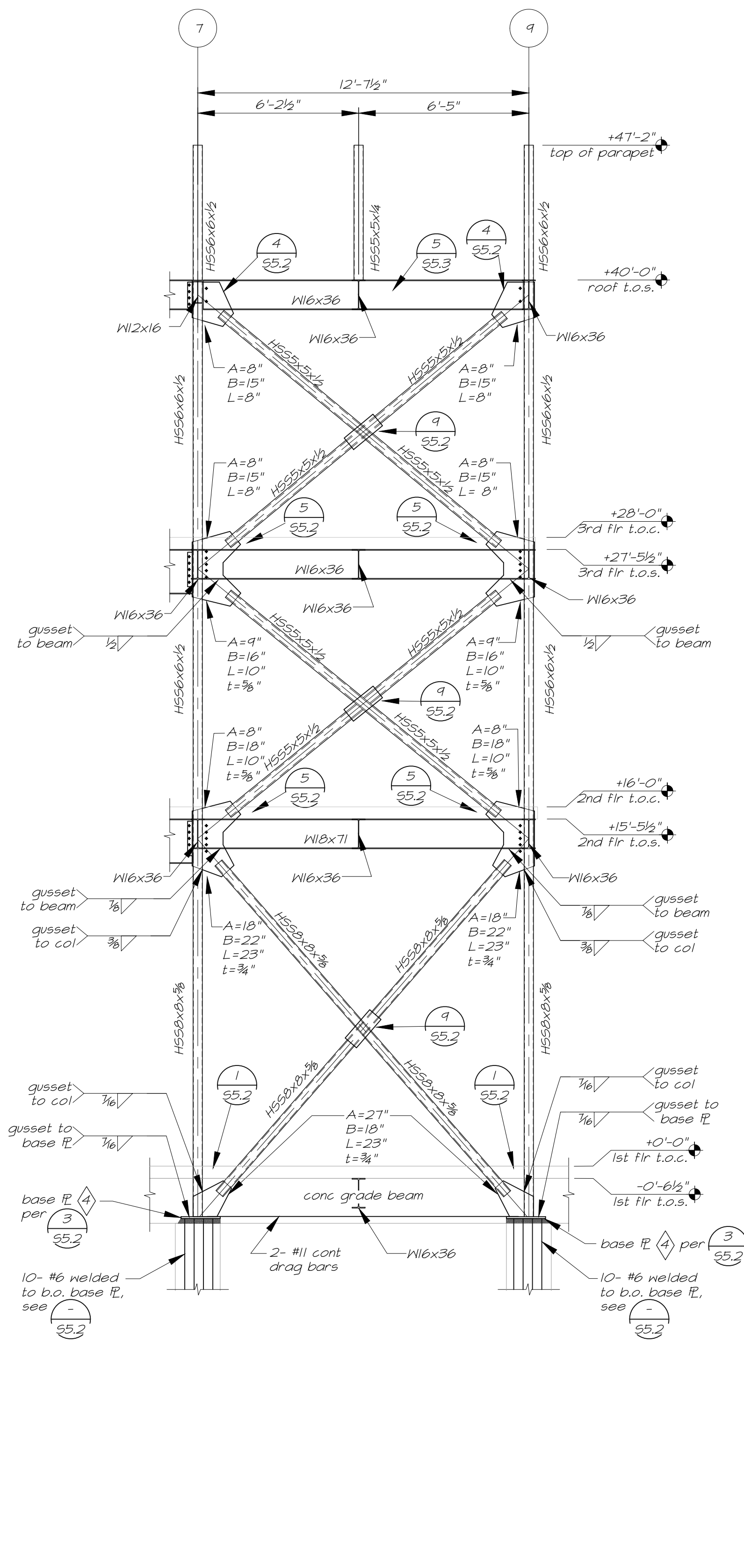
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**COLUMN LOAD SUMMARY AT FOUNDATION**



All welds shown in braced frame details on sheet 55.2 shall satisfy AISC 341-16 section A3.4 "Demand Critical Weld" requirements per AWS D1.8/D1.8M. See steel note #17 on sheet 51.2.



A Frame Line A.1 @ Grids 7-9  
53.1 1/4"=1'-0"

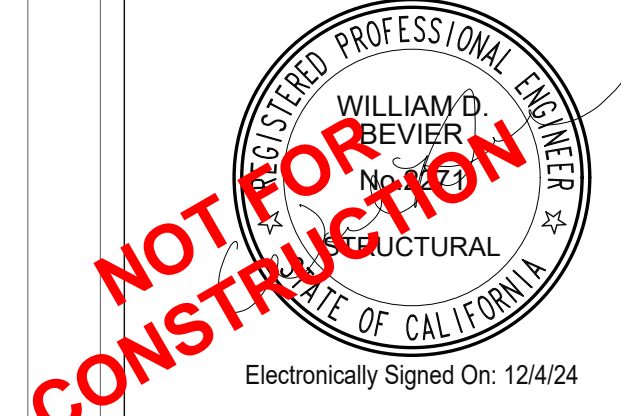
B Frame Line D @ Grids 1.1-3  
53.1 1/4"=1'-0"

C Frame Line M @ Grids 1.1-2.6  
53.1 1/4"=1'-0"



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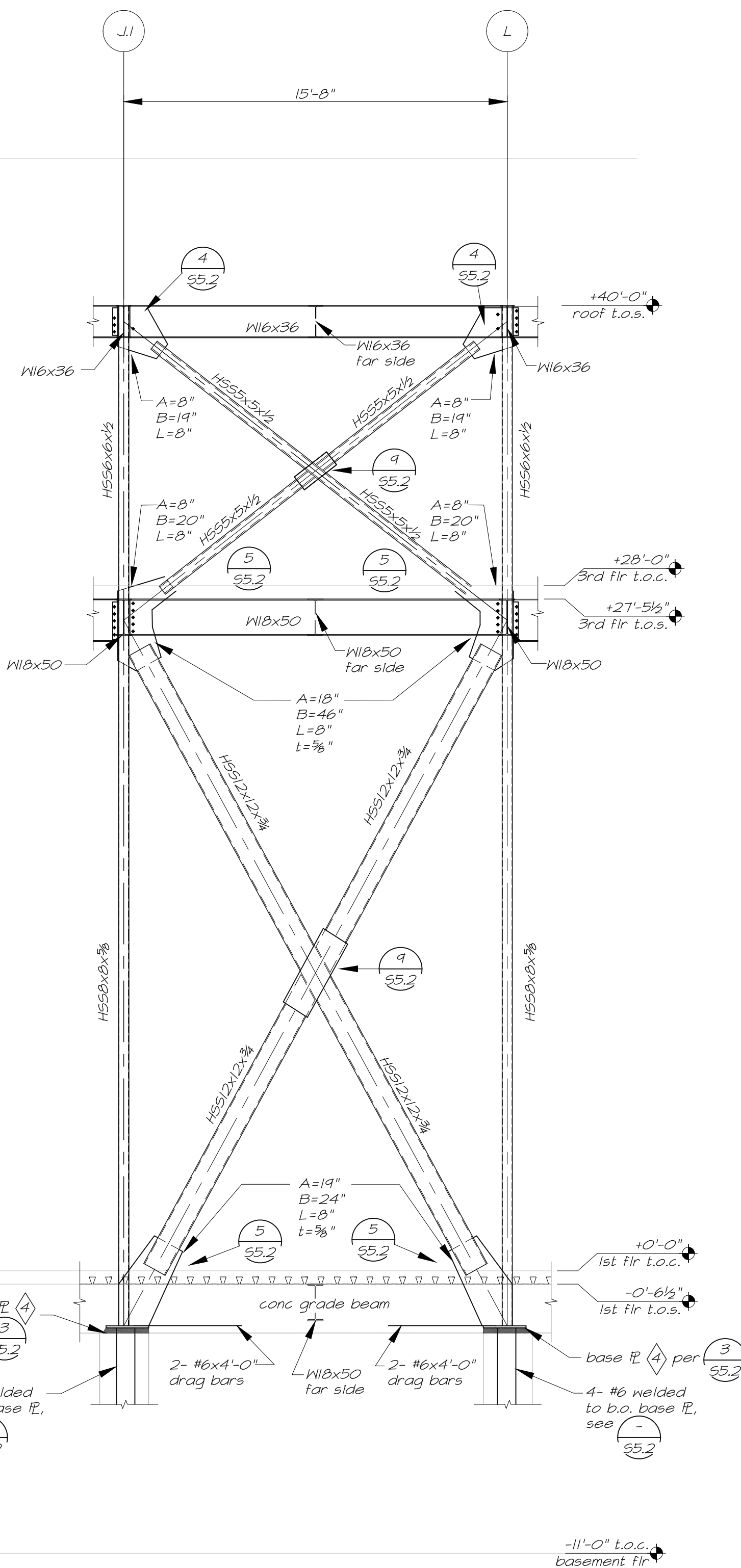
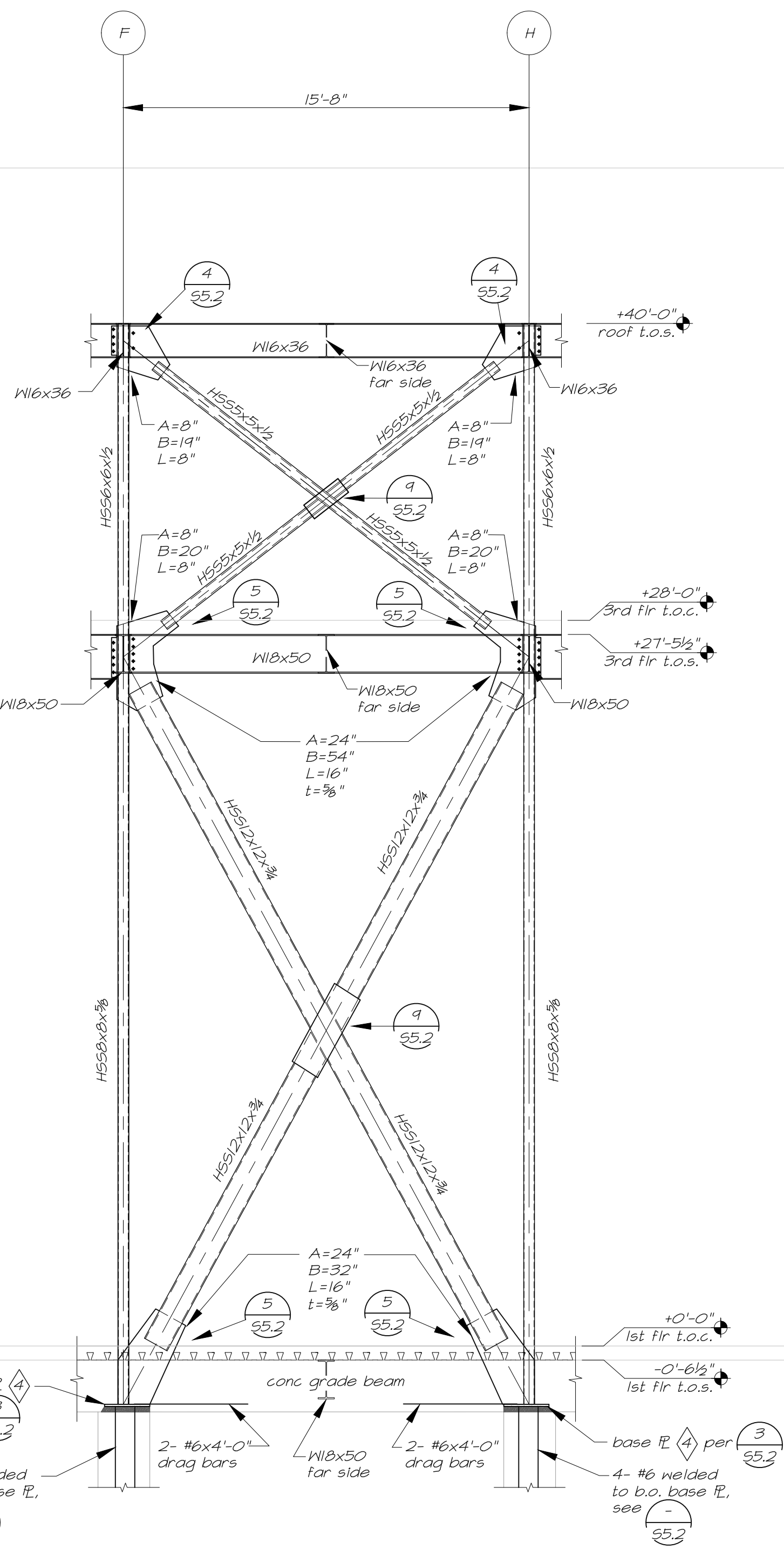
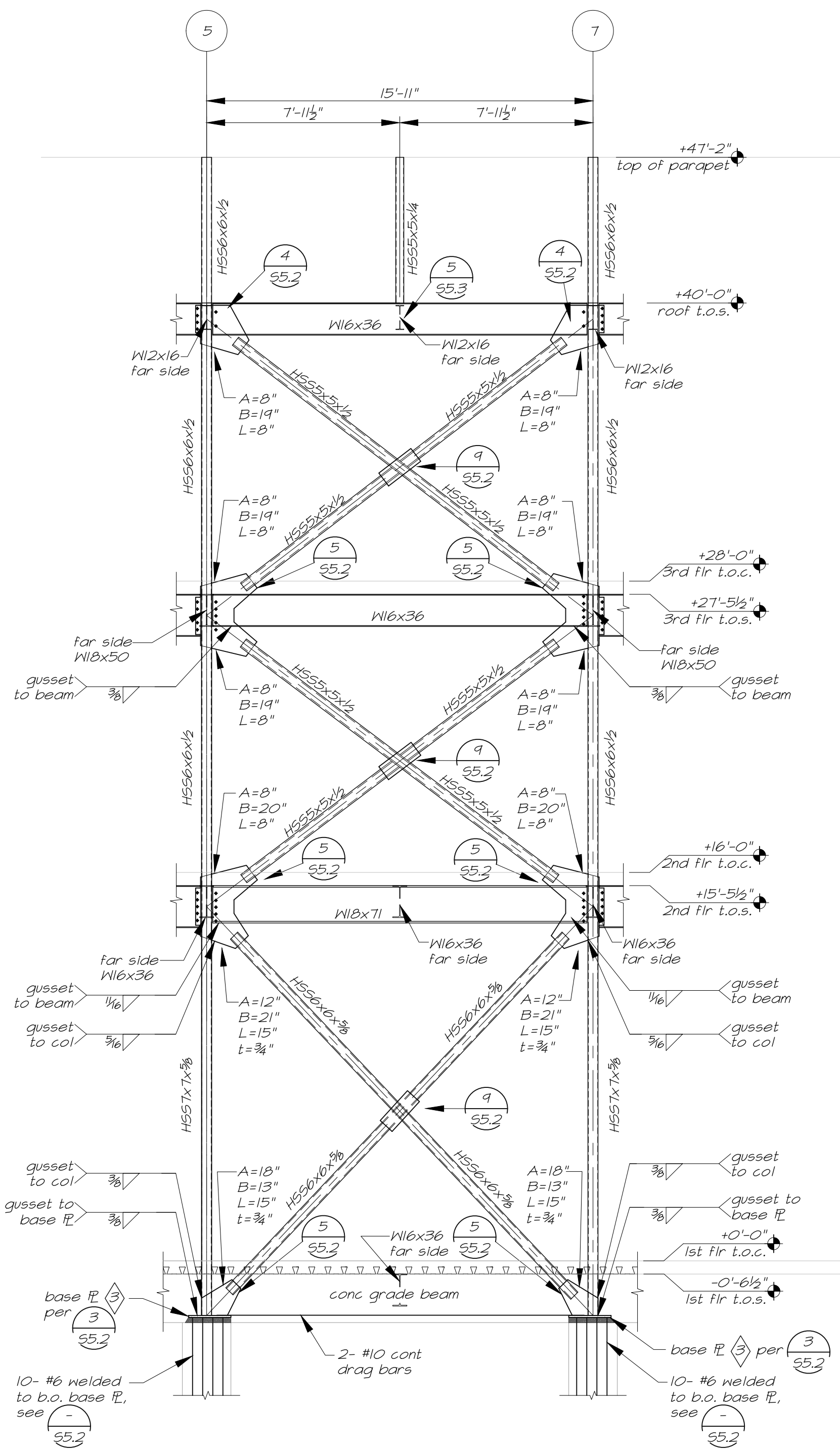
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FRAME ELEVATIONS



All welds shown in braced frame details on sheet S5.2 shall satisfy AISC 341-16 section A3.4 "Demand Critical Weld" requirements per AAS D1.8/D1.9M. See steel note #17 on sheet S1.2.



**A** Frame Line Q.3 @ Grids 5-7  
S3.2 1/4"=1'-0"

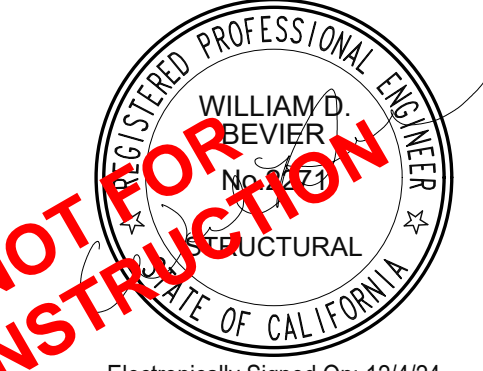
**B** Frame Line I.1 @ Grids F-H  
S3.2 1/4"=1'-0"

**C** Frame Line I.1 @ Grids J.I-L  
S3.2 1/4"=1'-0"



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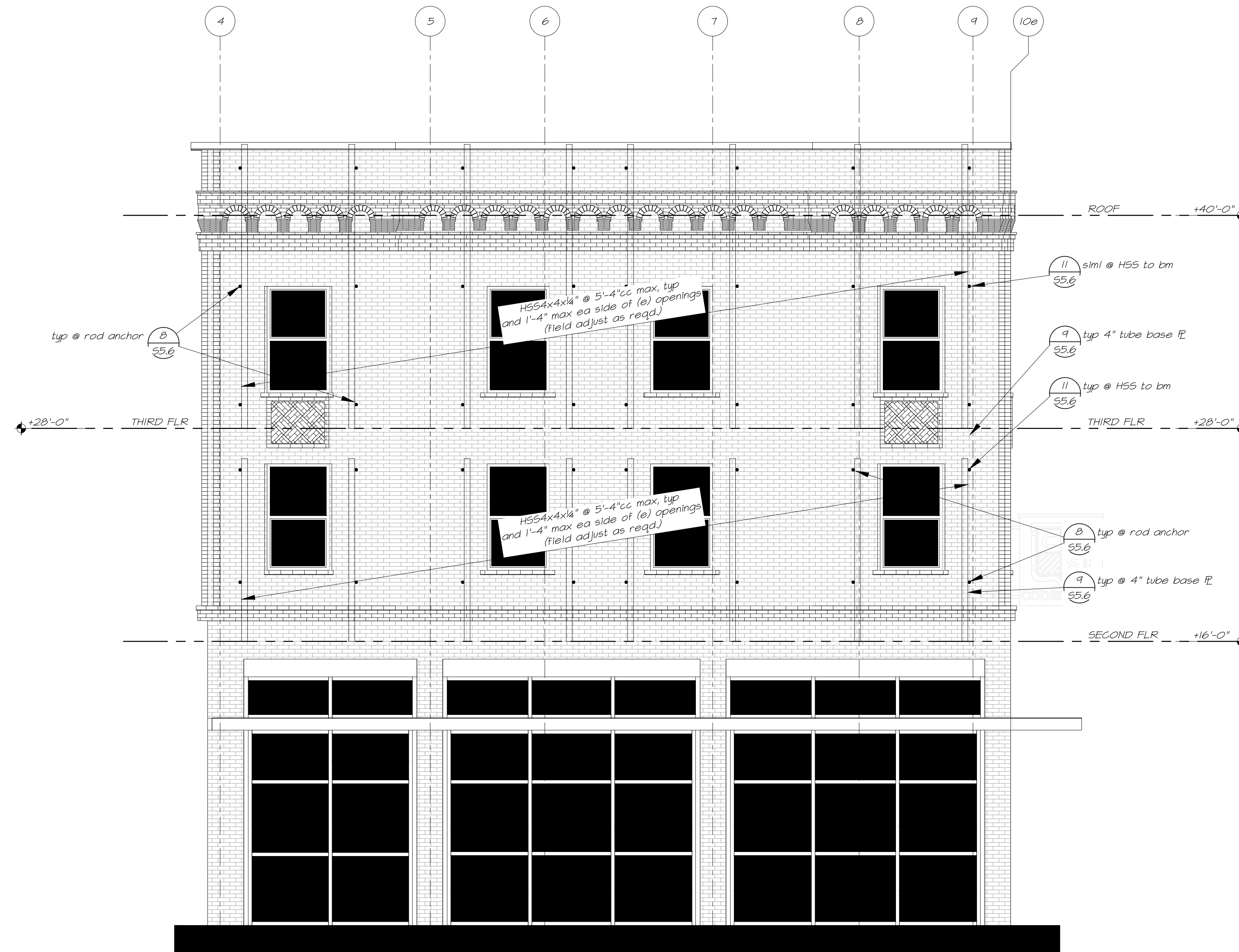
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FRAME ELEVATIONS









A South Elevation  
S3.4  
1/4"=1'-0"



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(E) BRICK FACADE  
BRACING BUILDING  
ELEVATION (SOUTH)

S3.4





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**(E) BRICK FACADE BRACING BUILDING ELEVATION (EAST)**

**S3.5**



**A** East Elevation  
 S3.5 1/4"=1'-0"



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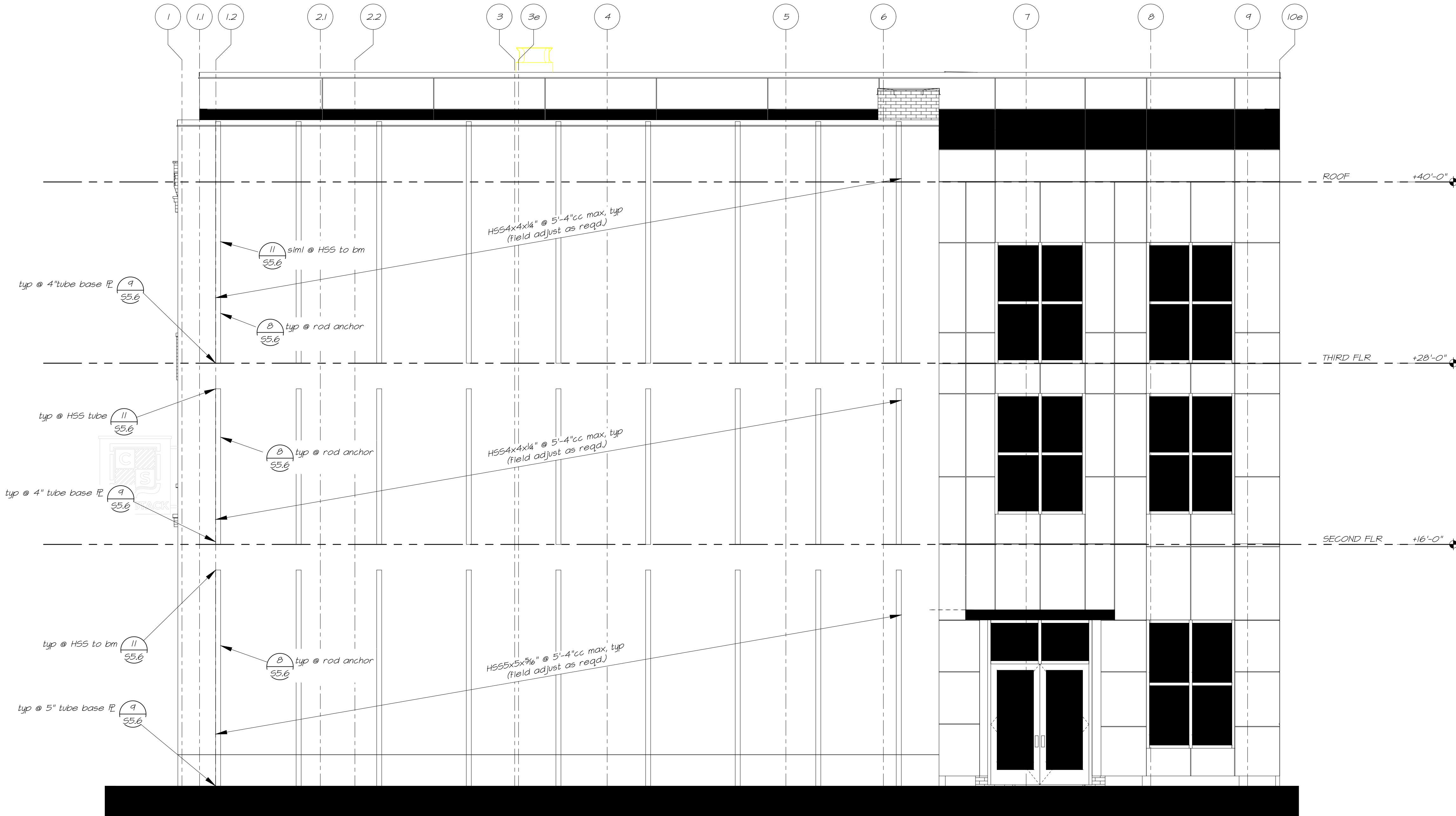
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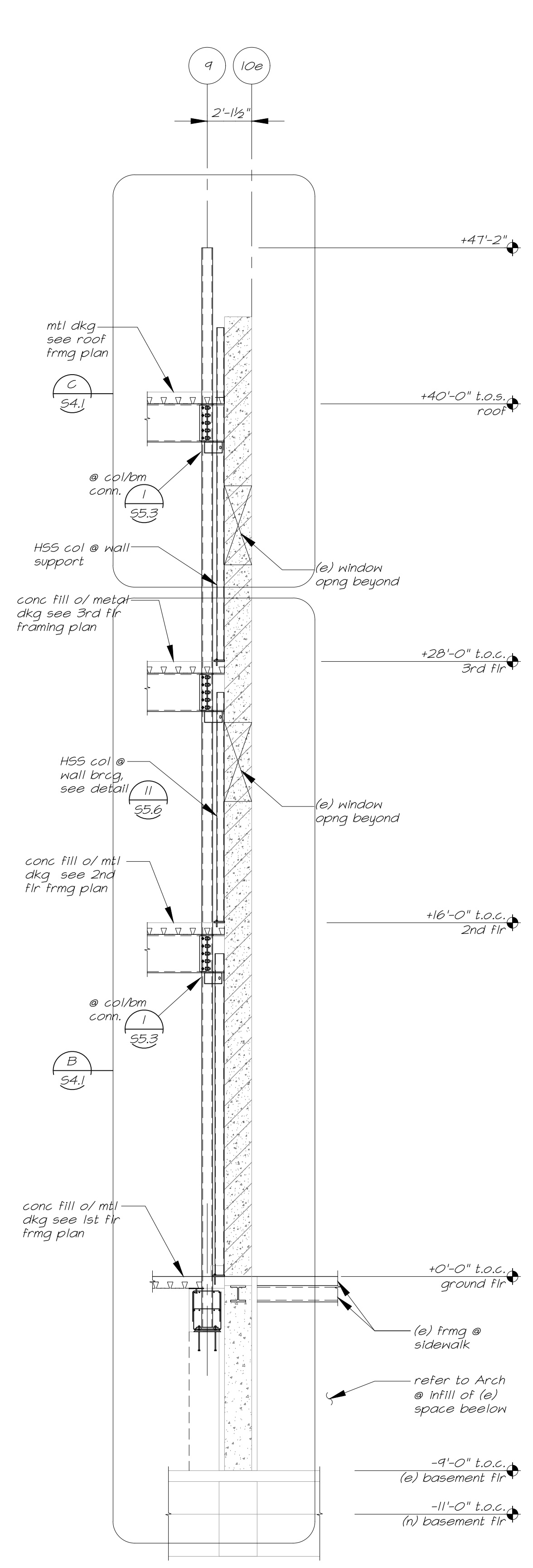
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(E) BRICK FACADE  
 BRACING BUILDING  
 ELEVATION (NORTH)

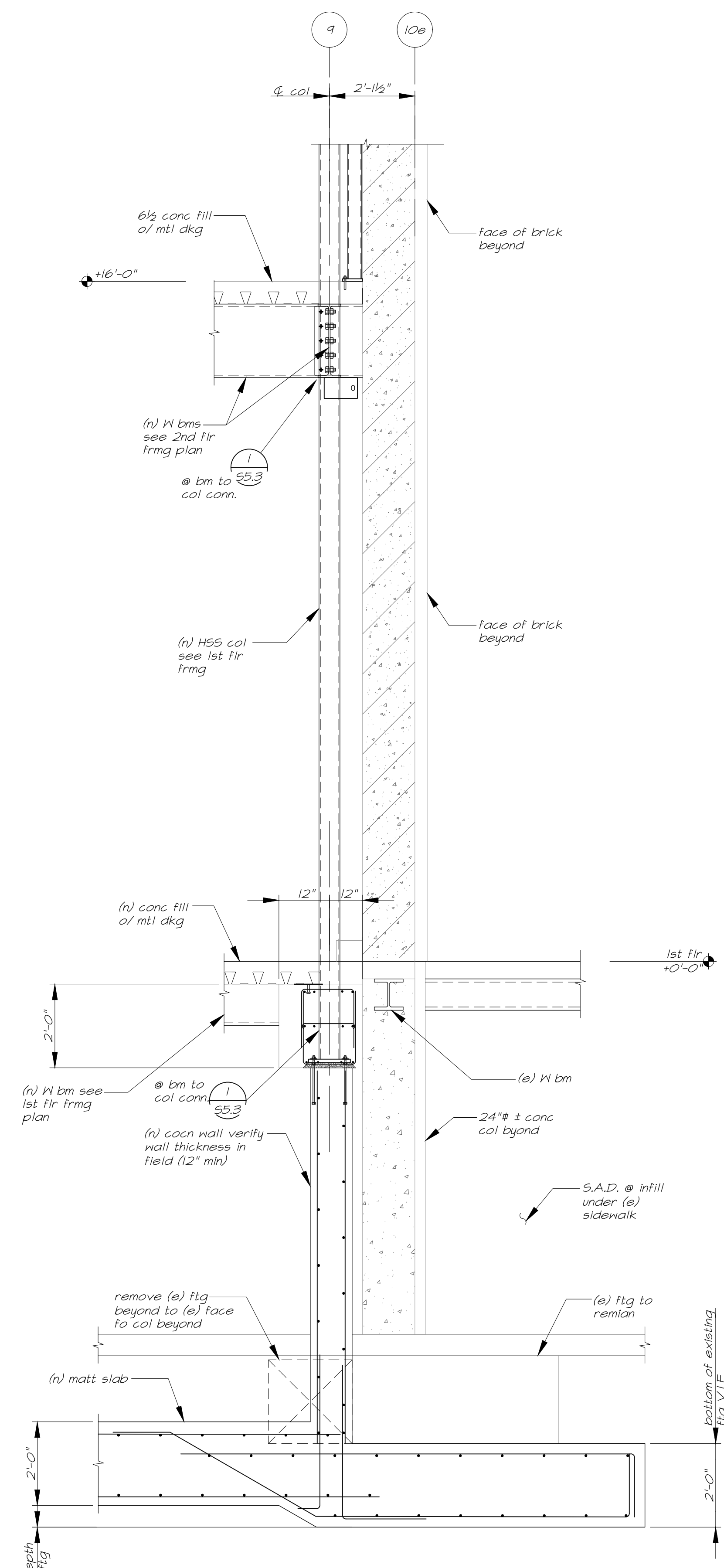


**A** North Elevation  
 S3.6 1/4"=1'-0"

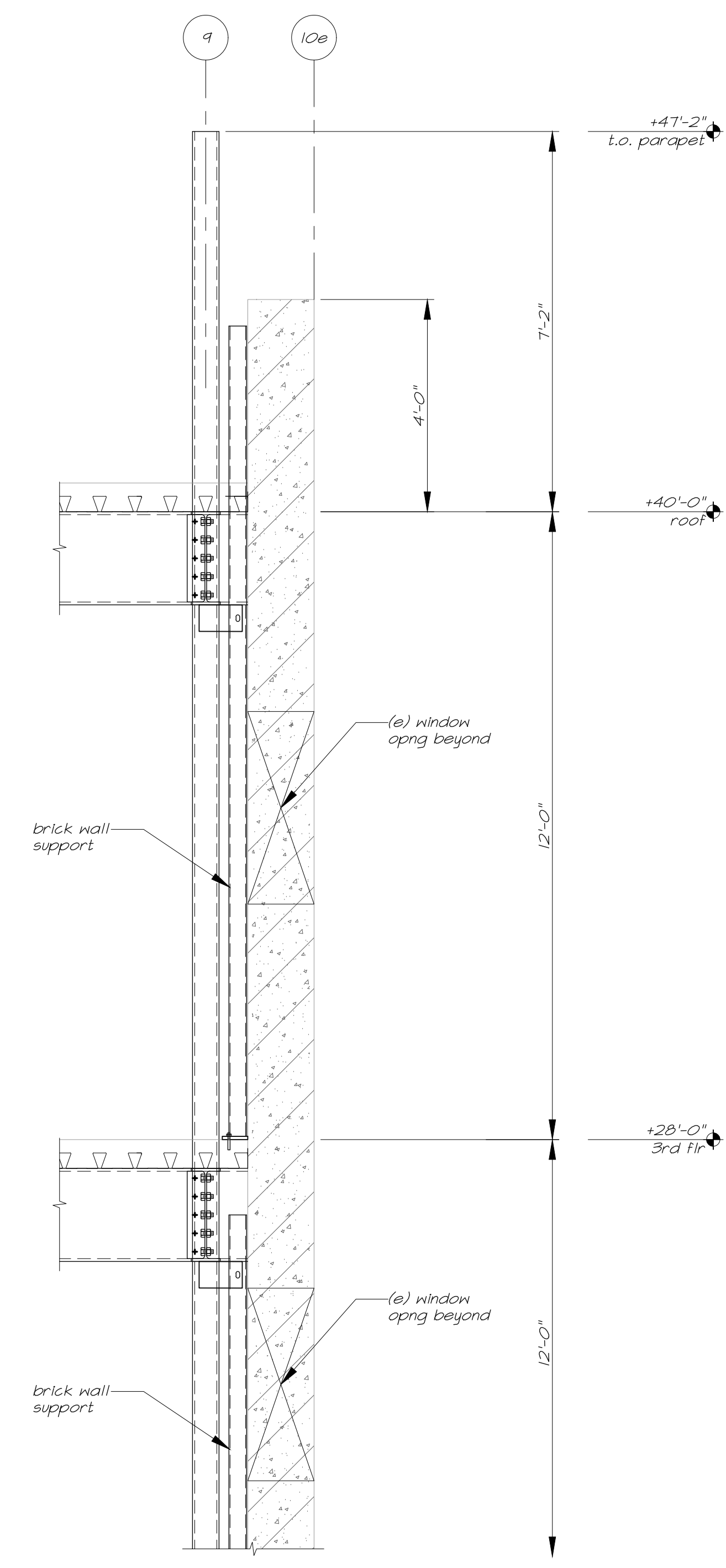




**A** Section - grid 9  
S4.1 1/4" = 1'-0"



**B** Section  
S4.1 1/2" = 1'-0"



**C** Section  
S4.1 1/2" = 1'-0"



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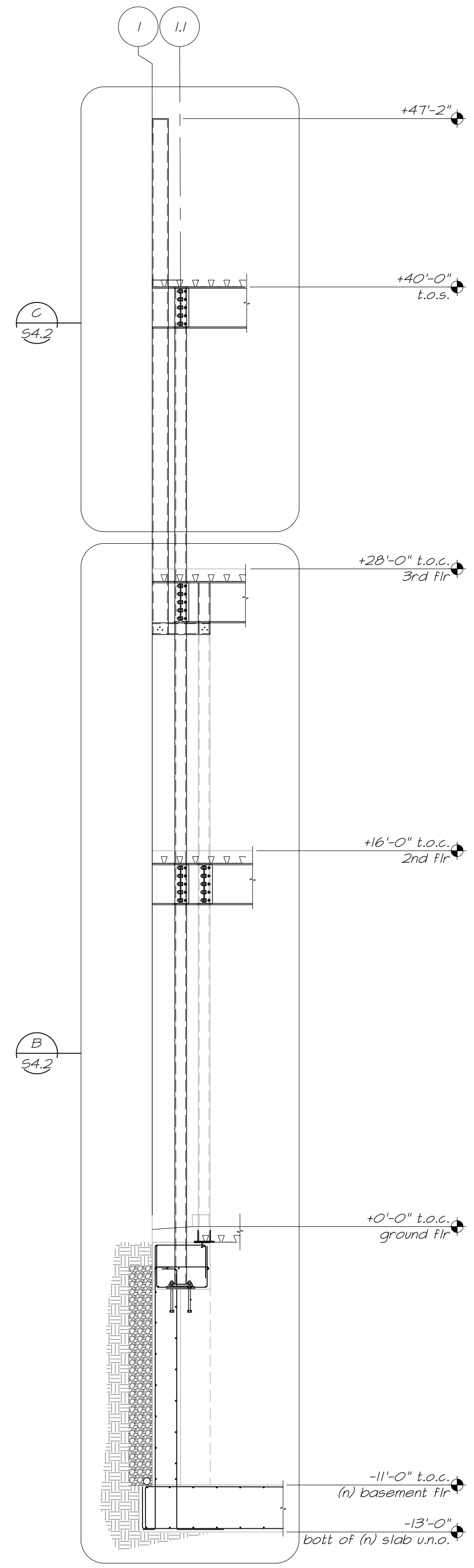
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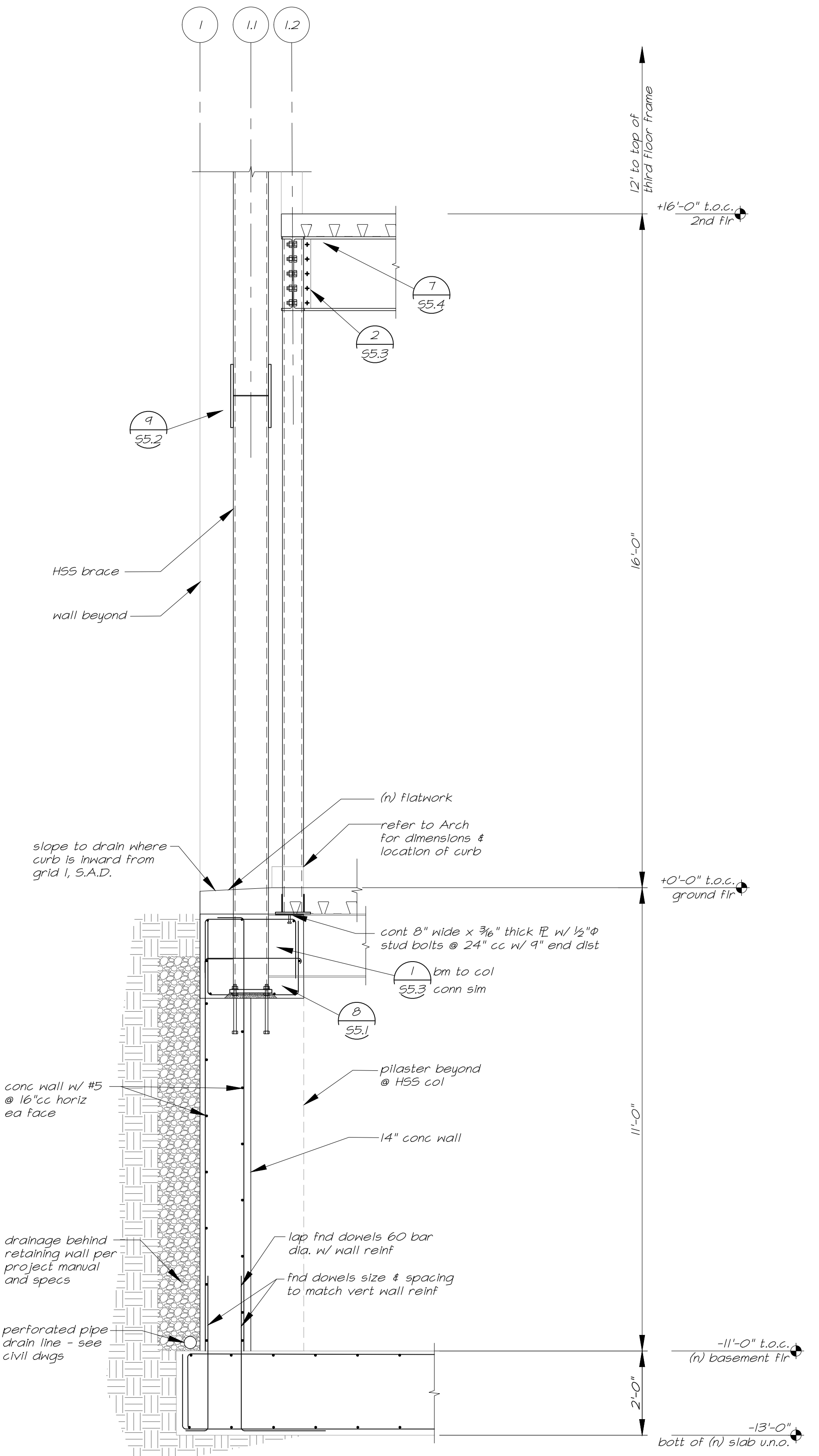
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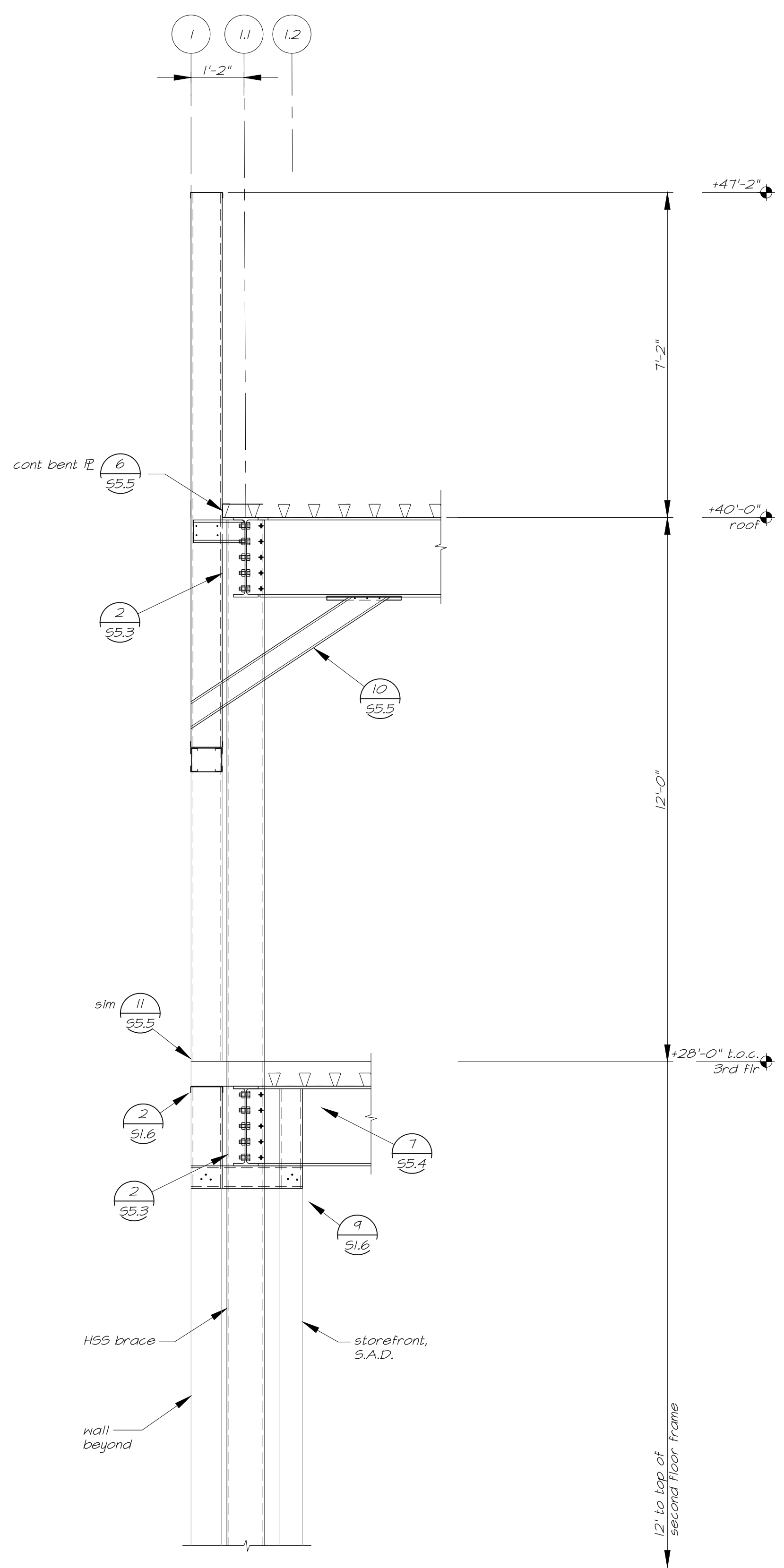
**SECTIONS**



**A** Section - grid I at F-H.1/J.1-L  
 1/4"=1'-0"



**B** Section  
 1/2"=1'-0"



**C** Section  
 1/2"=1'-0"



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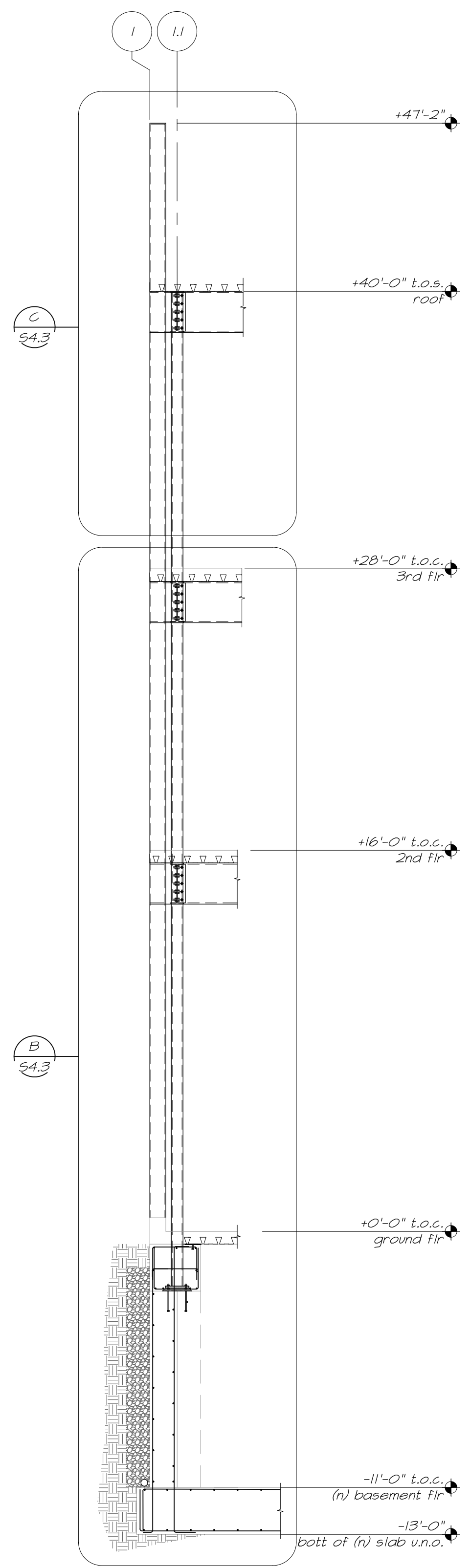
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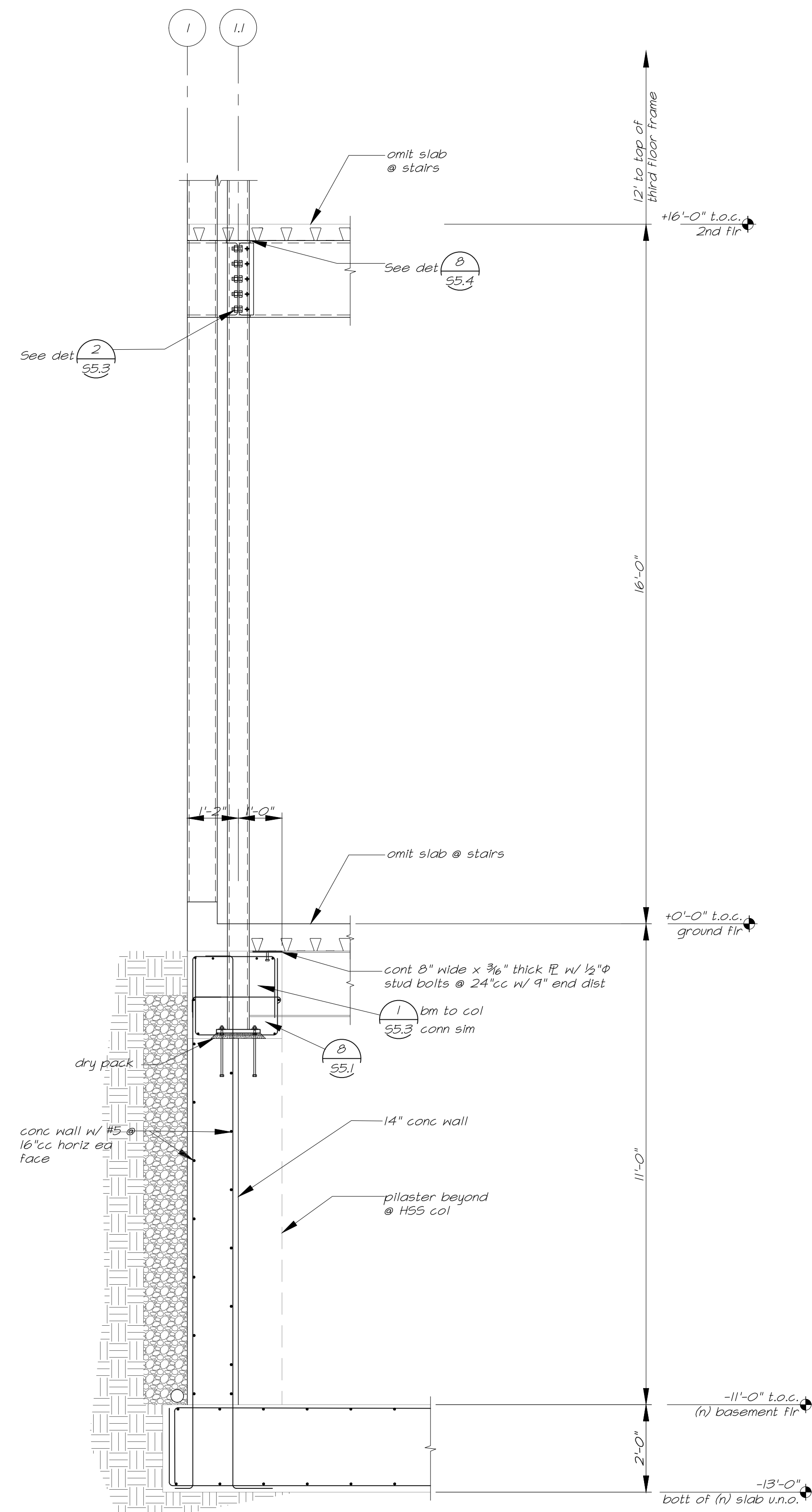
**BUILDING  
 ELEVATION (WEST)**

**S4.2**

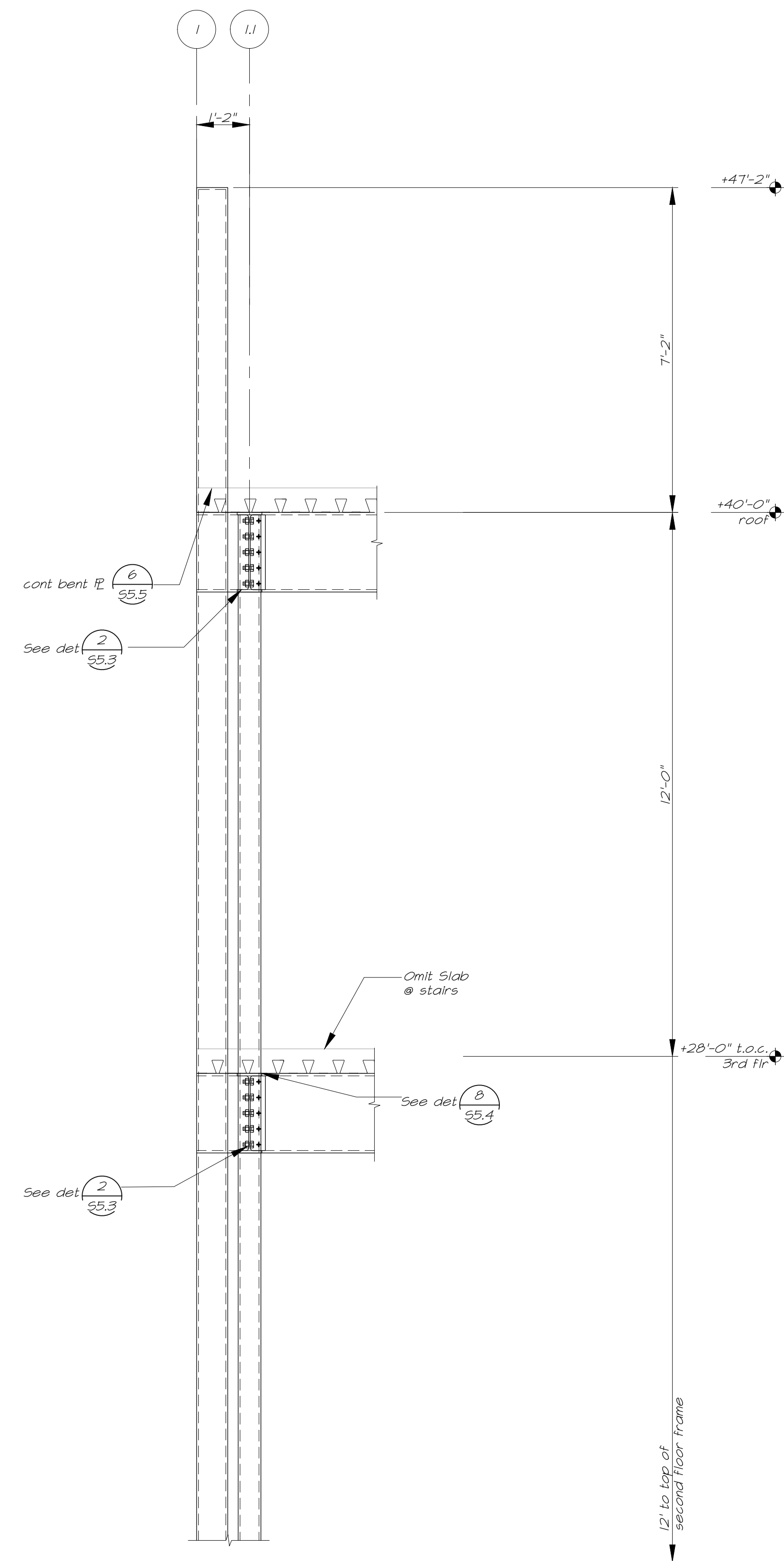




**A** Section - grid I at A-F/H.I-J.I/L-Q.3  
 S4.3 1/4"=1'-0"



**B** Section  
 S4.3 1/2"=1'-0"



**C** Section  
 S4.3 1/2"=1'-0"



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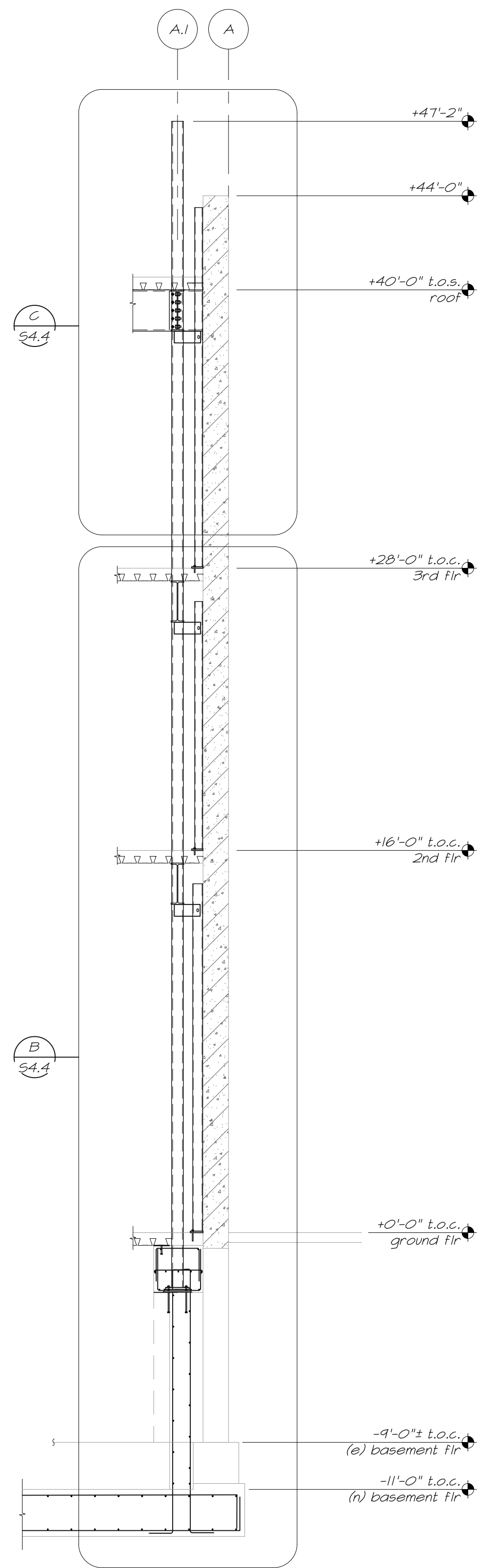
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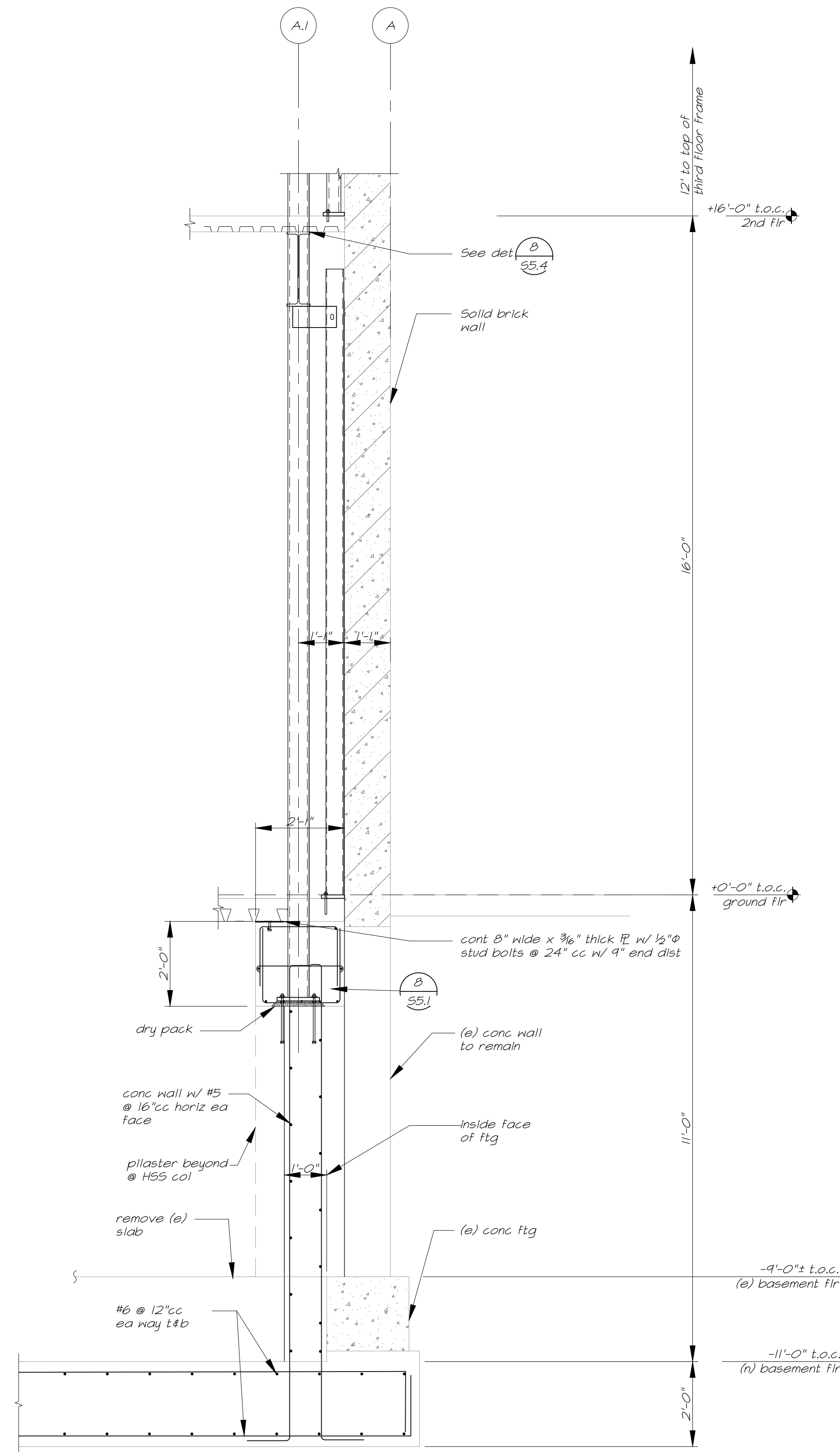
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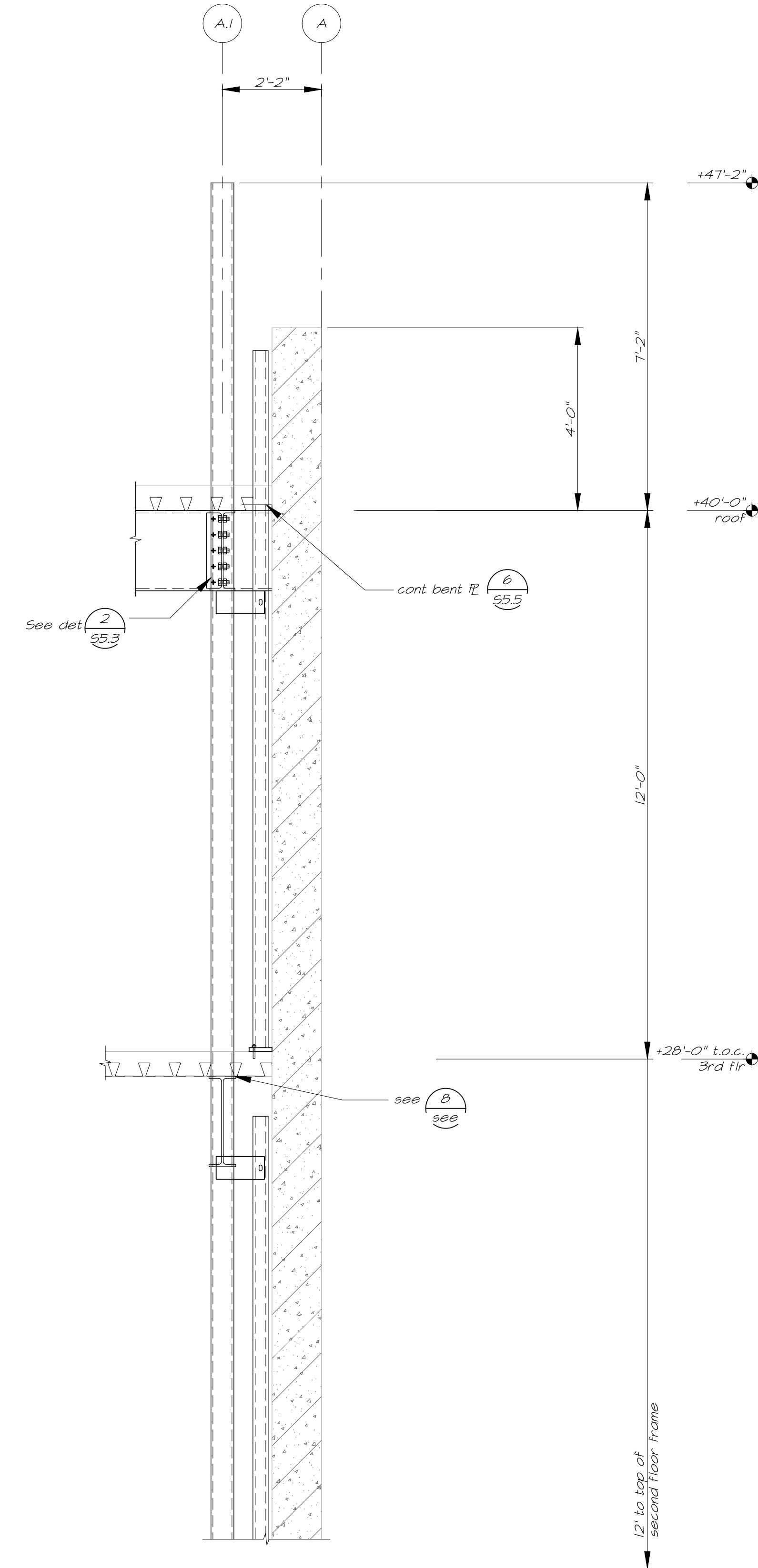
**S4.3**



**A** Section @ GL A from 3-9  
S4.4 1/4"=1'-0"



**B** Section  
S4.4 1/2"=1'-0"



**C** Section  
S4.4 1/2"=1'-0"



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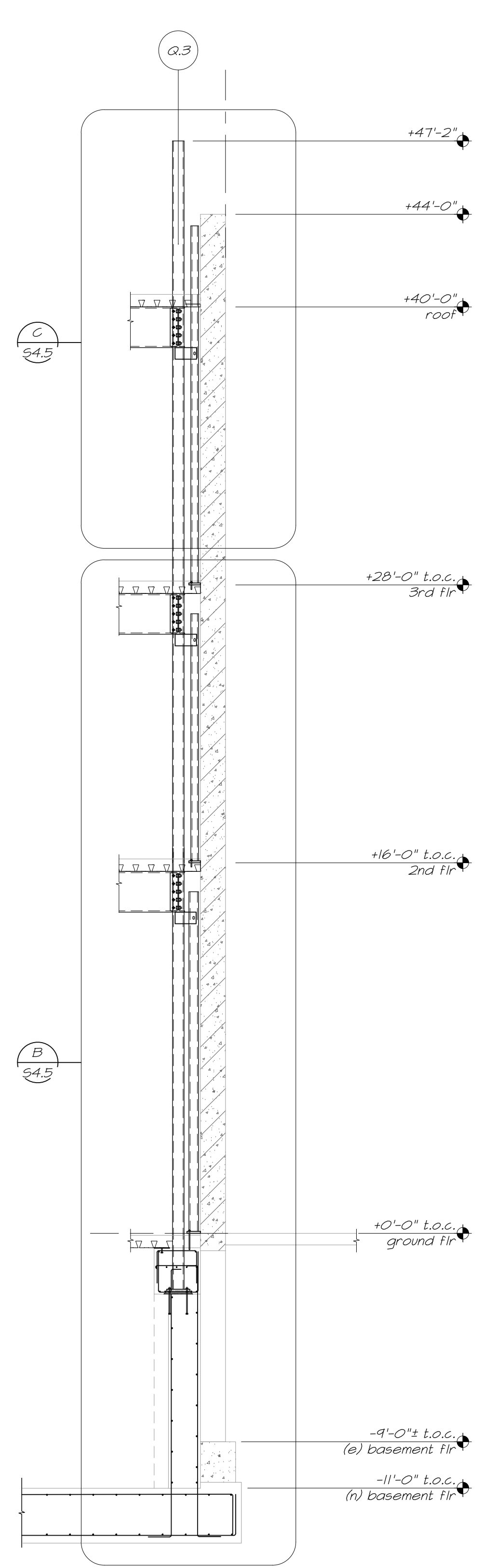
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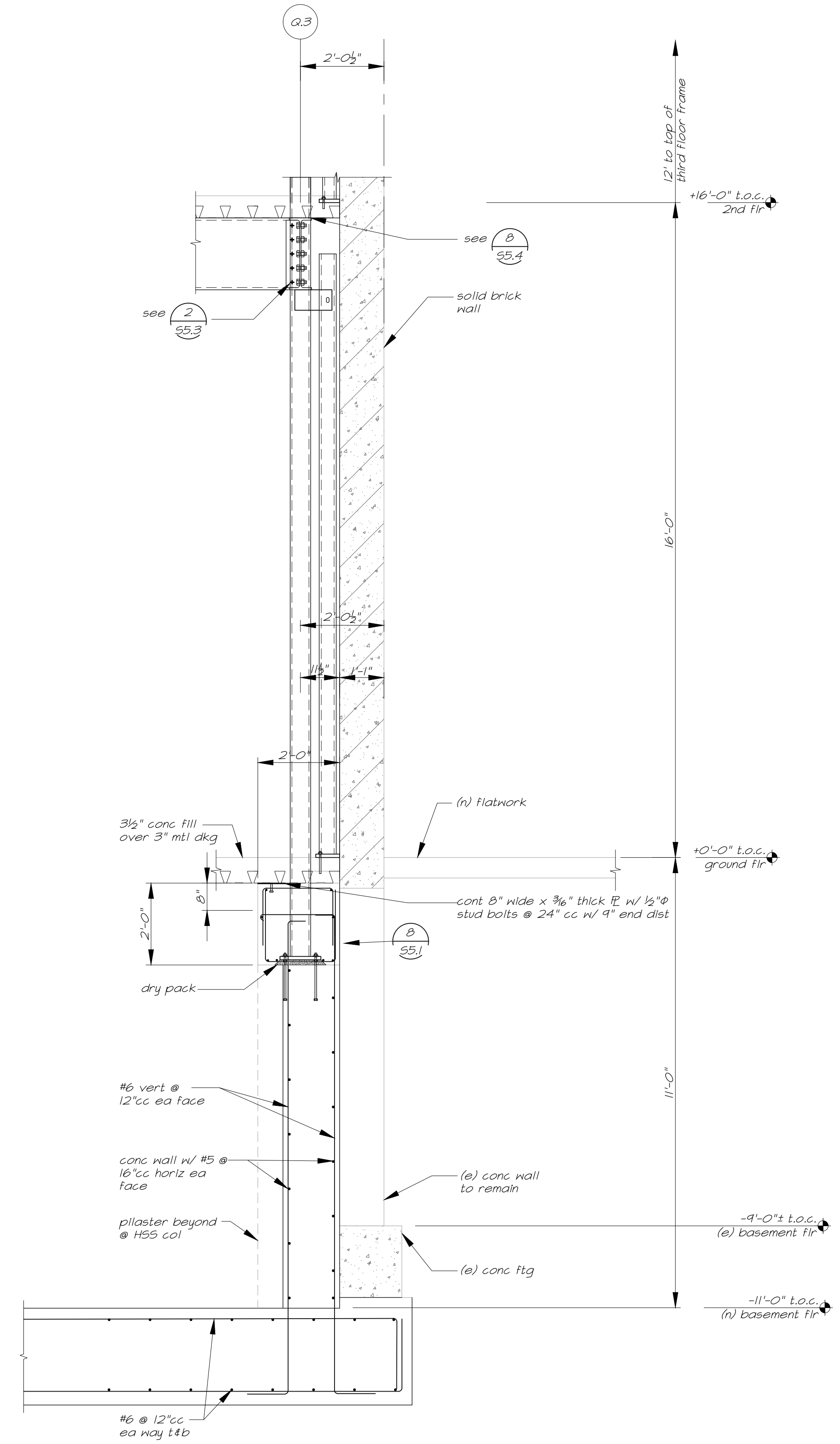
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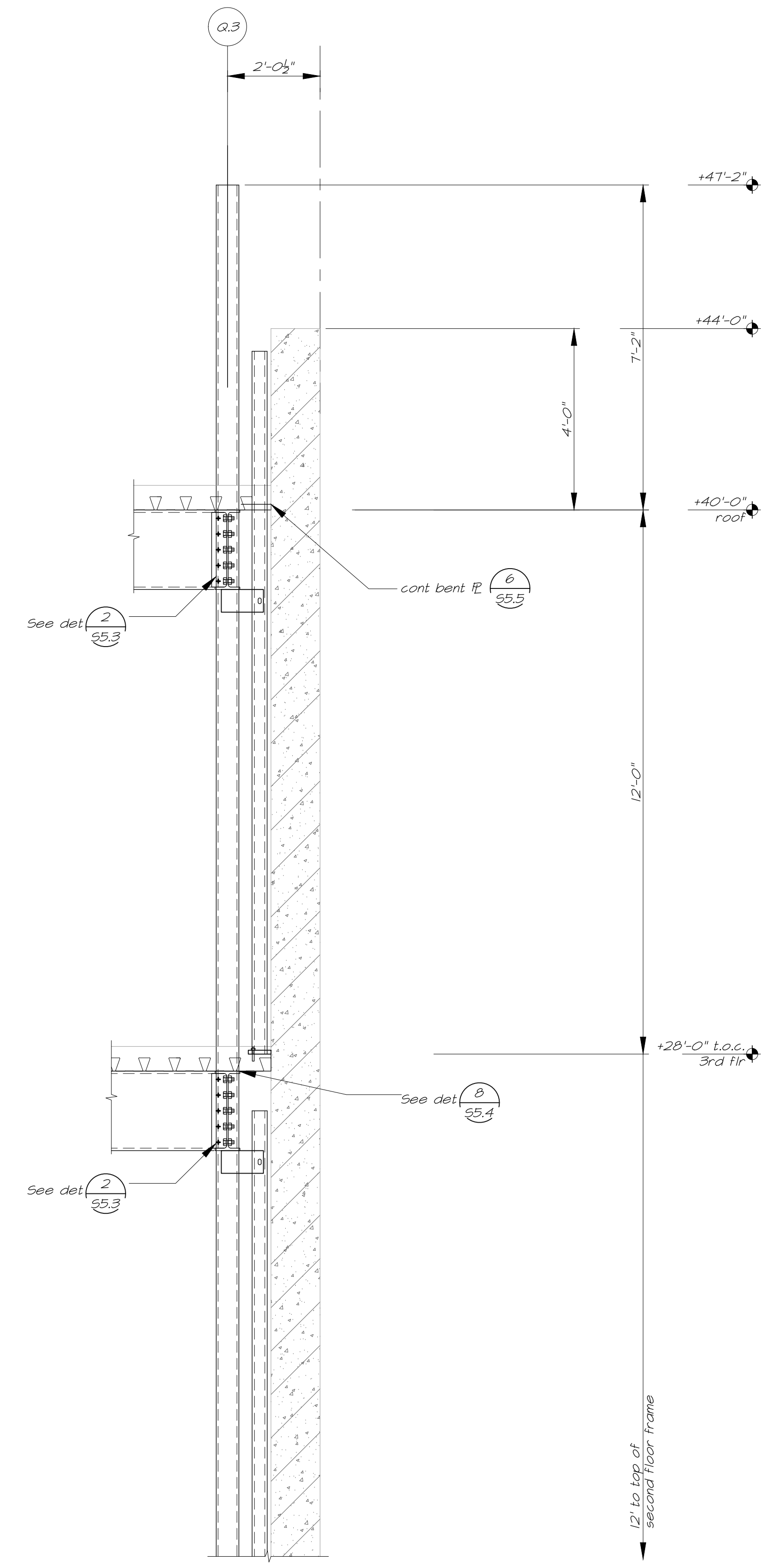
**SECTIONS**



**A** Section @GL Q.3 from 3-9  
 S4.5 1/2"=1'-0"



**B** Section  
 S4.5 1/2"=1'-0"



**C** Section  
 S4.5 1/2"=1'-0"



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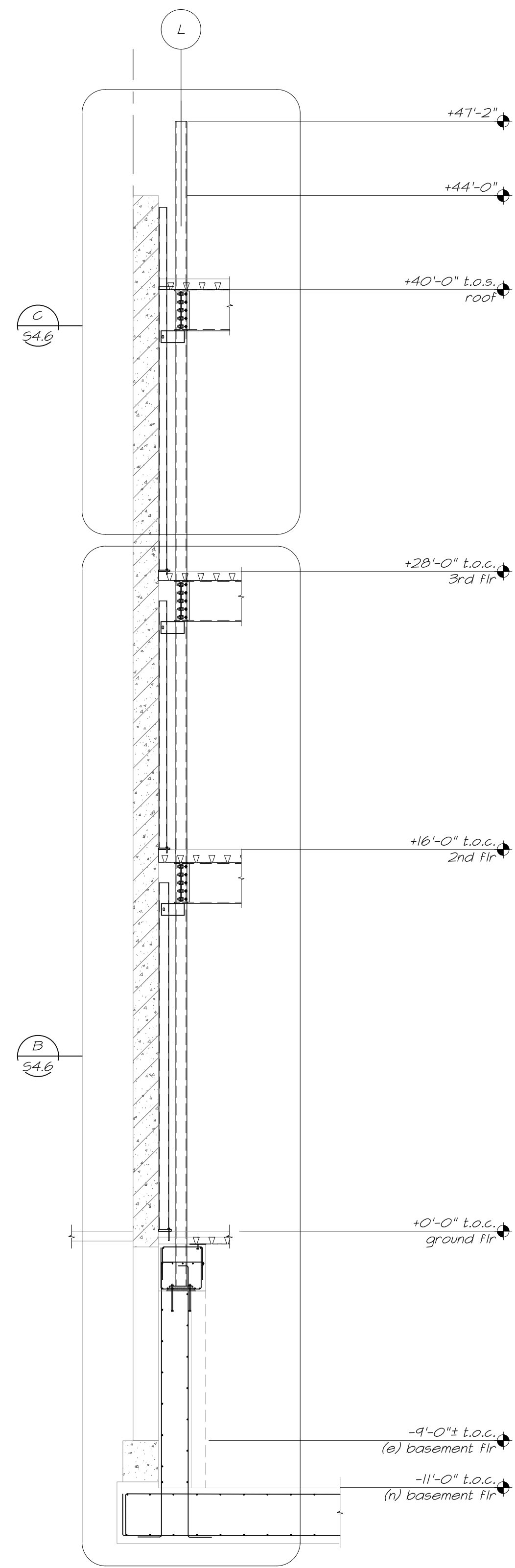
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	Addendum #5 12/04/2024

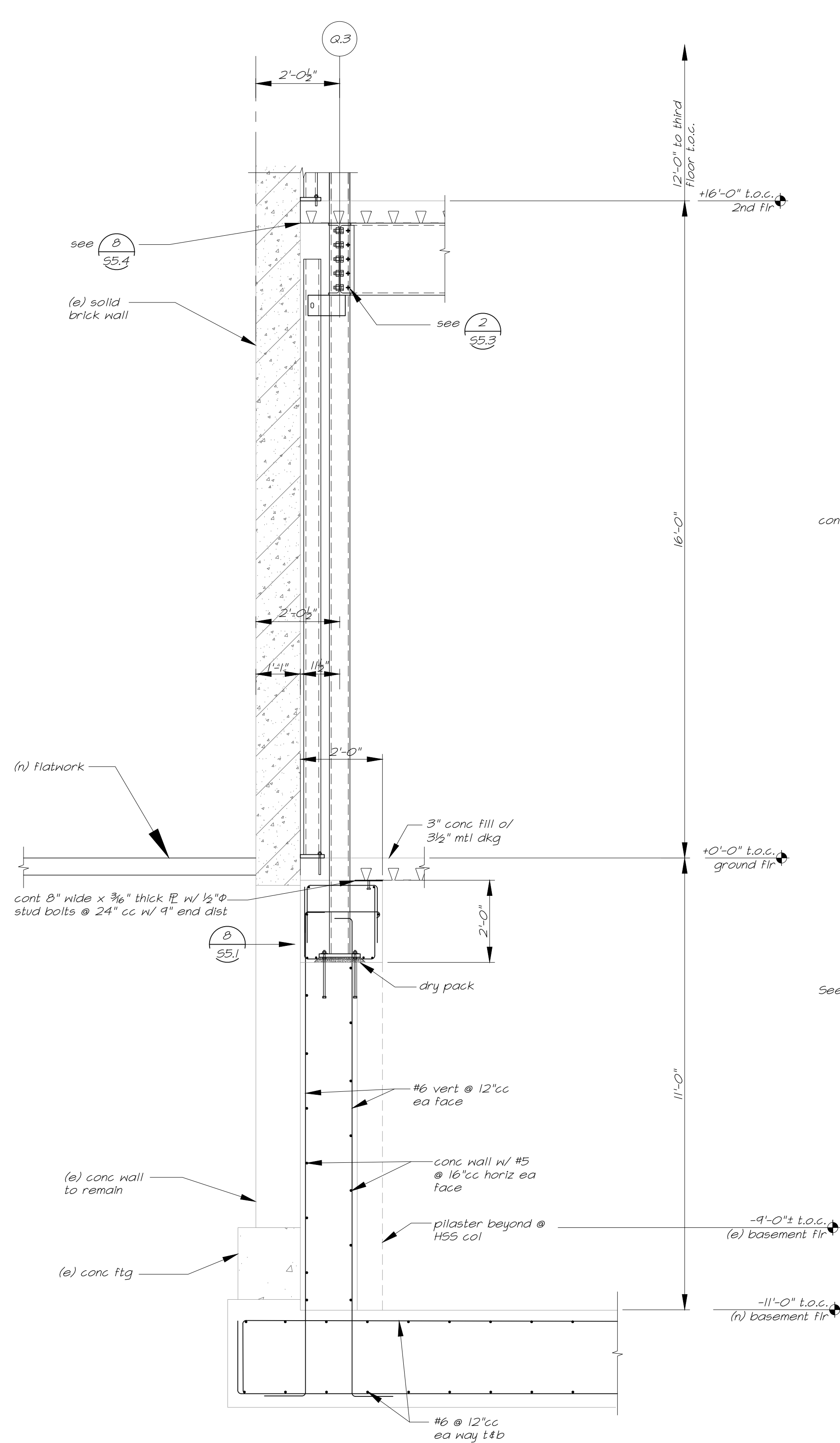
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 ISSUE DATE: 12.04.24  
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**SECTIONS**

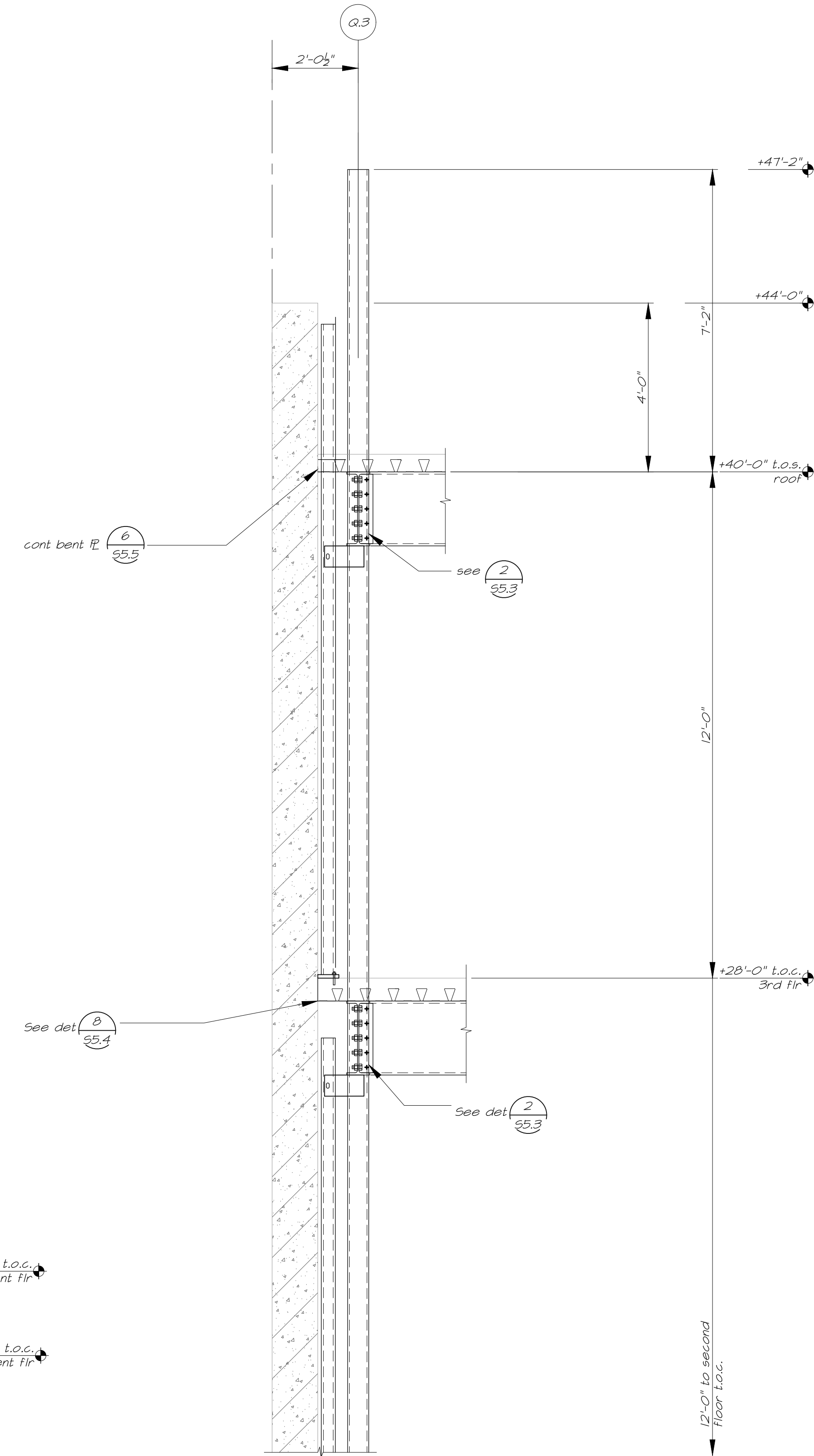




**A** Section  
S4.6 14'-1-0"



**B** Section  
S4.6 15'-1-0"



**C** Section  
S4.6 15'-1-0"



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SECTIONS

**S4.6**





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 Stockton, California 95207  
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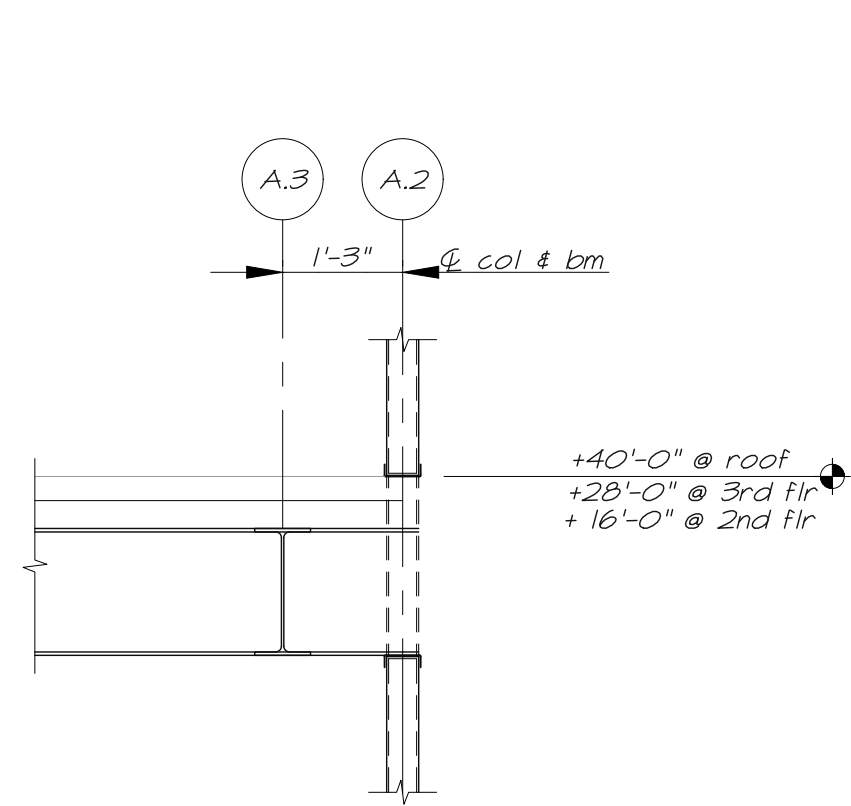
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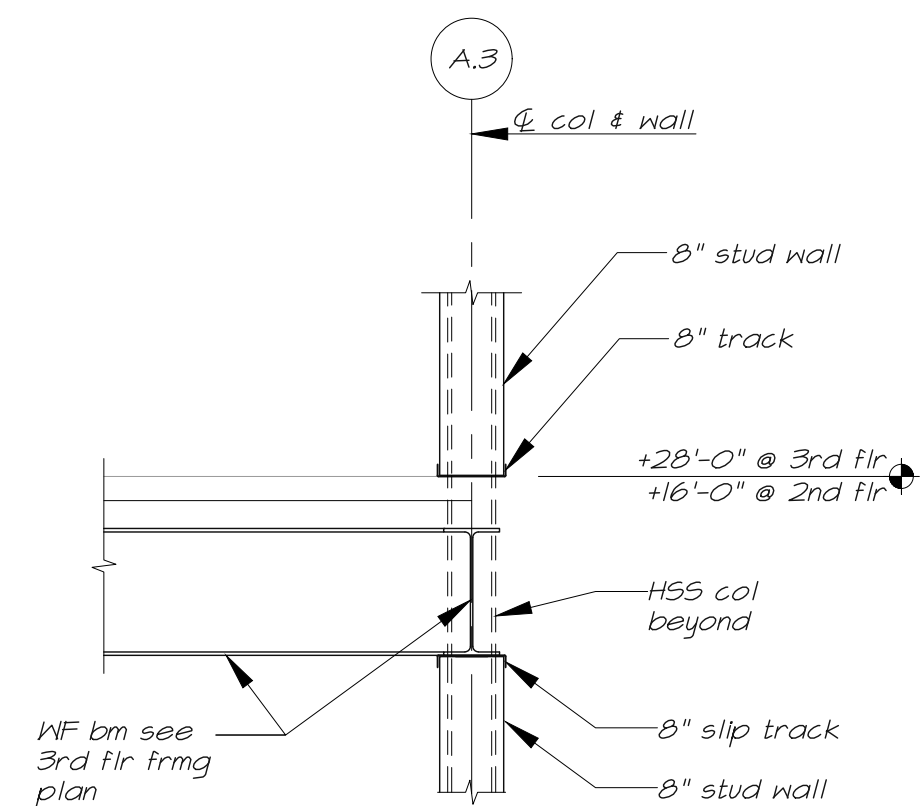
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 ISSUE DATE: 12.04.24  
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**SECTIONS & DETAILS**

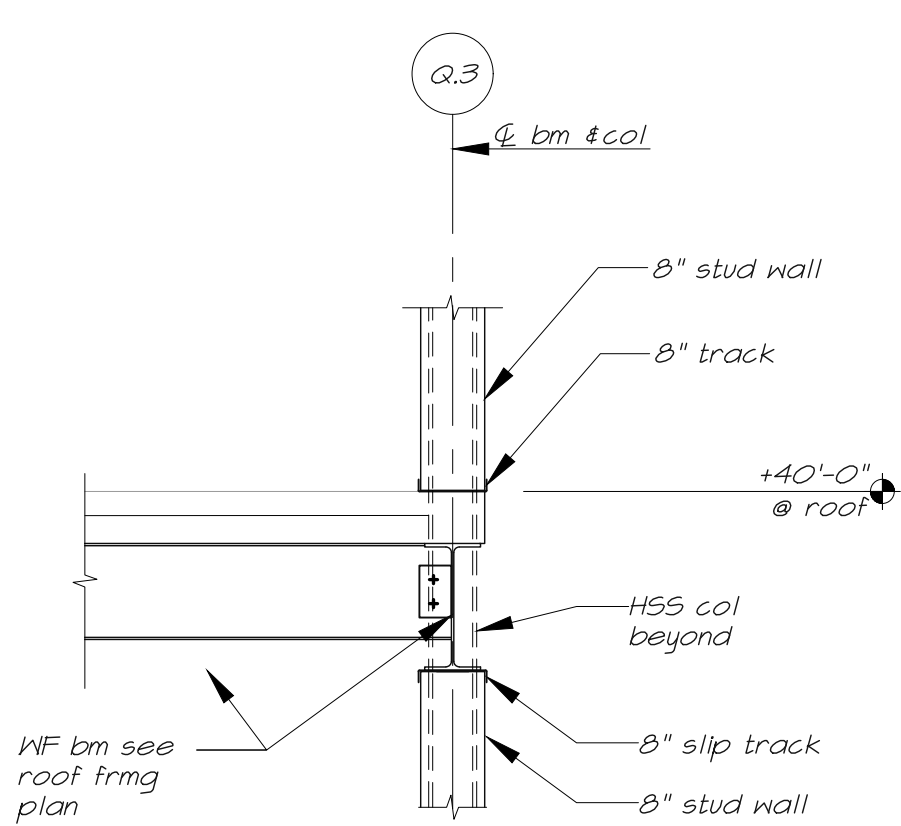
**S4.7**



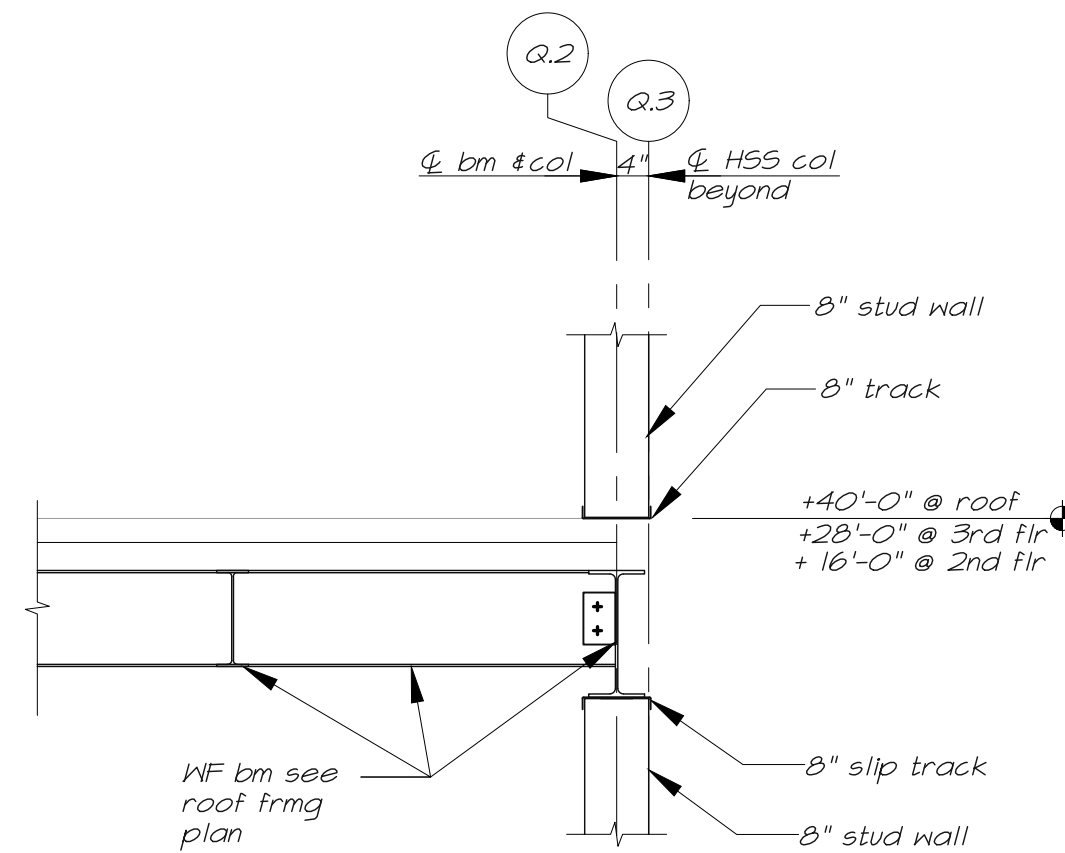
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 S4.7 1/2" = 1'-0"



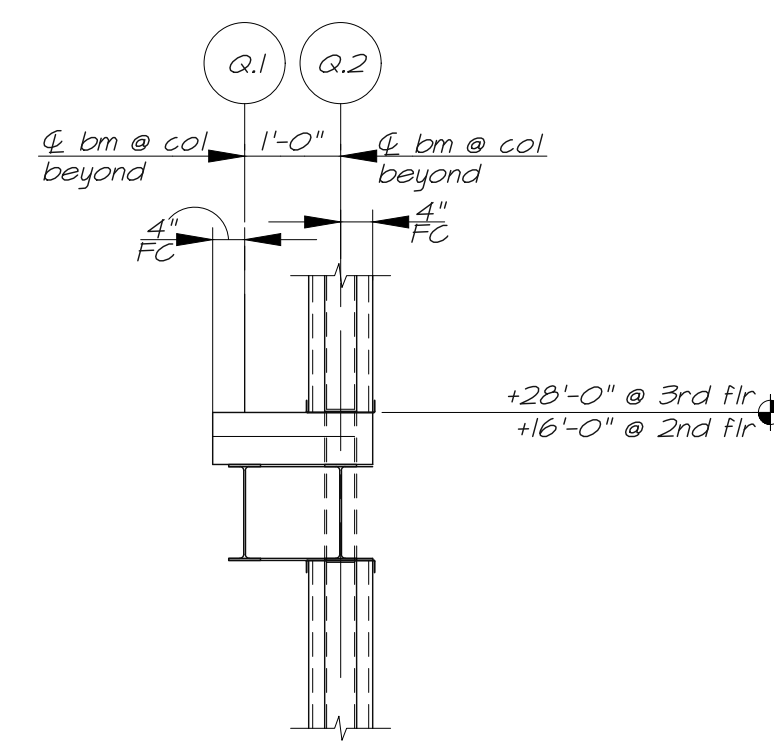
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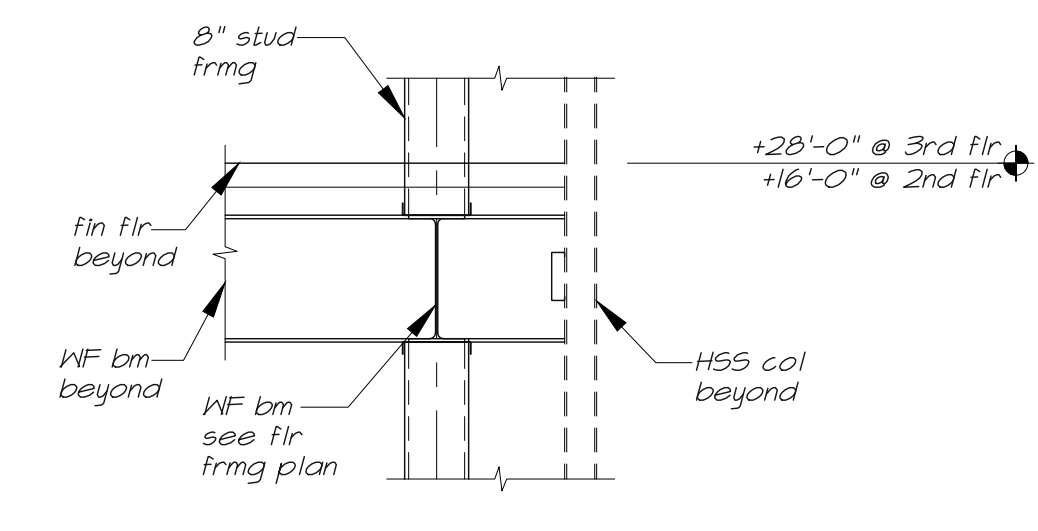
**5 Section**  
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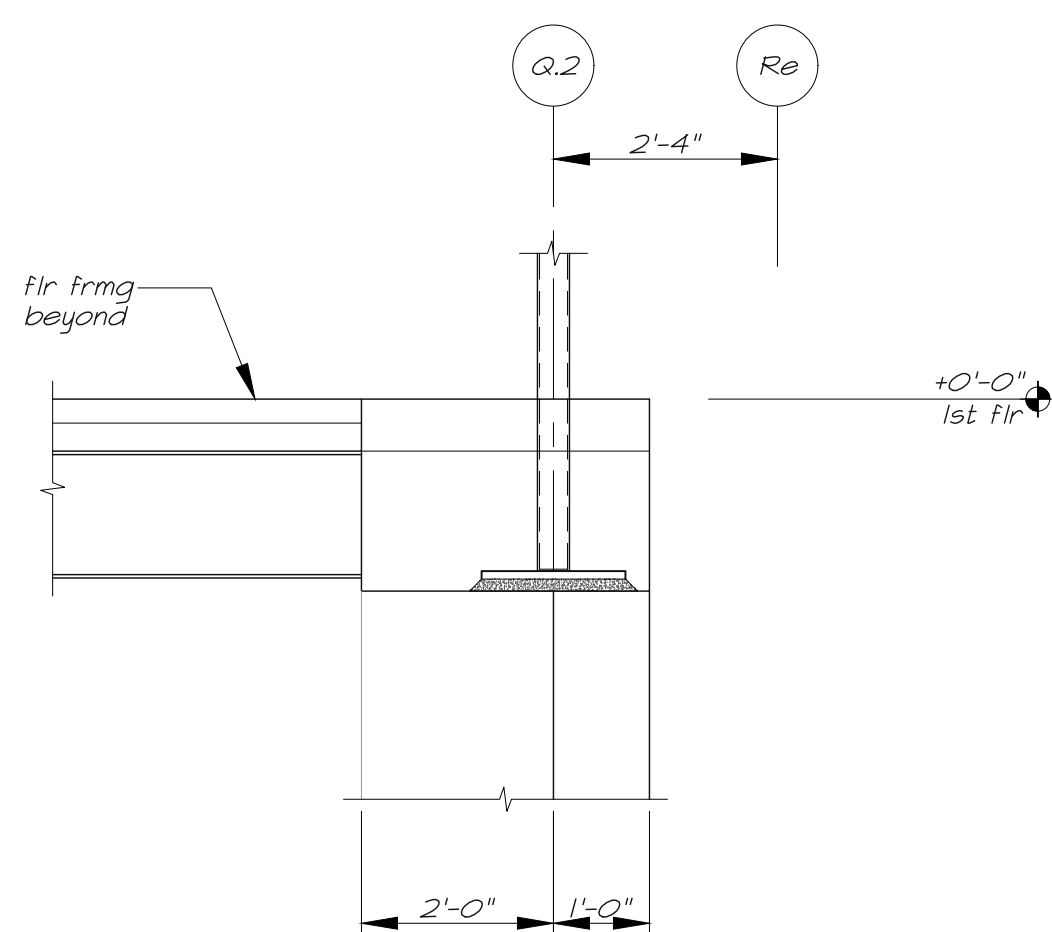
**6 Section**  
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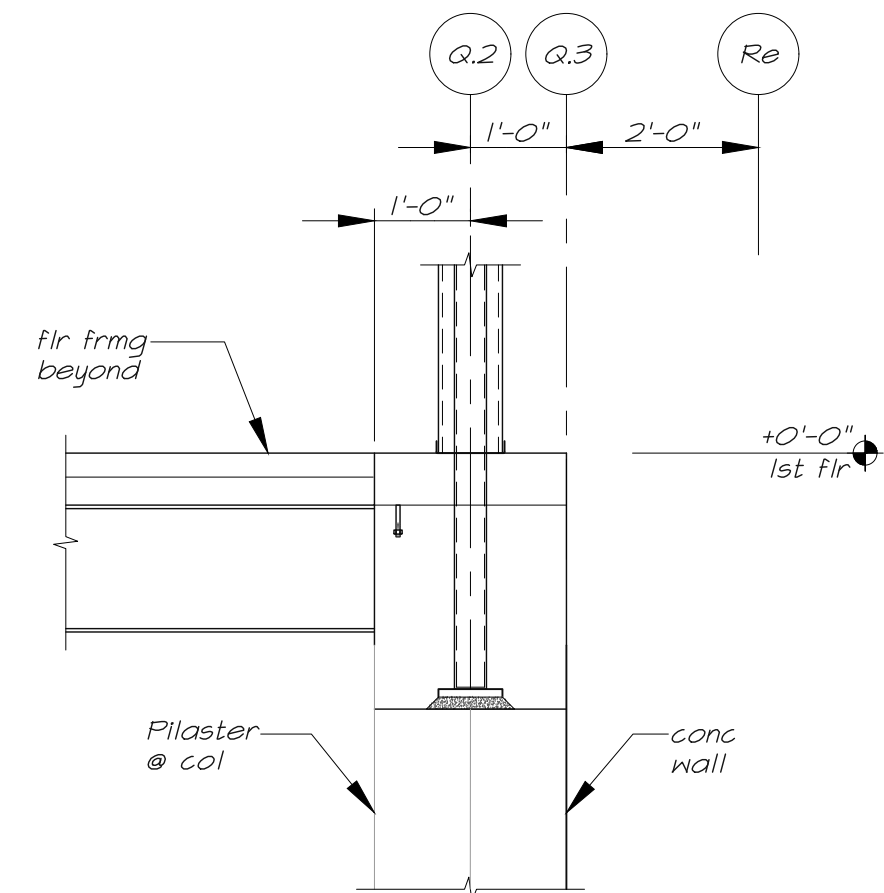
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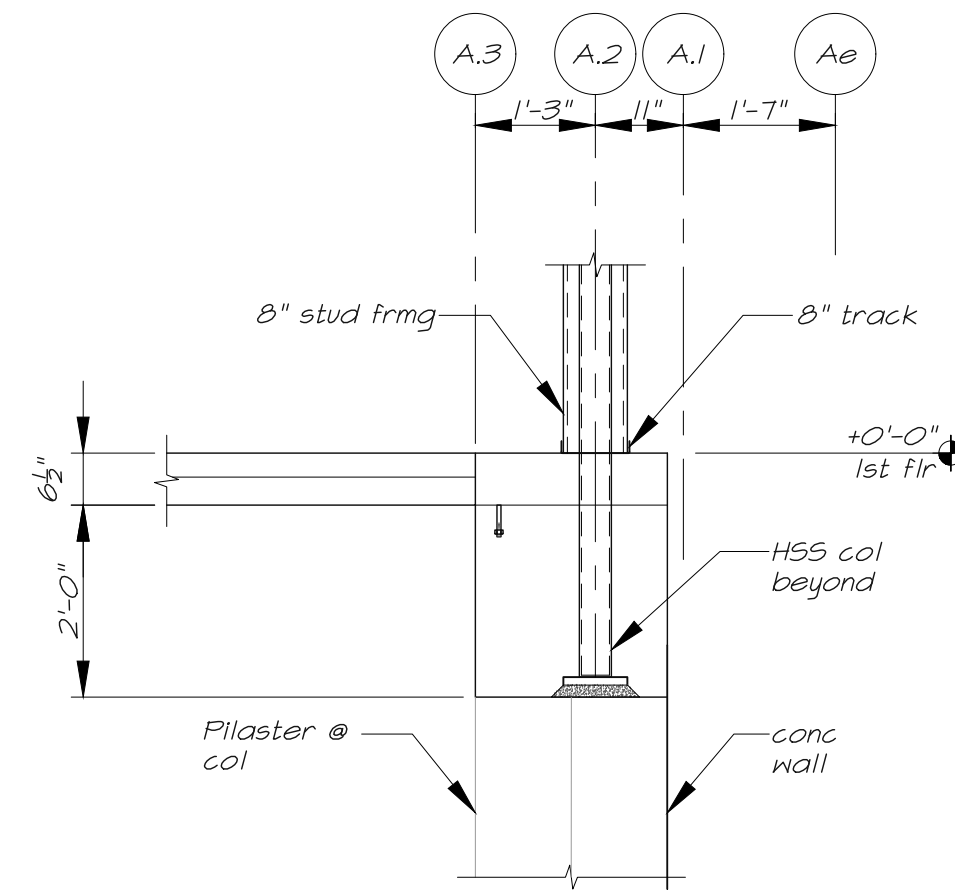
**8 Section**  
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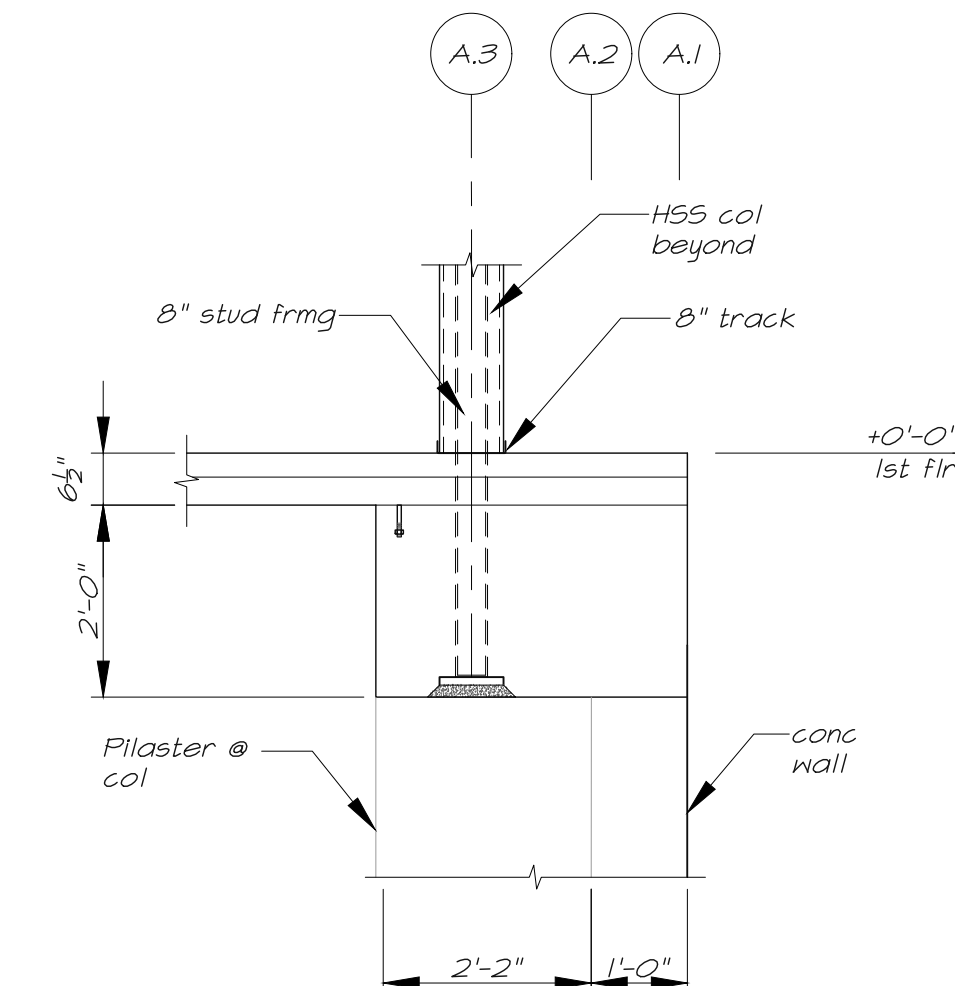
**1 Section**  
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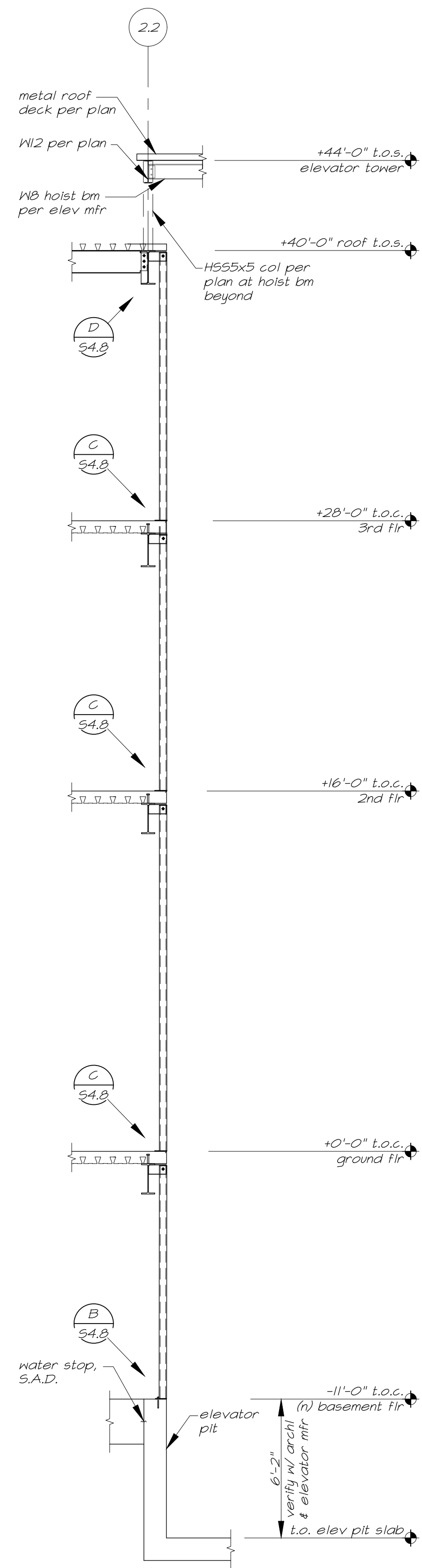
**2 Section**  
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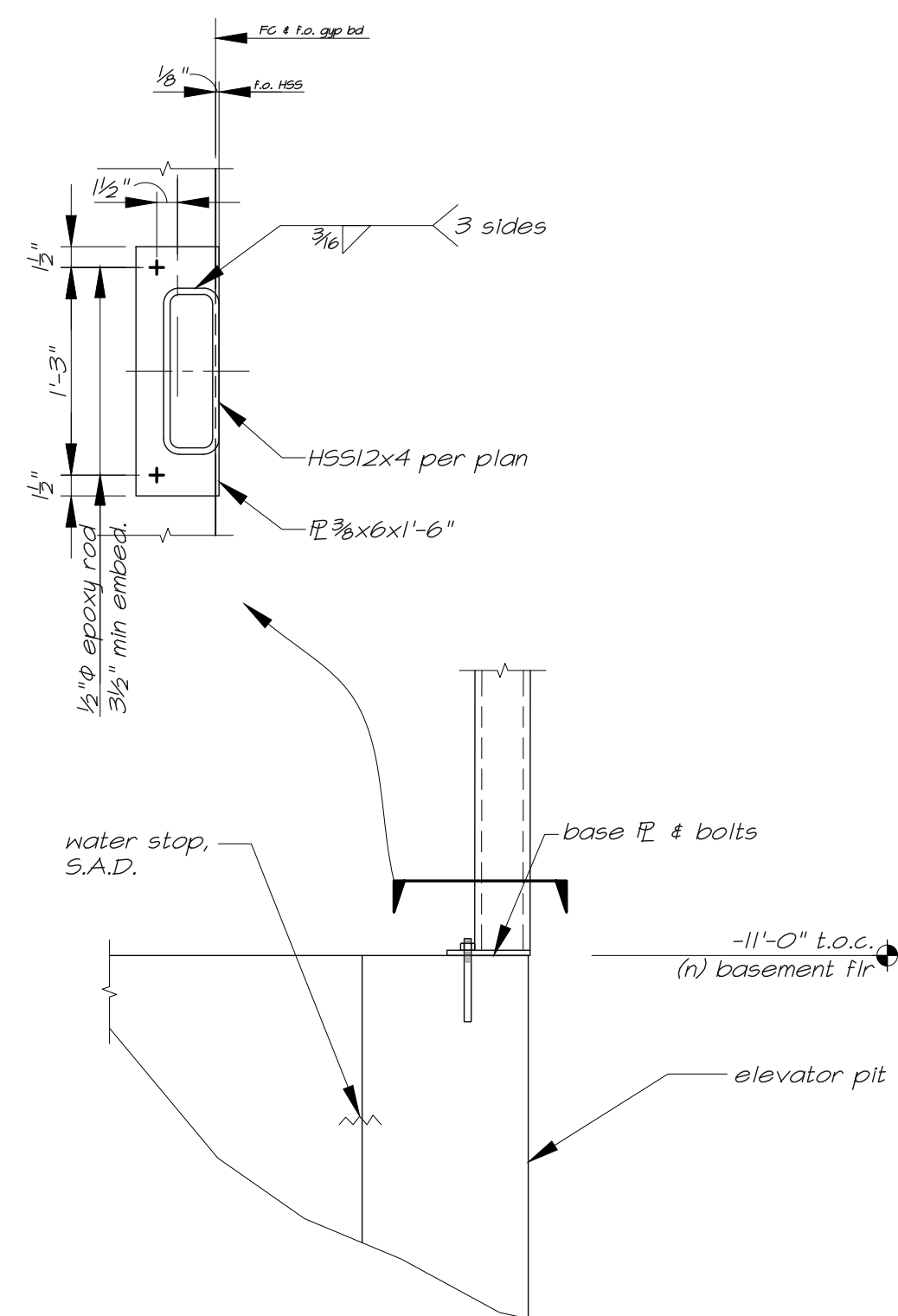
**3 Section**  
 S4.7 1/2" = 1'-0"



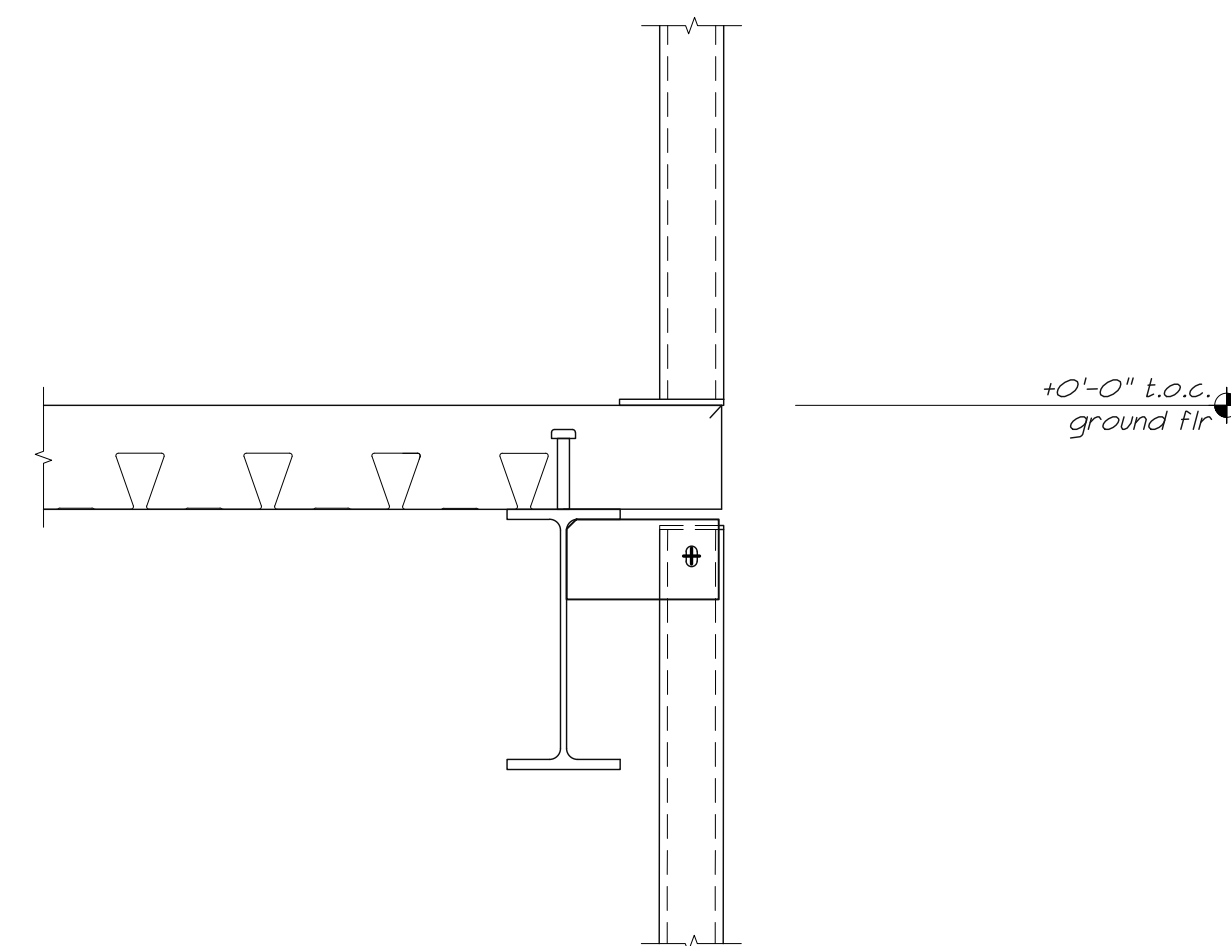
**4 Section**  
 S4.7 1/2" = 1'-0"



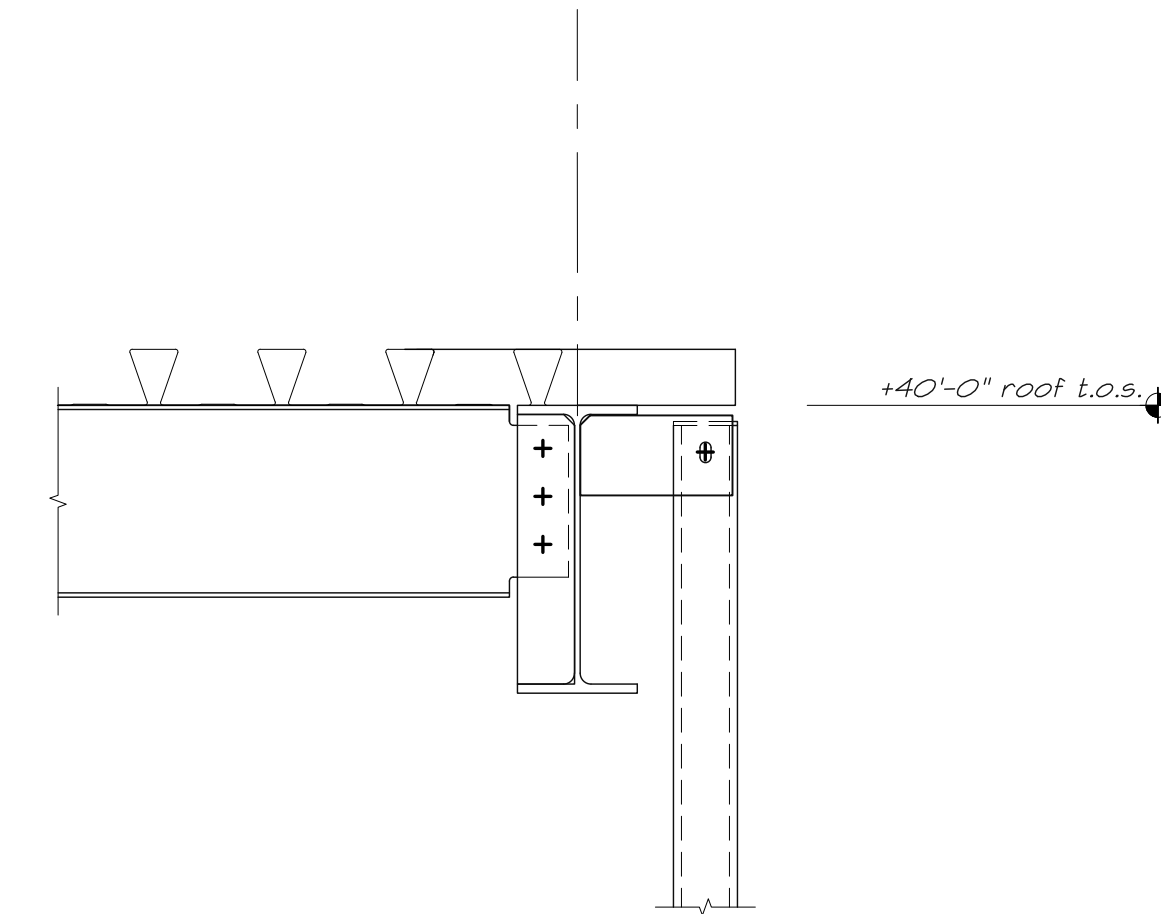
**A** Section - grid 2.2 at elevator  
 S4.8 1/4"=1'-0" grid 3 sim



**B** Section  
 S4.8 1"=1'-0" at elevator pit wall



**C** Section  
 S4.8 1"=1'-0" at conc of metal deck



**D** Section  
 S4.8 1"=1'-0" at roof



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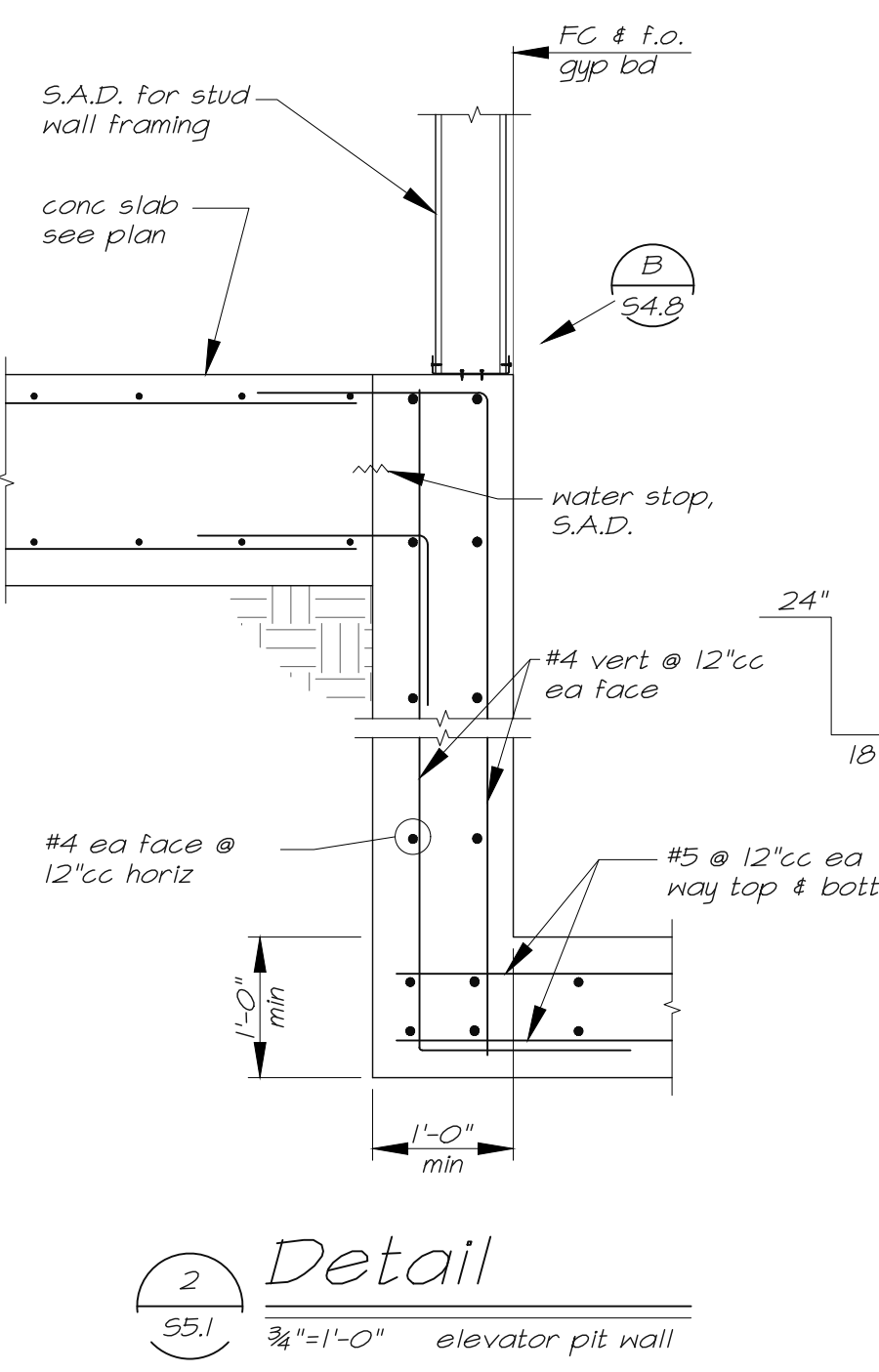
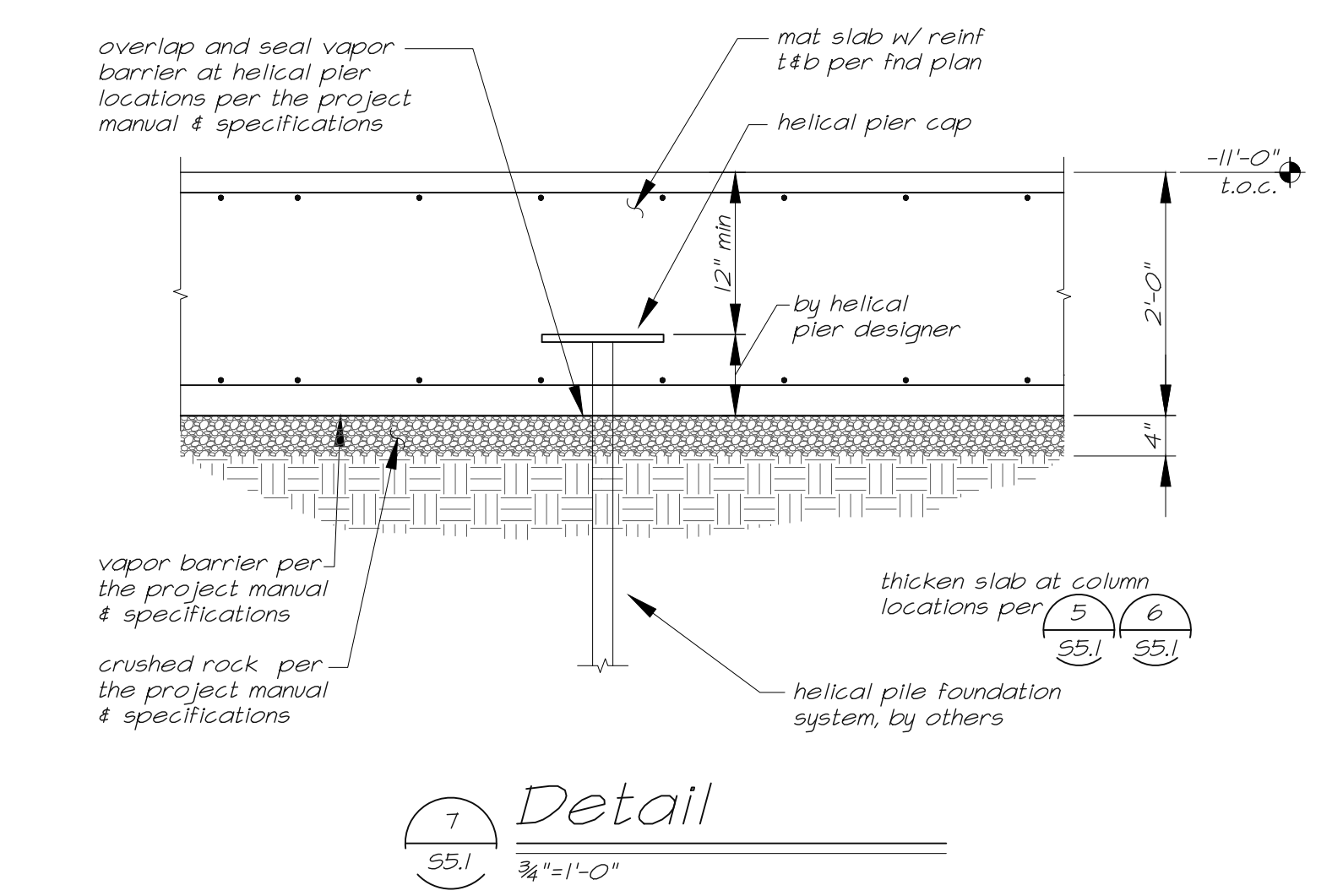
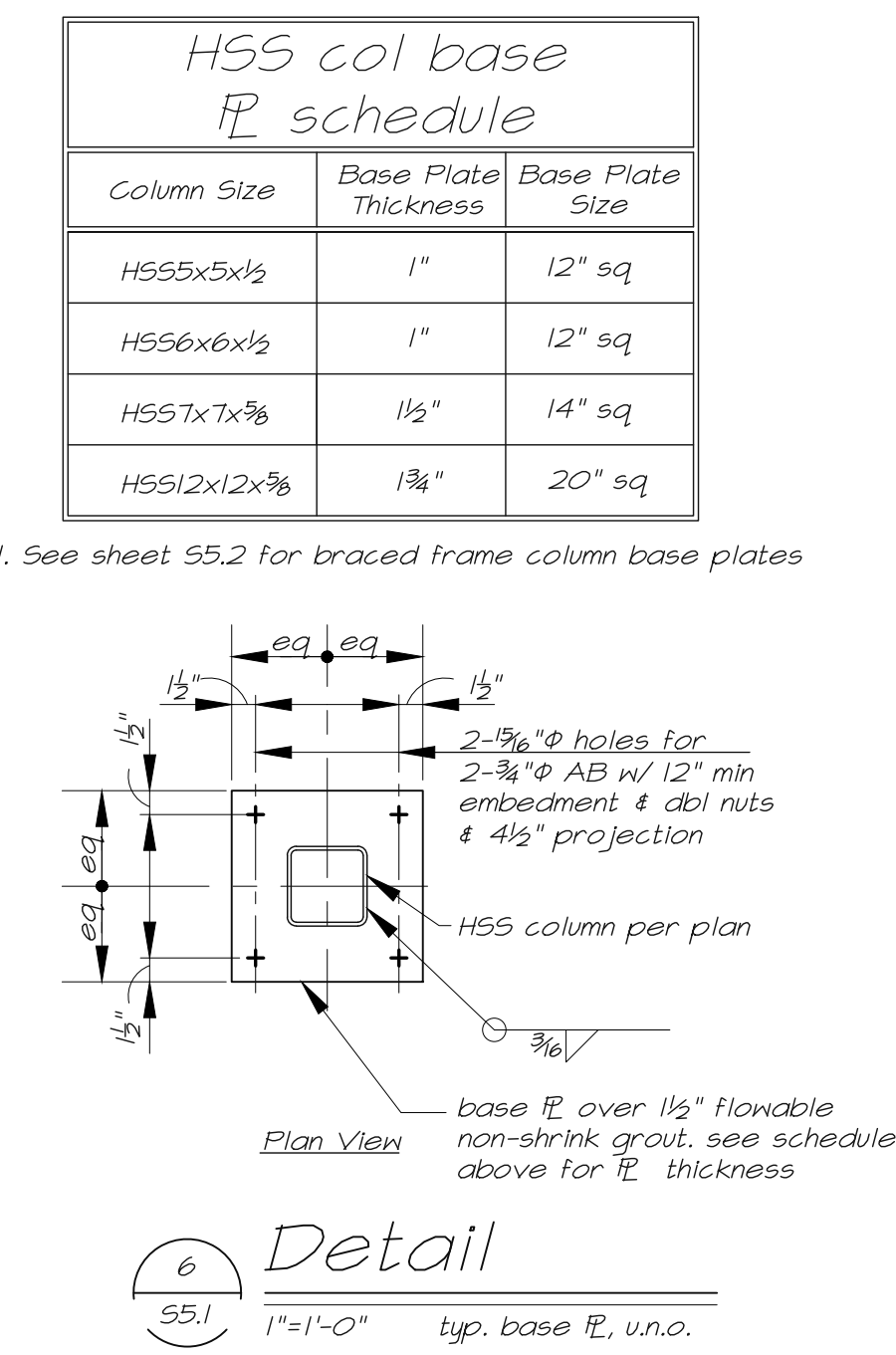
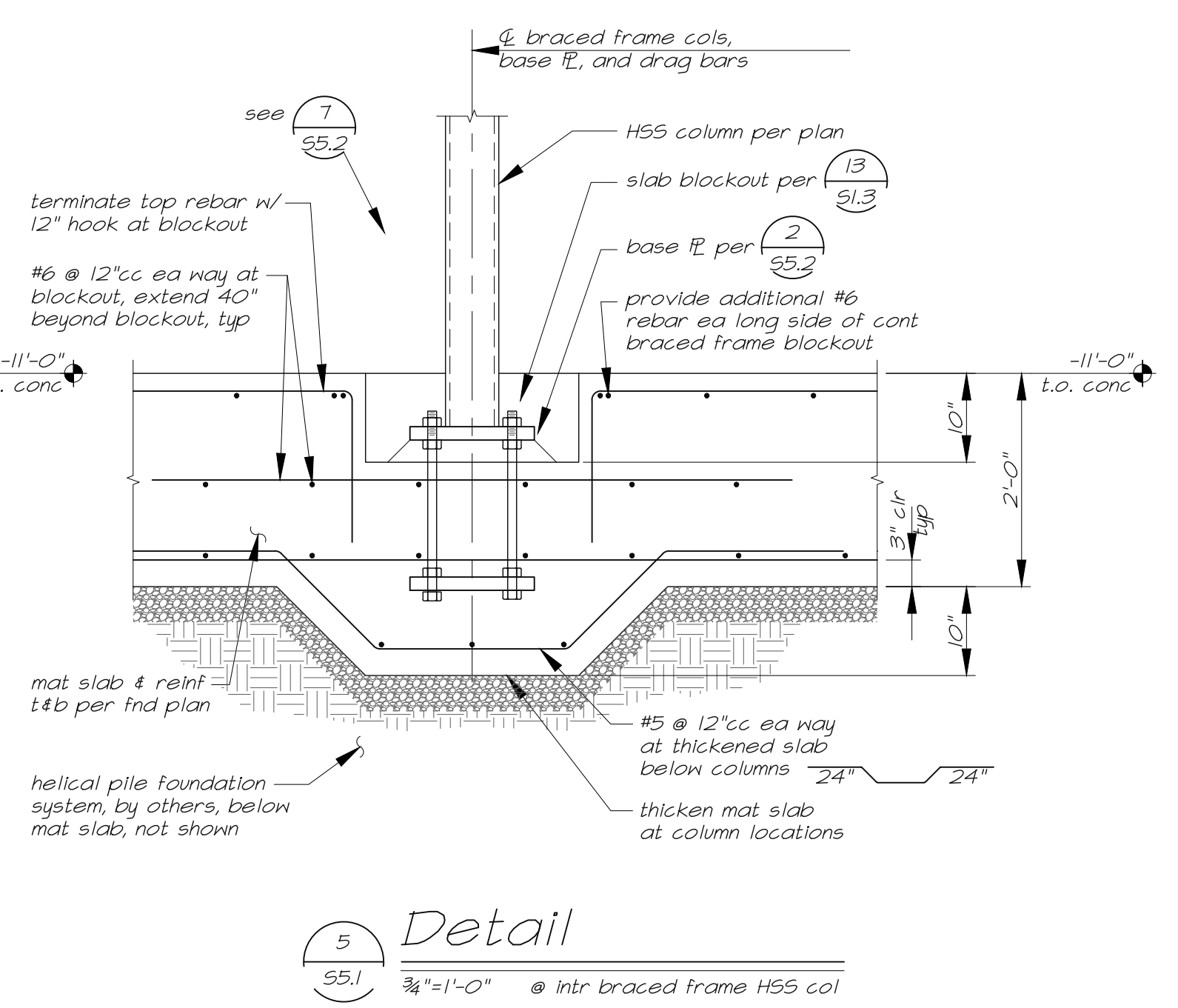
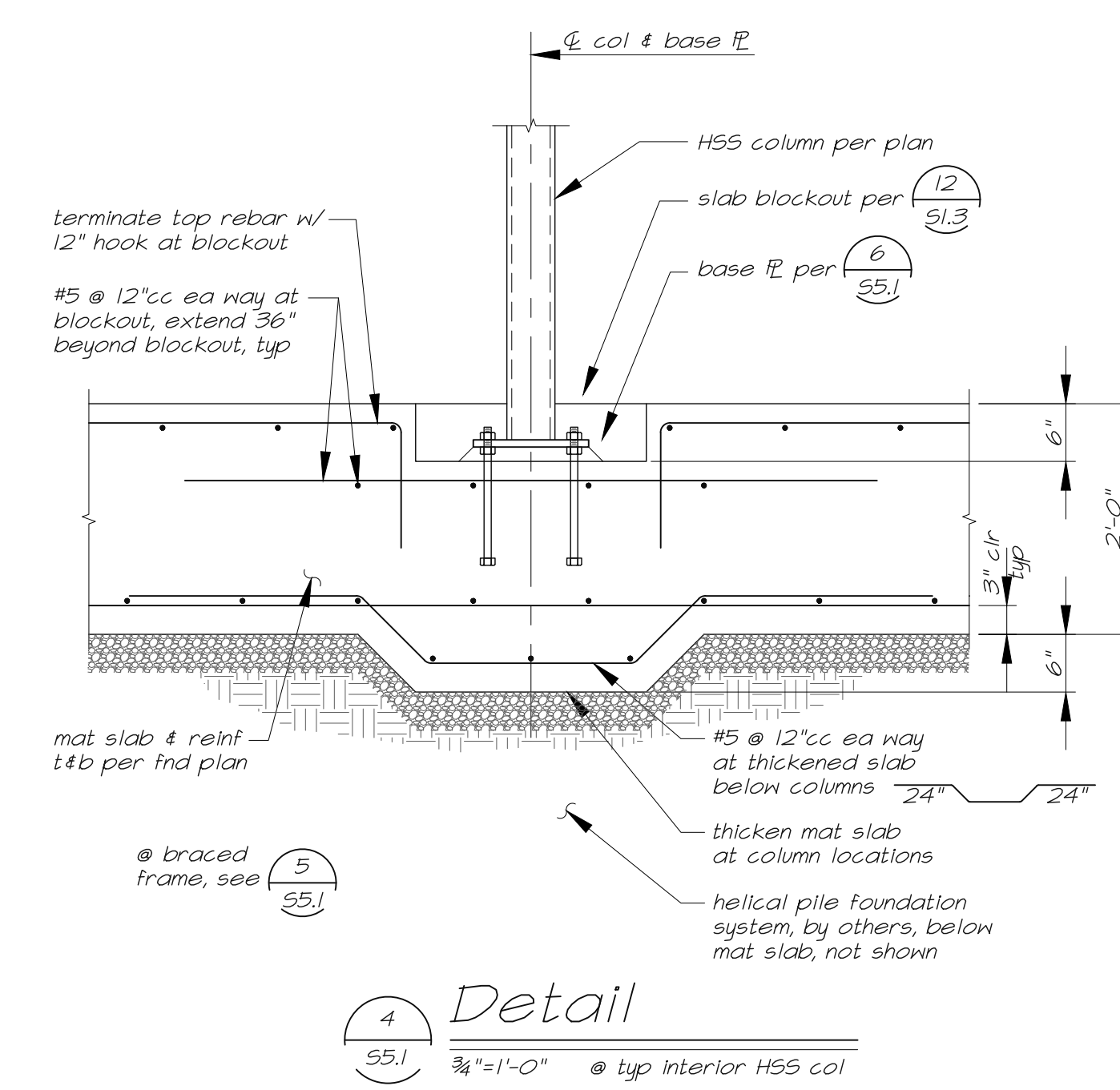
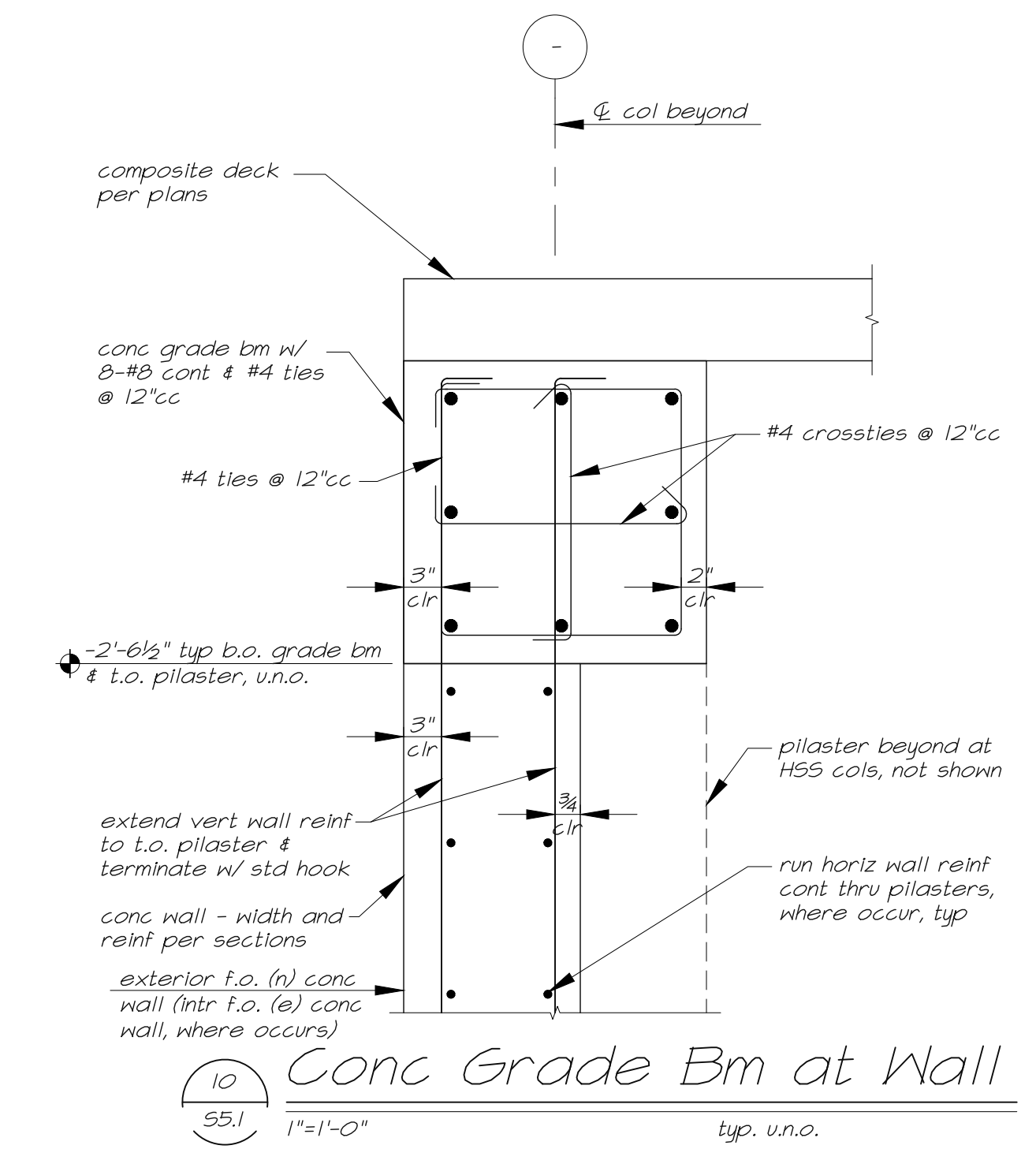
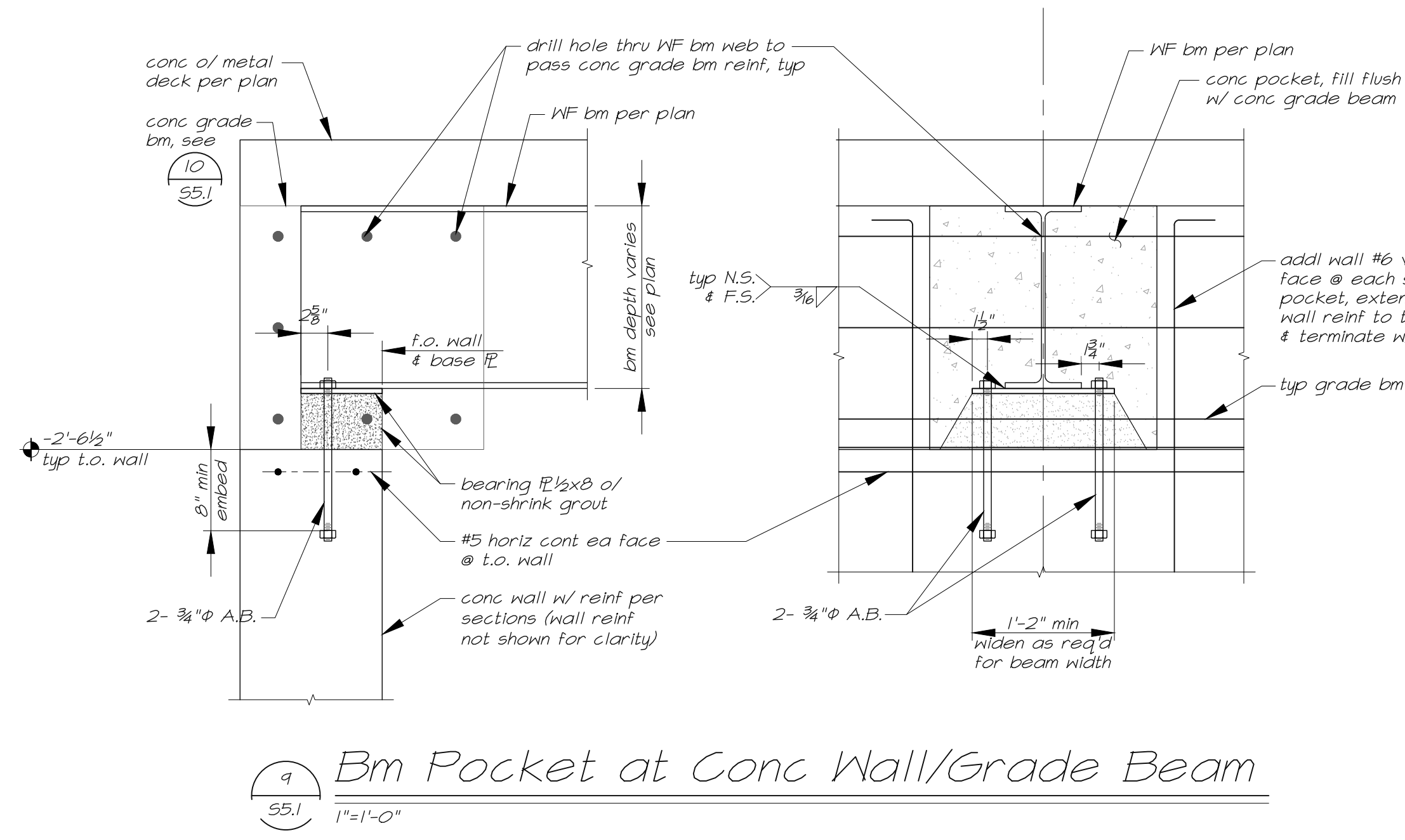
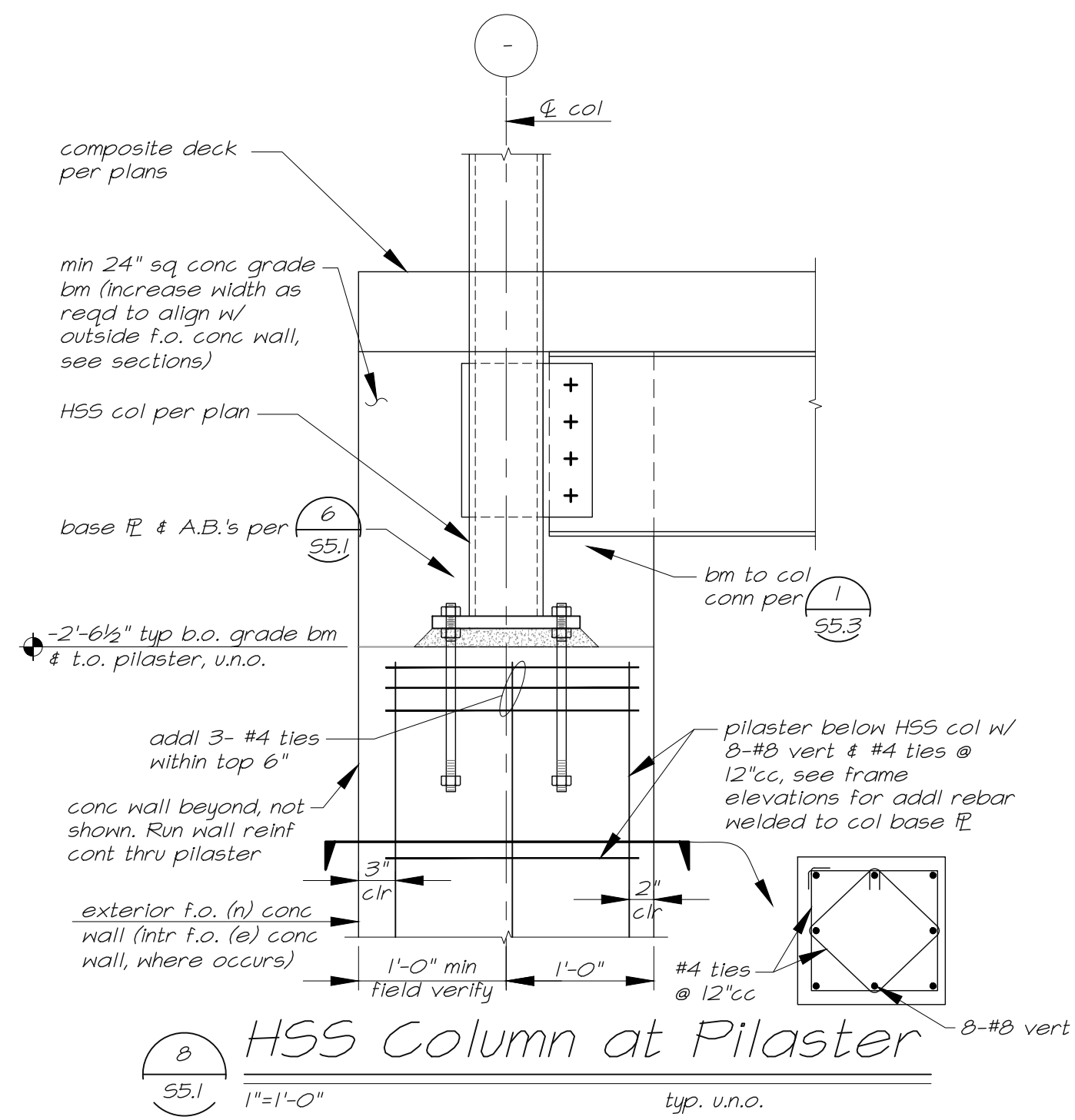
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**ELEVATOR SECTIONS**

**S4.8**





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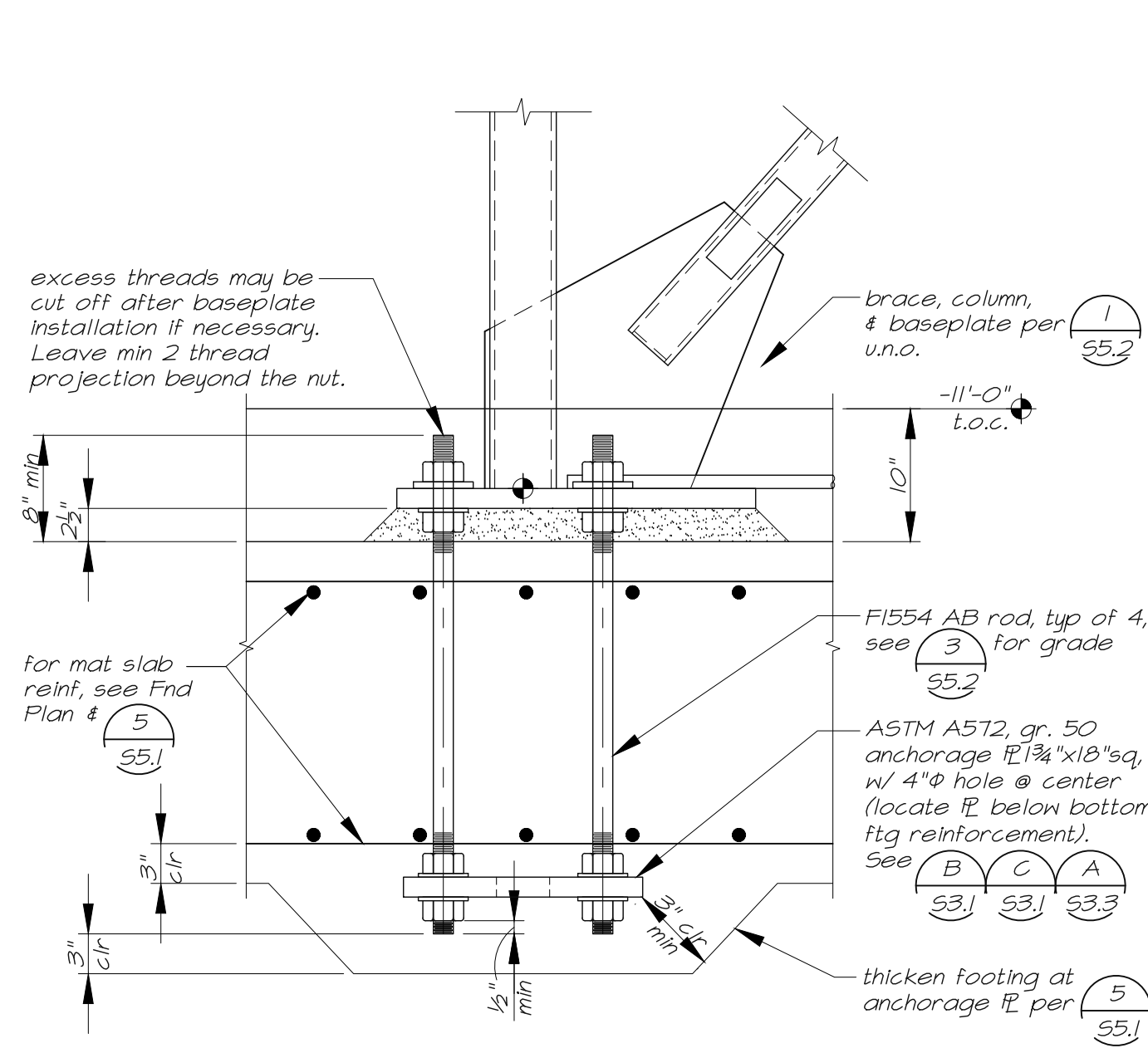
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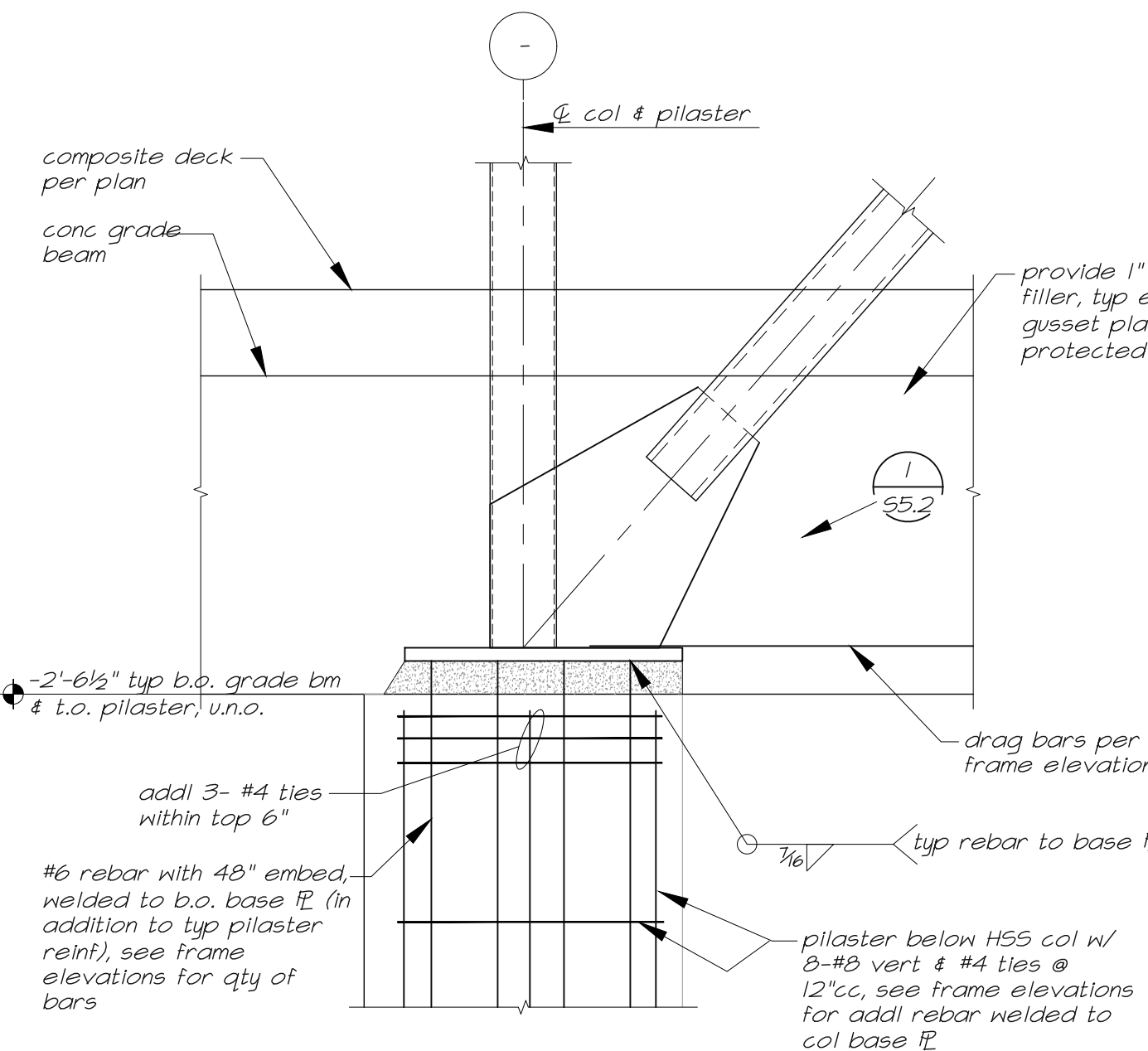
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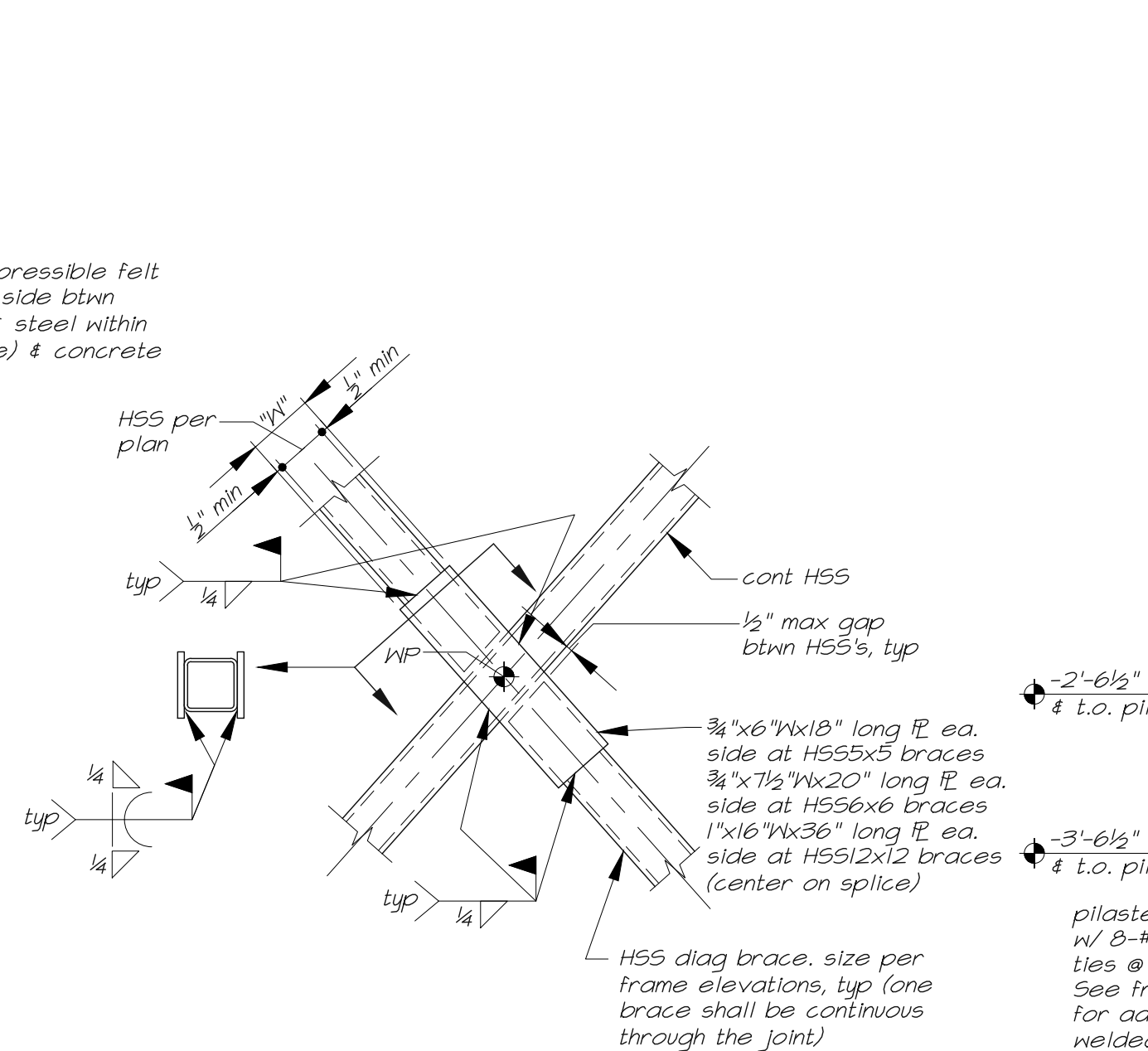




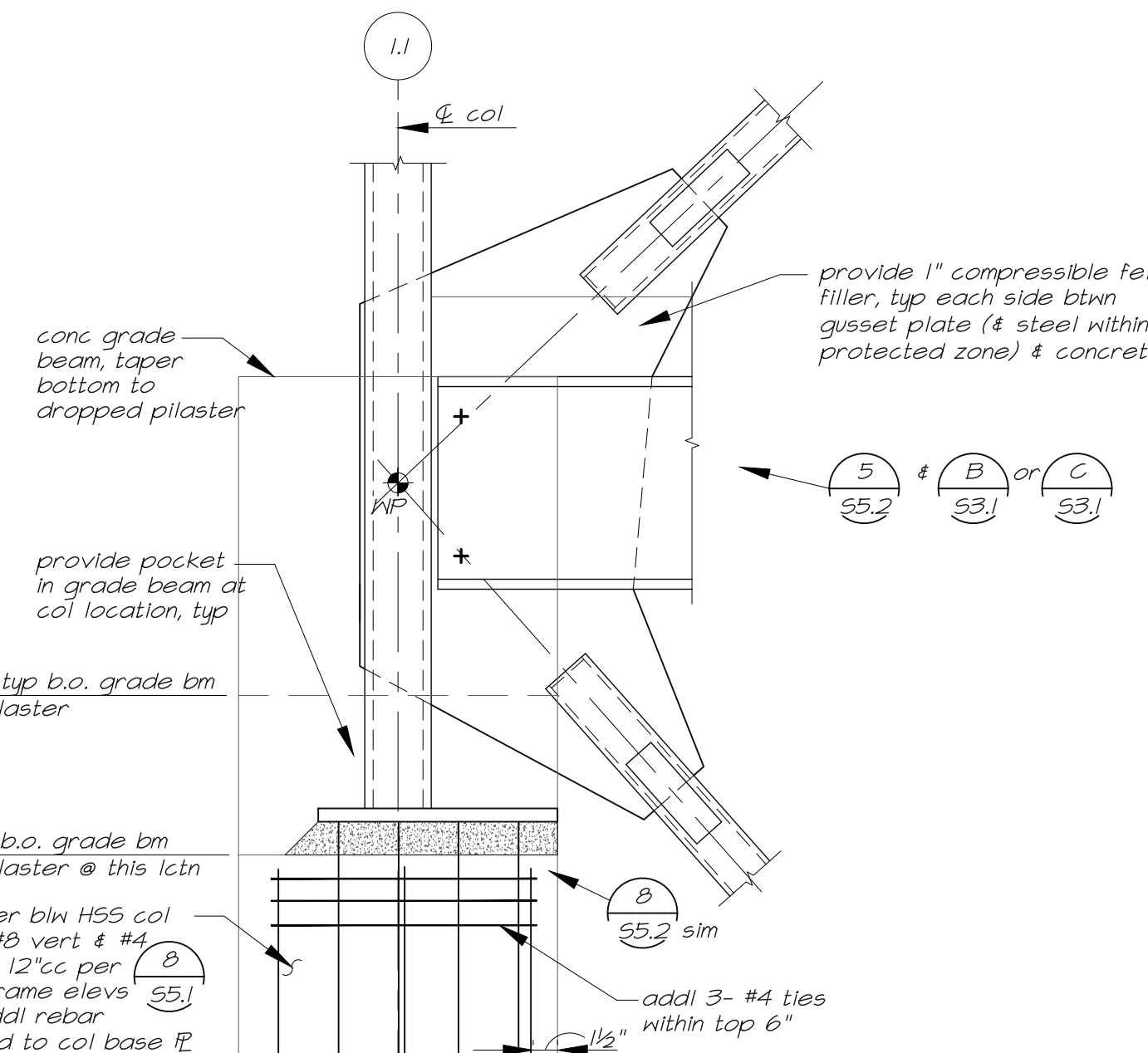
**7** Braced Frame Anchor  
1"=1'-0" HSS6x6 brace frame columns



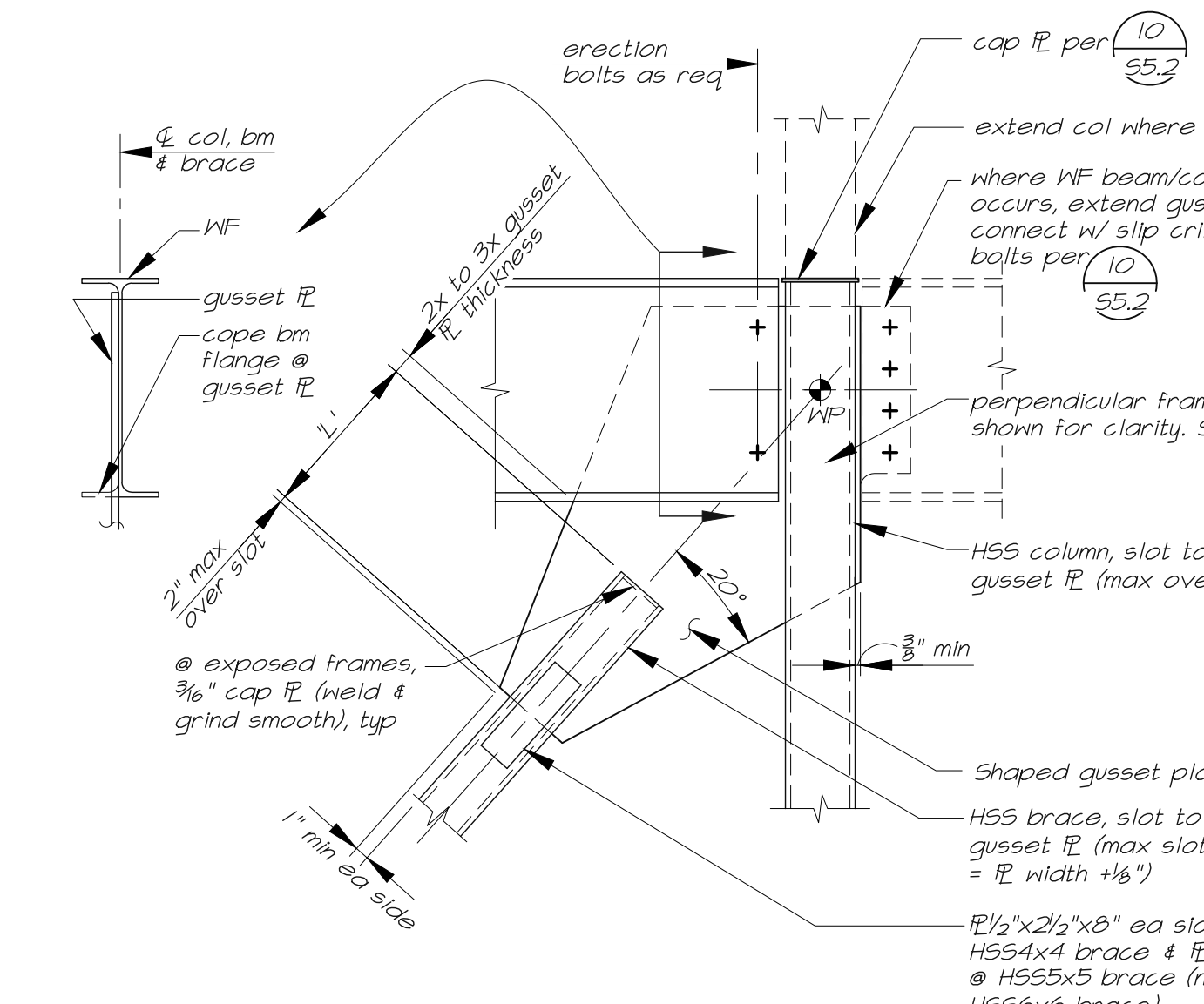
**8** Braced Frame at Pilaster  
1"=1'-0" typ. u.n.o.



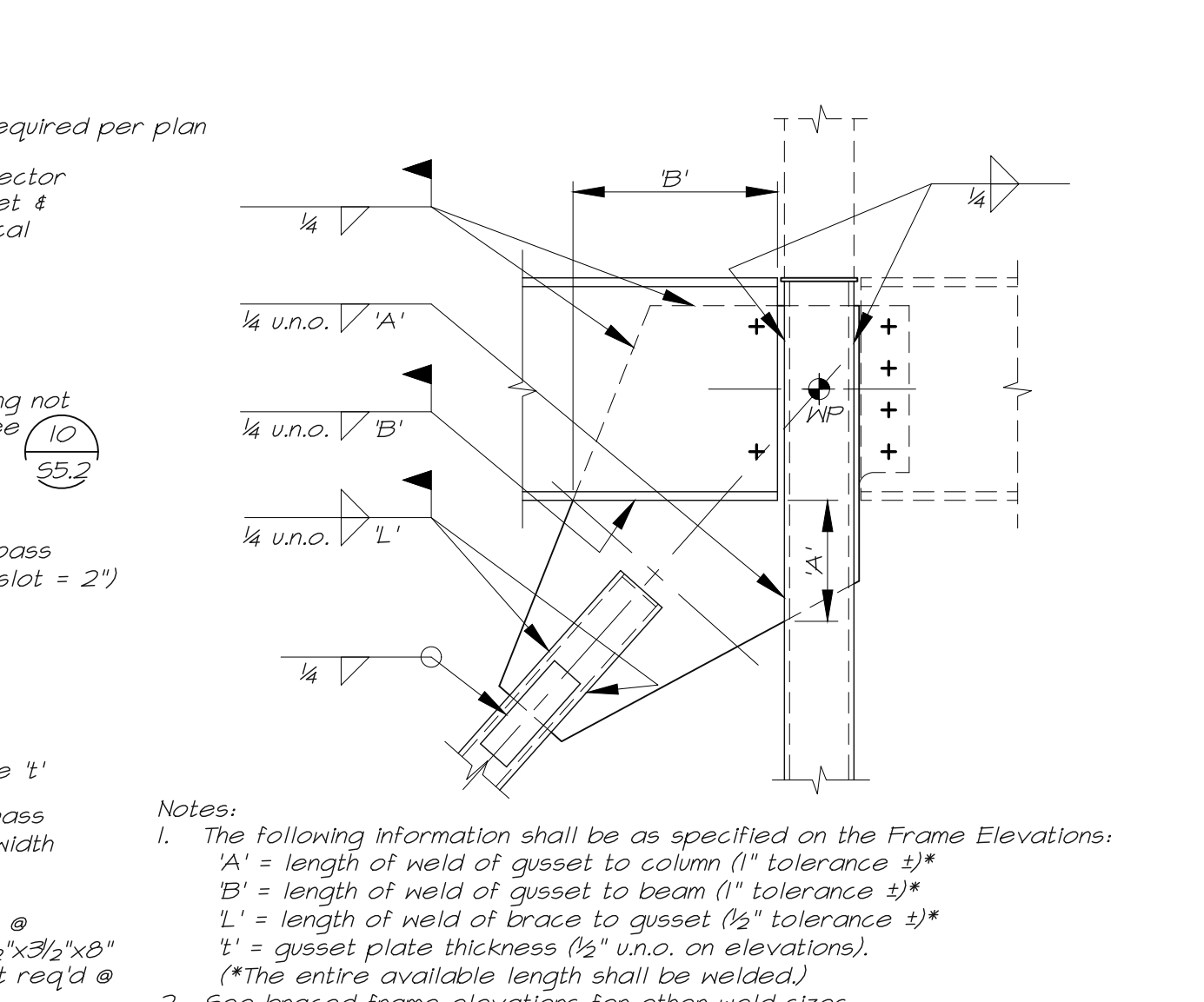
**9** Typ X-Brace Connection  
1"=1'-0"



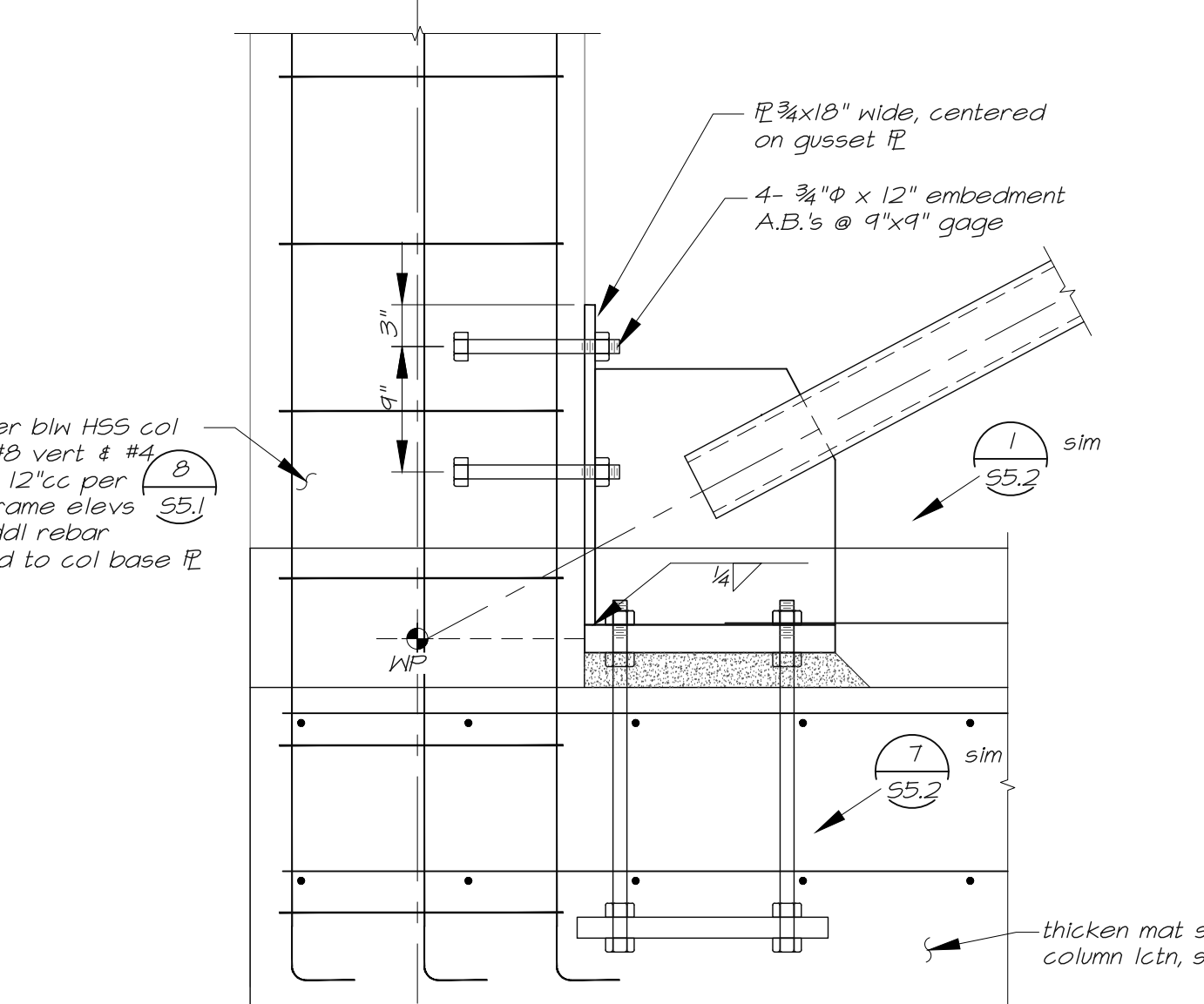
**10** Braced Frame at Pilaster  
1"=1'-0" Frame Lines D & M at Grid 1.1



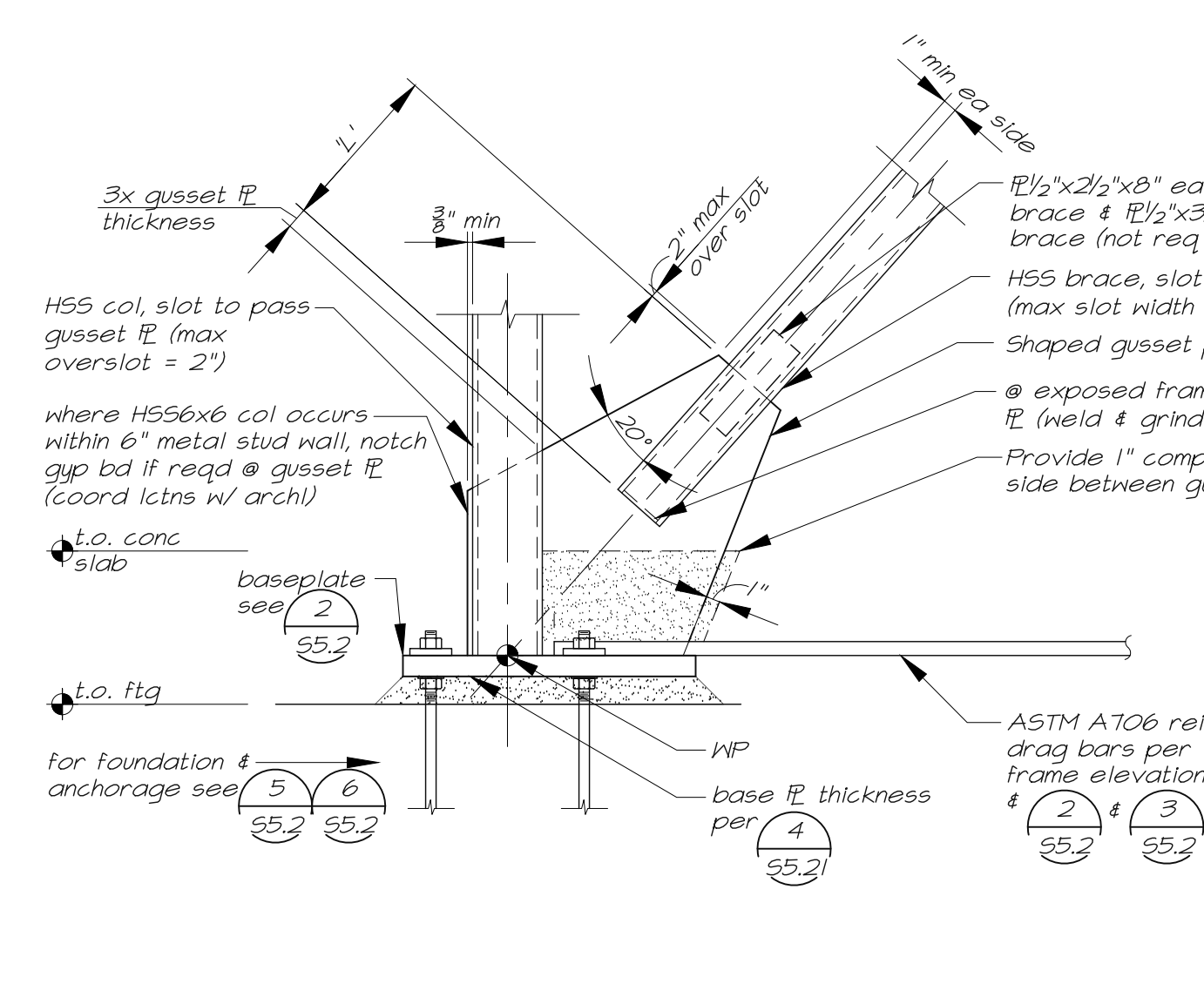
**4** Typ Braced Frame @ Column/Beam  
1"=1'-0"



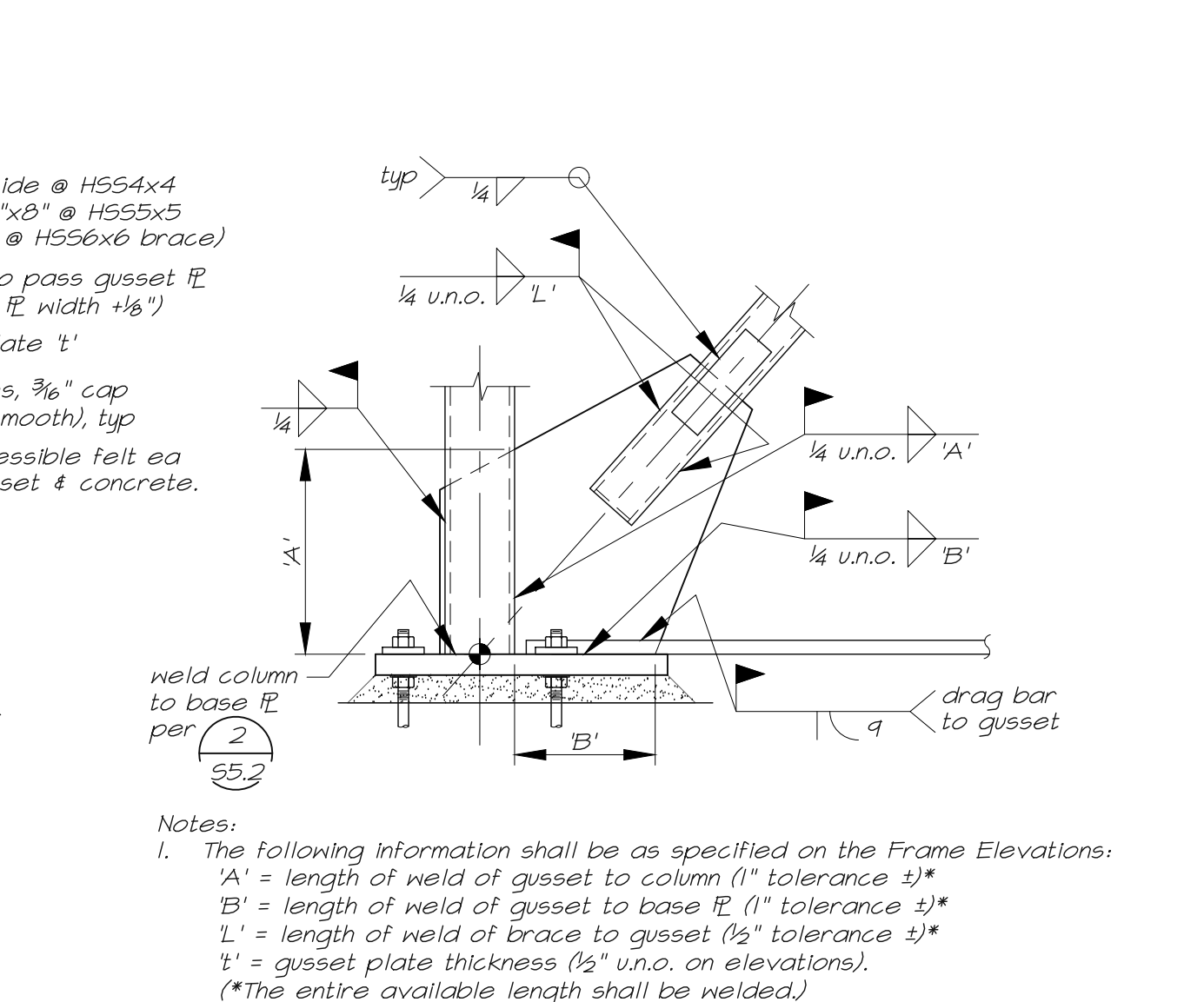
**5** Typ Brace Top & Bott @ Col  
1"=1'-0"



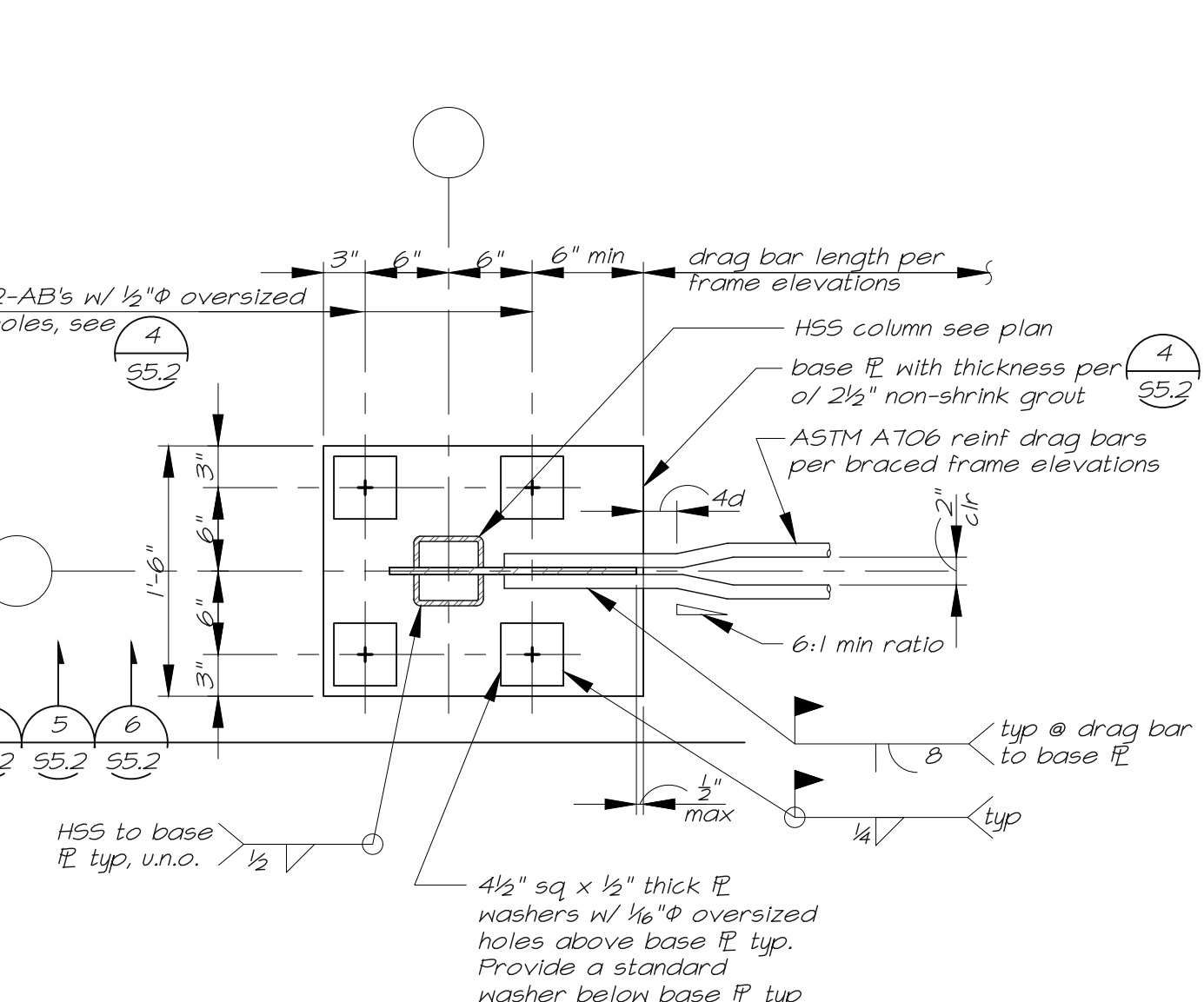
**6** Braced Frame at Pilaster  
1"=1'-0" Frame Lines D & M at Grid 1.1



**1** Typ Braced Frame at Base Plate  
1"=1'-0"



**2** Typ Base @ Braced Frame  
1"=1'-0"



**3** HSS Braced Frame Base Plate Connection Schedule

mark	column size	base PL thickness	AB size	ref. detail
1	HSS6x6x1/2	2 1/8"	(4) 1 1/2" x 24" embedment F1554, gr 55	7/55.2
2	HSS6x6x3/8	2 1/8"	(4) 1 1/2" x 24" embedment F1554, gr 55	7/55.2
3	HSS7x7x1/2	1 3/4"	n/a	8/55.2
4	HSS8x8x3/8	1 3/4"	n/a	8/55.2

Notes:  
1. The following information shall be as specified on the Frame Elevations:  
A' = length of weld of gusset to column (1" tolerance ±)  
B' = length of weld of brace to beam (1" tolerance ±)  
L' = length of weld of gusset to base PL (1" tolerance ±)  
t' = length of weld of brace to gusset (1/2" tolerance ±)  
t = gusset plate thickness (1/2" u.n.o. on elevations).  
(\*The entire available length shall be welded.)  
2. See braced frame elevations for other weld sizes.  
3. Overslot shall terminate with a radius = 1/2 the slot width, grind smooth for erection and weld purposes, typ.

Notes:  
1. The following information shall be as specified on the Frame Elevations:  
A' = length of weld of gusset to column (1" tolerance ±)  
B' = length of weld of brace to beam (1" tolerance ±)  
L' = length of weld of gusset to base PL (1" tolerance ±)  
t' = length of weld of brace to gusset (1/2" tolerance ±)  
t = gusset plate thickness (1/2" u.n.o. on elevations).  
(\*The entire available length shall be welded.)  
2. See foundation details for spread footing, slab, curb, blockouts, etc  
3. See braced frame elevations for other weld sizes.  
4. Overslot shall terminate with a radius = 1/2 the slot width, grind smooth for erection and weld purposes, typ.



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
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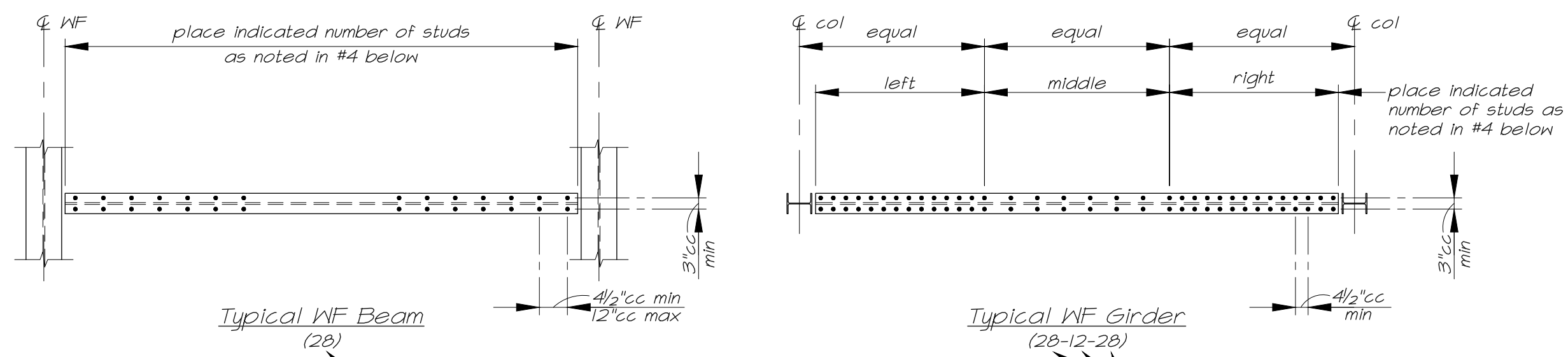
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Bid Set	11/06/2024	
Addendum #5	12/04/2024	

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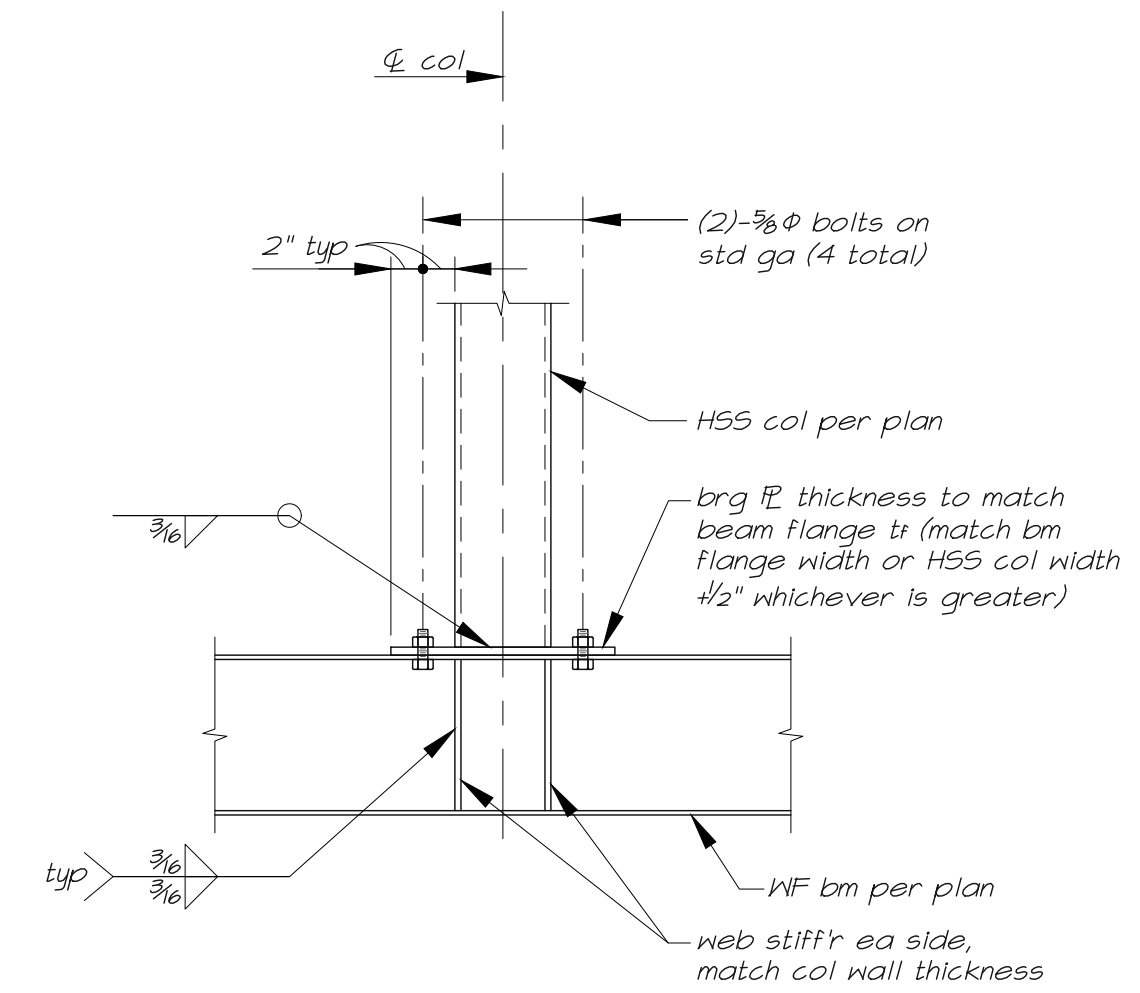
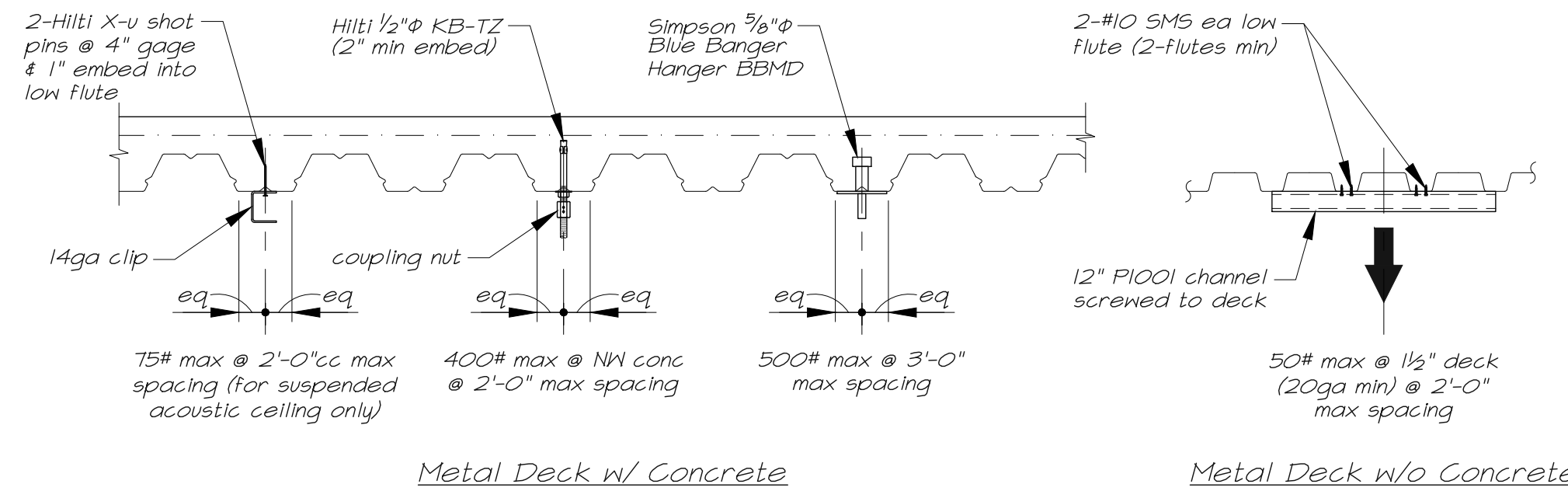
DETAILS

S5.2





- Notes:**
1. Shear connectors for composite beams shall be automatic end welded high strength 3/4" headed anchors. See "Automatic End Welded Stud Notes" on typical notes sheet.
  2. All welding of shear connectors shall conform to the latest edition for "Recommended Practices for Stud Welding" and "Structural Welding Code" published by the American Welding Society.
  3. Shear connectors shall project a minimum of 1/2" above the top of the metal deck and shall be held a minimum of 3/4" clear from top of the concrete slab.
  4. Shear connectors shall be uniformly spaced. Use no more than one stud per rib where the number of shear connectors required is less than or equal to the number of ribs available. Where the number of shear connectors exceeds the number of ribs available, place one shear connector per rib beginning at the supports at each end and moving toward mid-span until the required number of shear connectors are supplied.
  5. Where not indicated otherwise, provide 3/4" shear connectors @ 8"cc maximum spacing @ all beams receiving metal deck and concrete fill.
  6. Contractor shall submit show drawings for review prior to install.



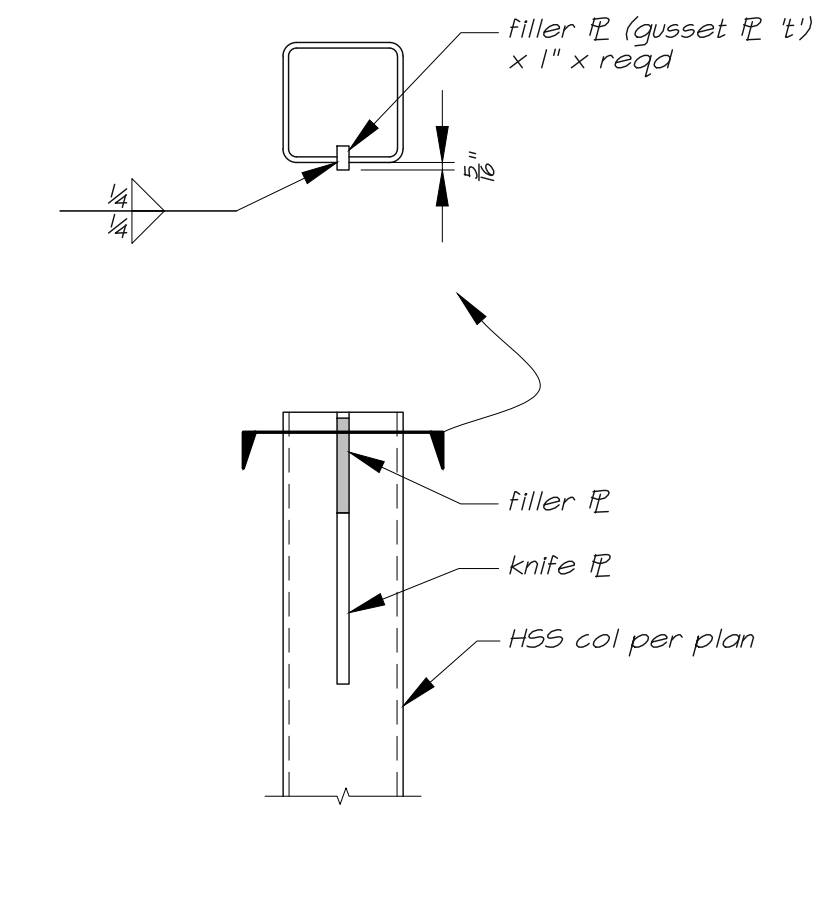
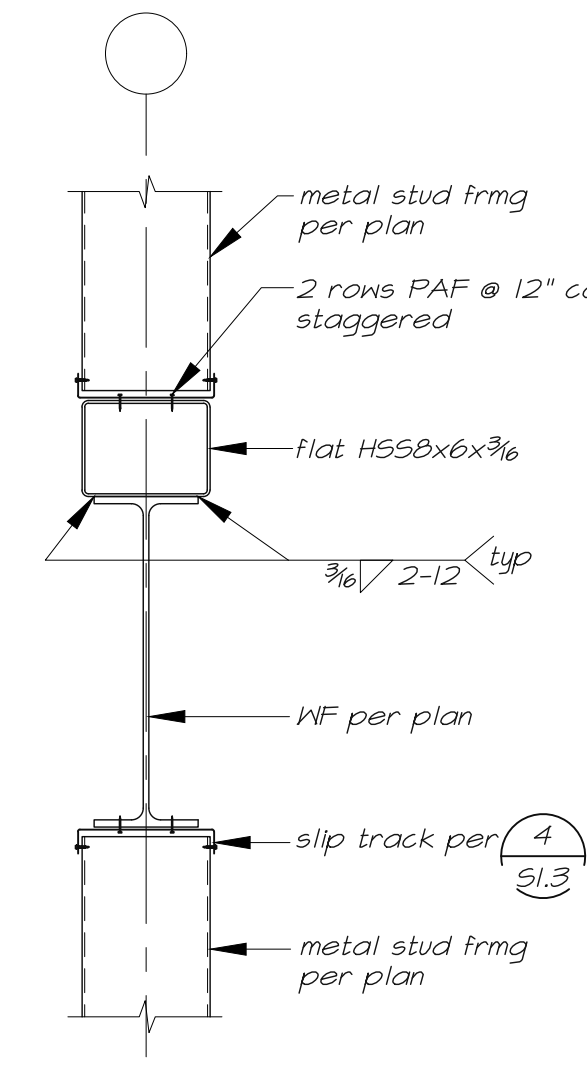
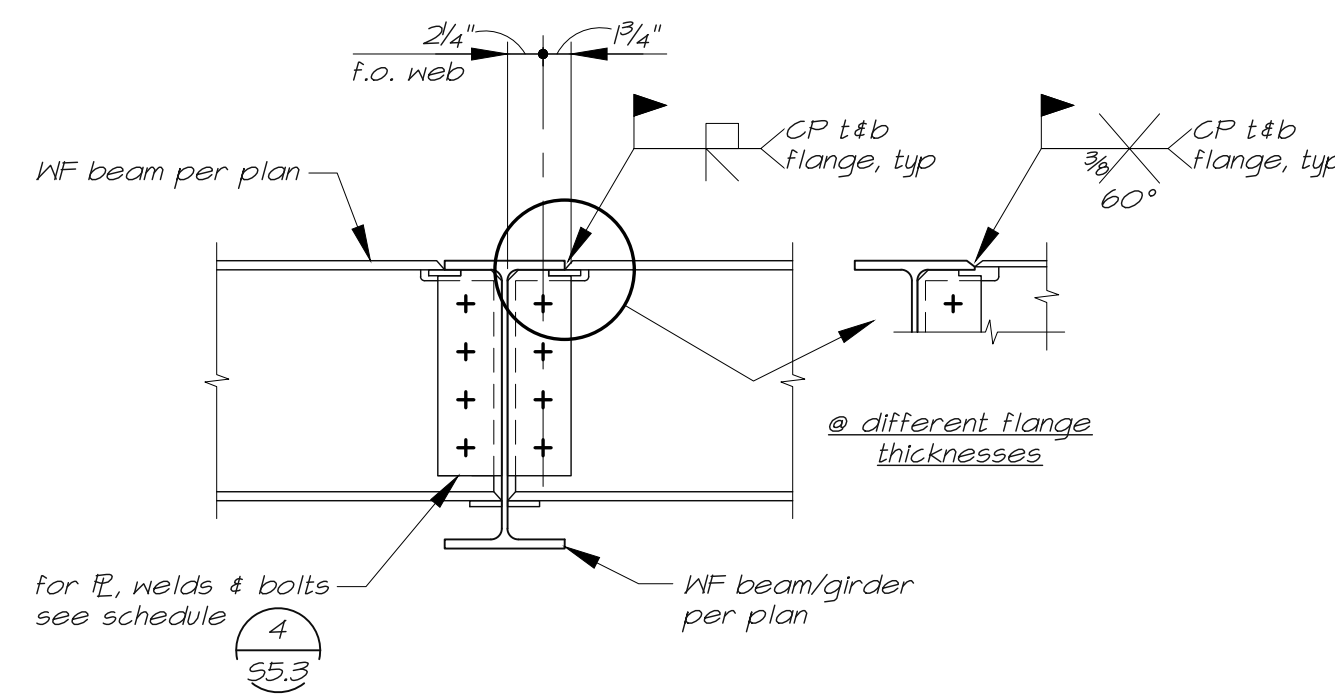
8 Shear Connector Layout  
S5.3 1/4"=1'-0"

9 Detail  
S5.3 1"=1'-0" hangers @ metal deck

10 HSS Col to Bm Conn  
S5.3 1"=1'-0"

bm size	No. & Dia. A325-N Bolts per row, u.n.o.	shear fl thickness	W
CB, CB & C10	2 - 3/8"	1/4"	1/4"
WB & W10	2 - 3/8"	1/4"	1/4"
W12 & W14	3 - 7/8"	3/16"	1/4"
W16	4 - 3/8"	3/16"	1/4"
W18	5 - 3/8"	3/16"	1/4"
W21	5 - 7/8"	3/16"	3/16"
W24	6 - 3/8"	1/2"	3/16"
W27	7 - 3/8"	1/2"	3/16"
W30	7 - 7/8"	1/2"	3/8"
W33	8 - 3/8"	1/2"	3/8"

- Notes:**
1. Use A325-N bolts at connections, typ. A325 SC group A bolts are to be used at specific locations as indicated on framing plans. Use multiple rows of no. & dia. shown in schedule to achieve total number of bolts specified on plans. All slip critical connections shall have full-depth shear plates.

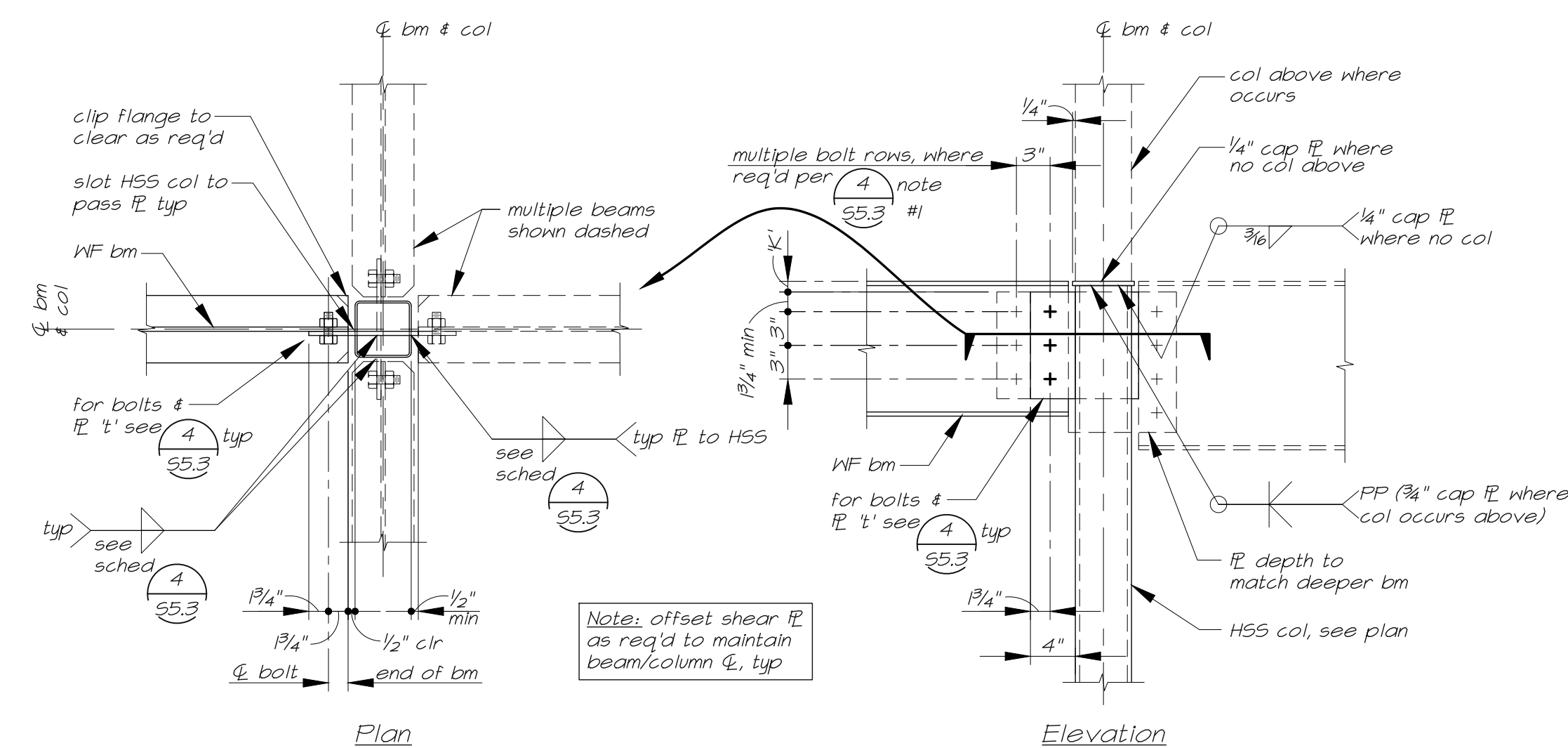


4 Connection Schedule  
S5.3 n.t.s.

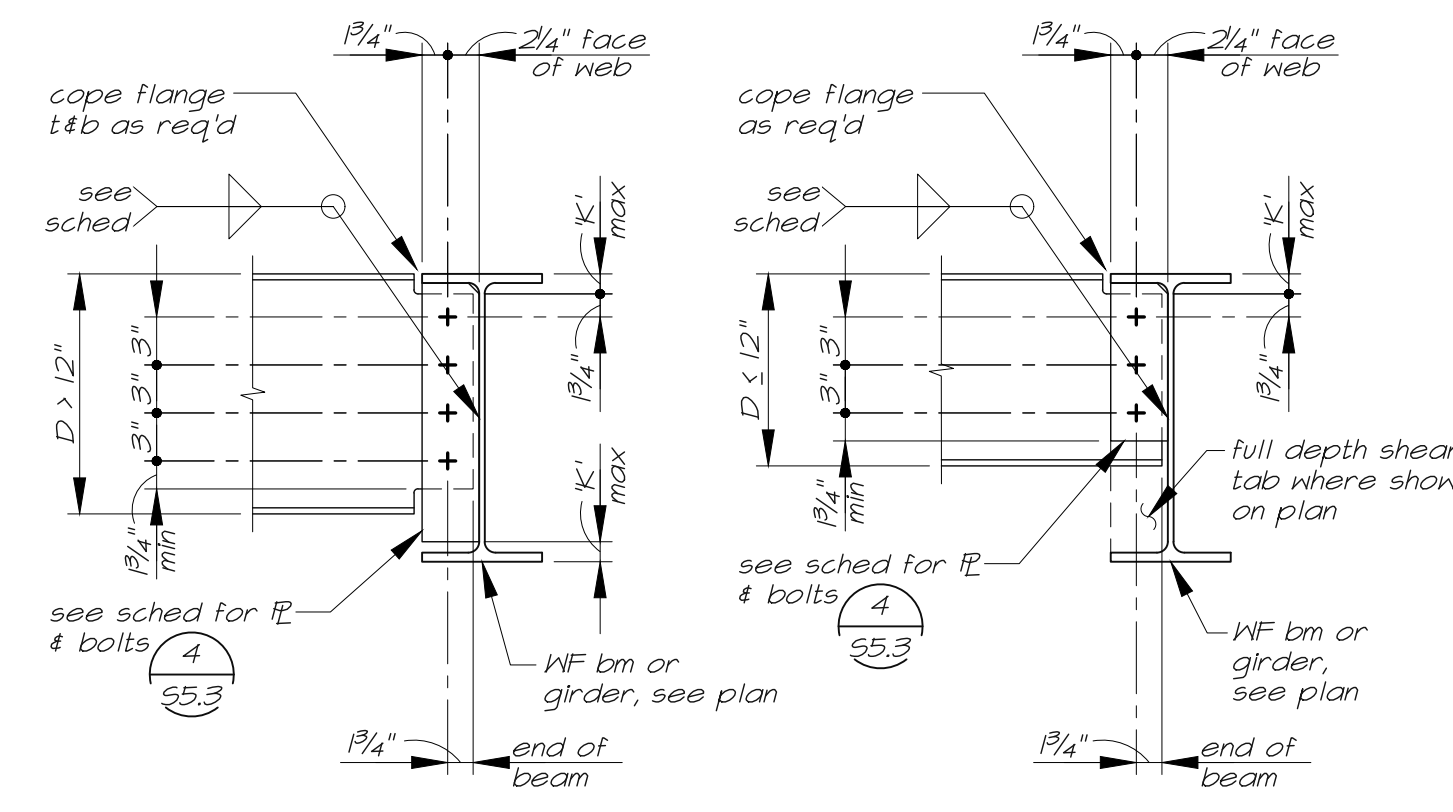
5 Detail  
S5.3 1"=1'-0" moment connection - beam to beam

6 Detail  
S5.3 1"=1'-0" wind beam @ stairwell

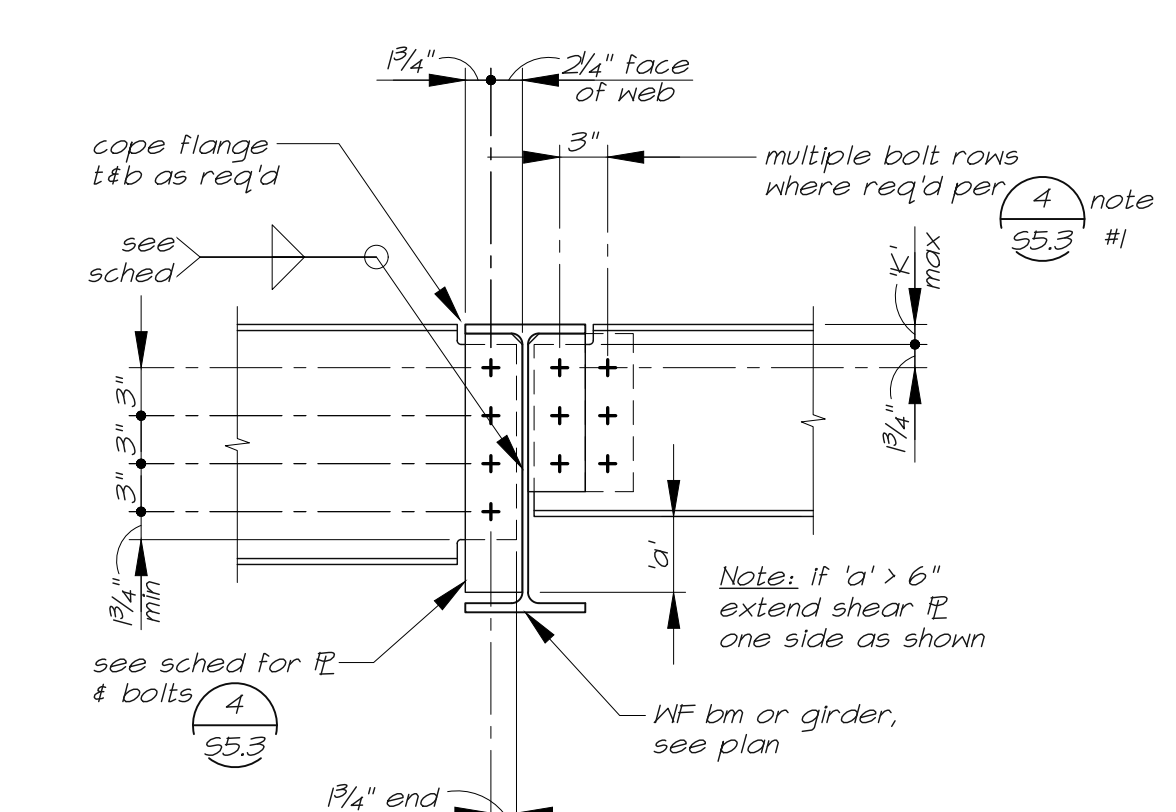
7 Detail  
S5.3 1"=1'-0" filler fl @ overslot



1 Beam to Column Connection  
S5.3 1"=1'-0" one or multiple sides



2 Beam to Beam Connection  
S5.3 1"=1'-0" one sided



3 Beam to Beam Connection  
S5.4 1"=1'-0" two sided



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Stockton, California 95270  
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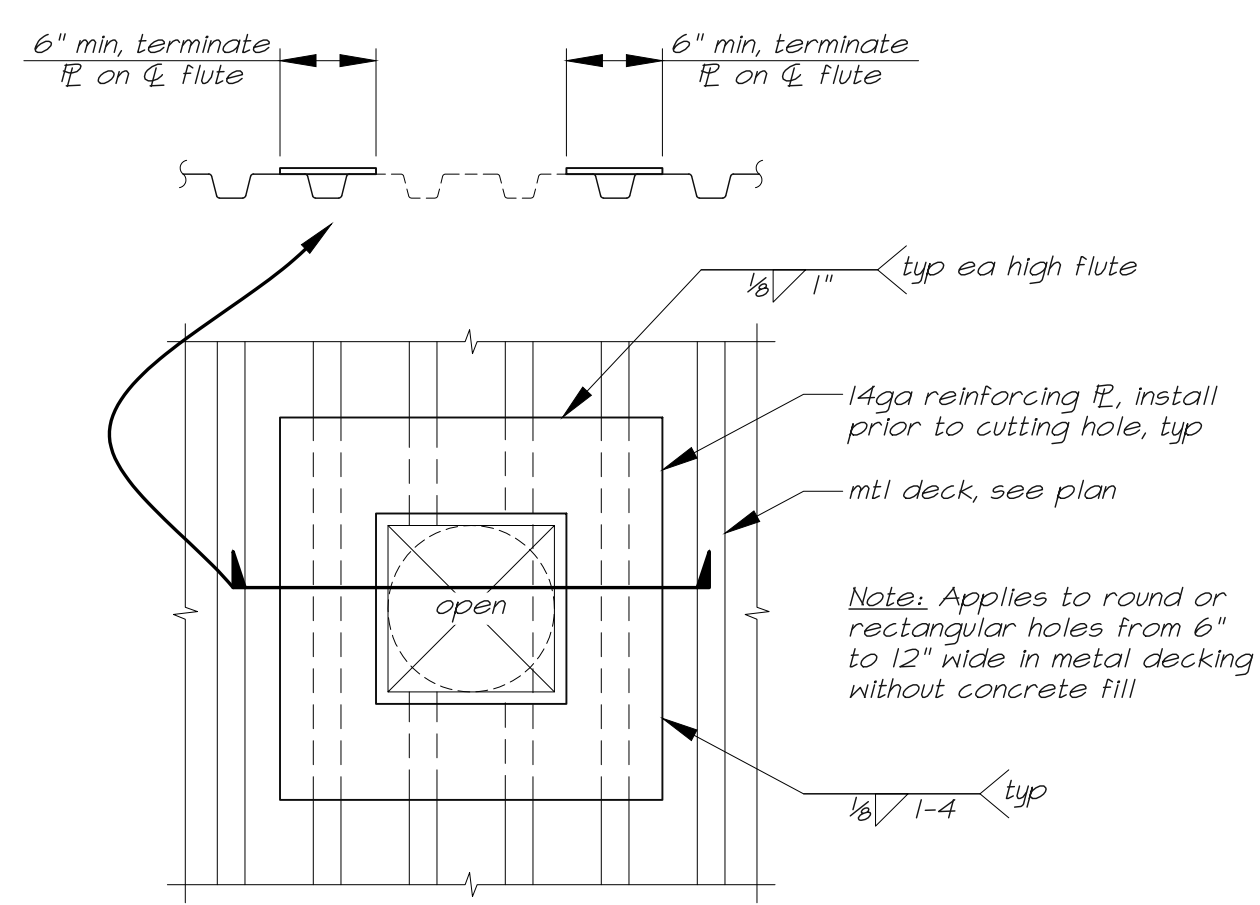
DETAILS

S5.3

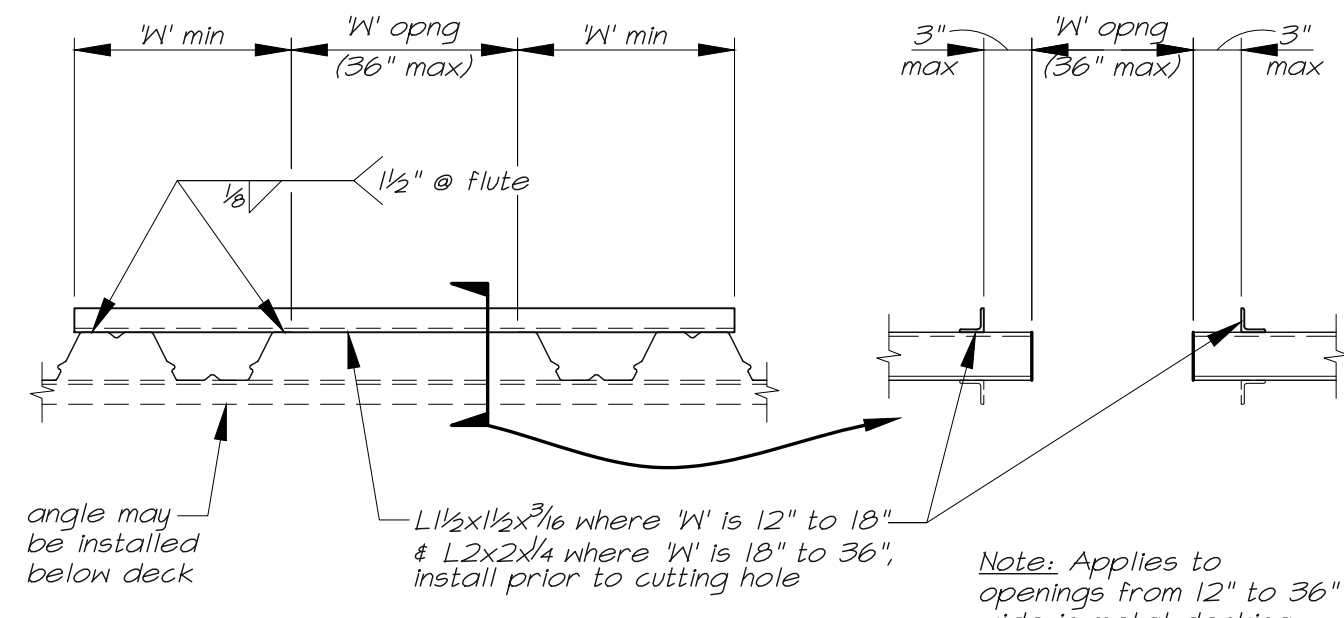




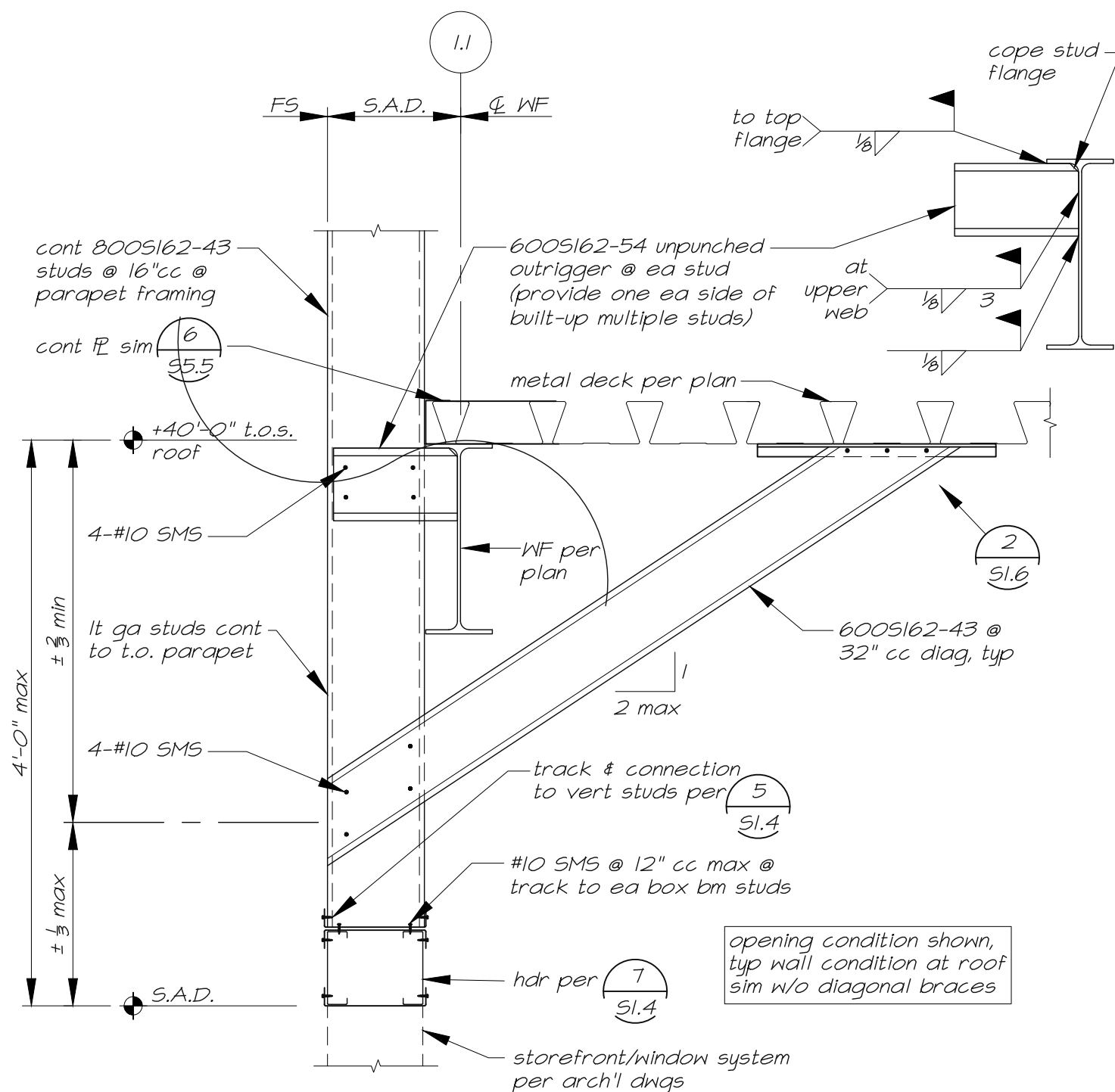




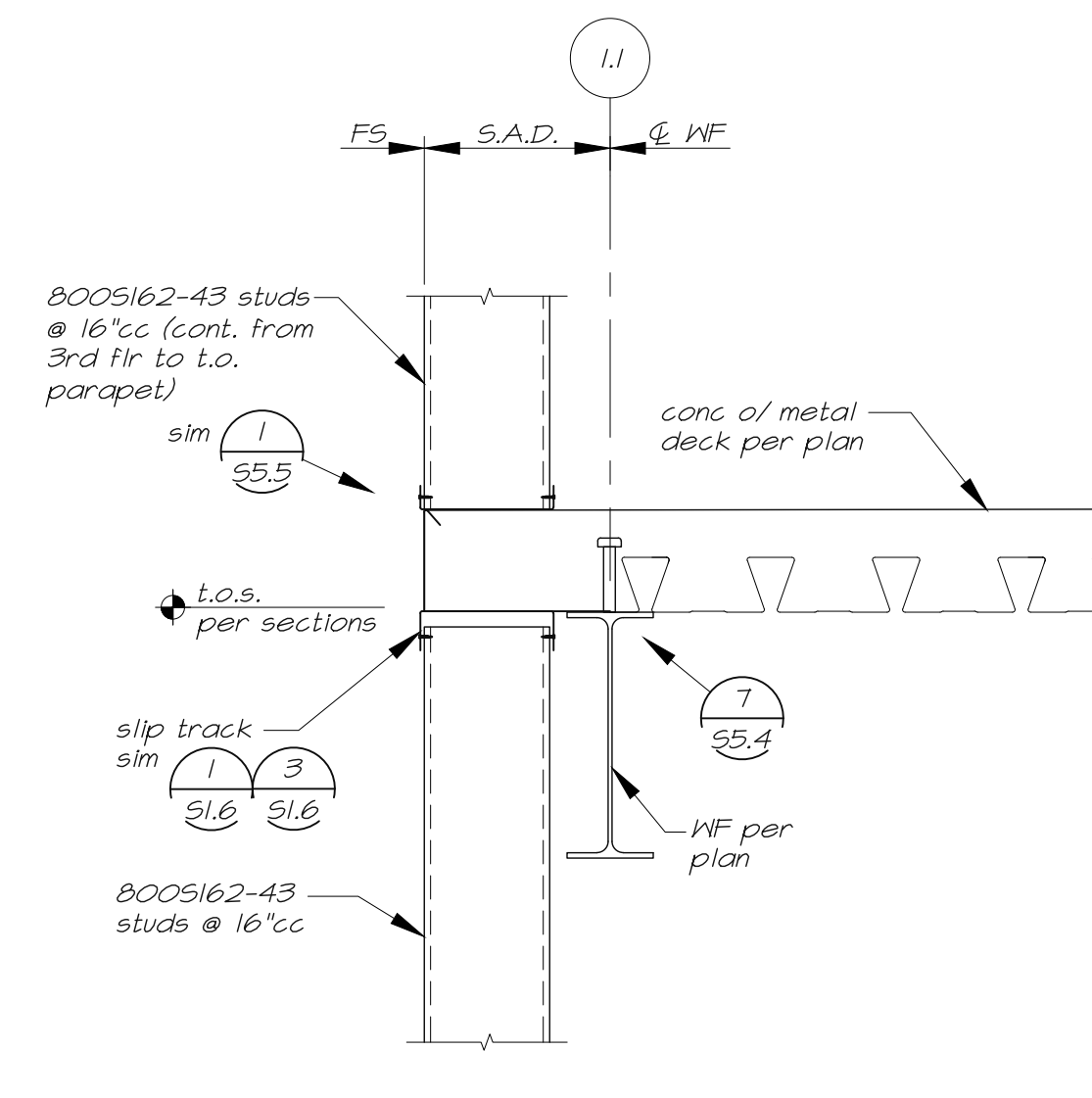
**8** Opening in Metal Deck  
 55.5 1"=1'-0" w/o conc fill (openings from 6" to 12")



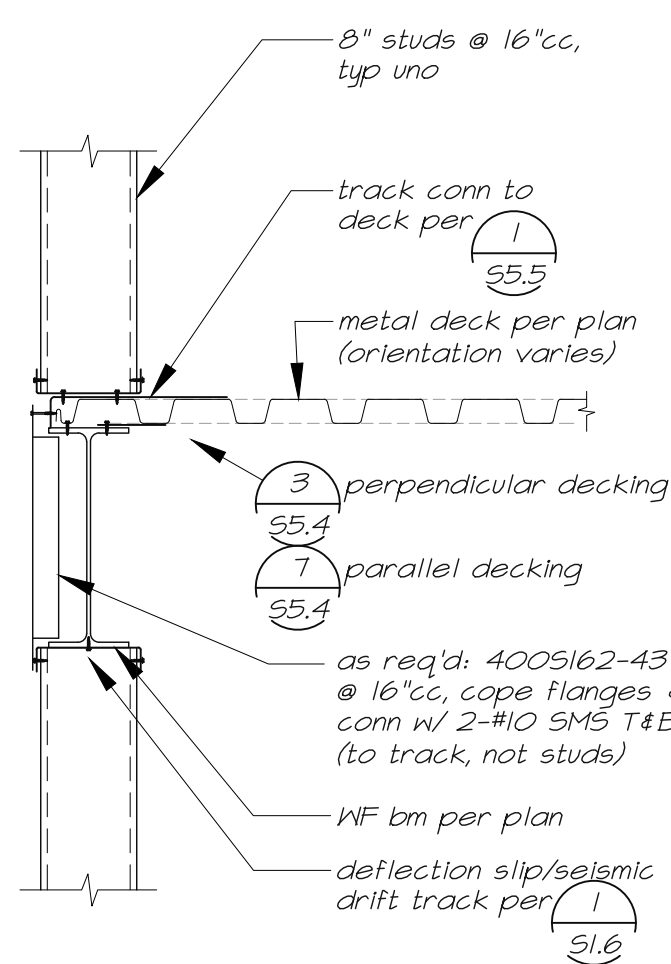
**9** Opening in Metal Deck  
 55.5 1"=1'-0" w/o conc fill (opng from 12" to 36")



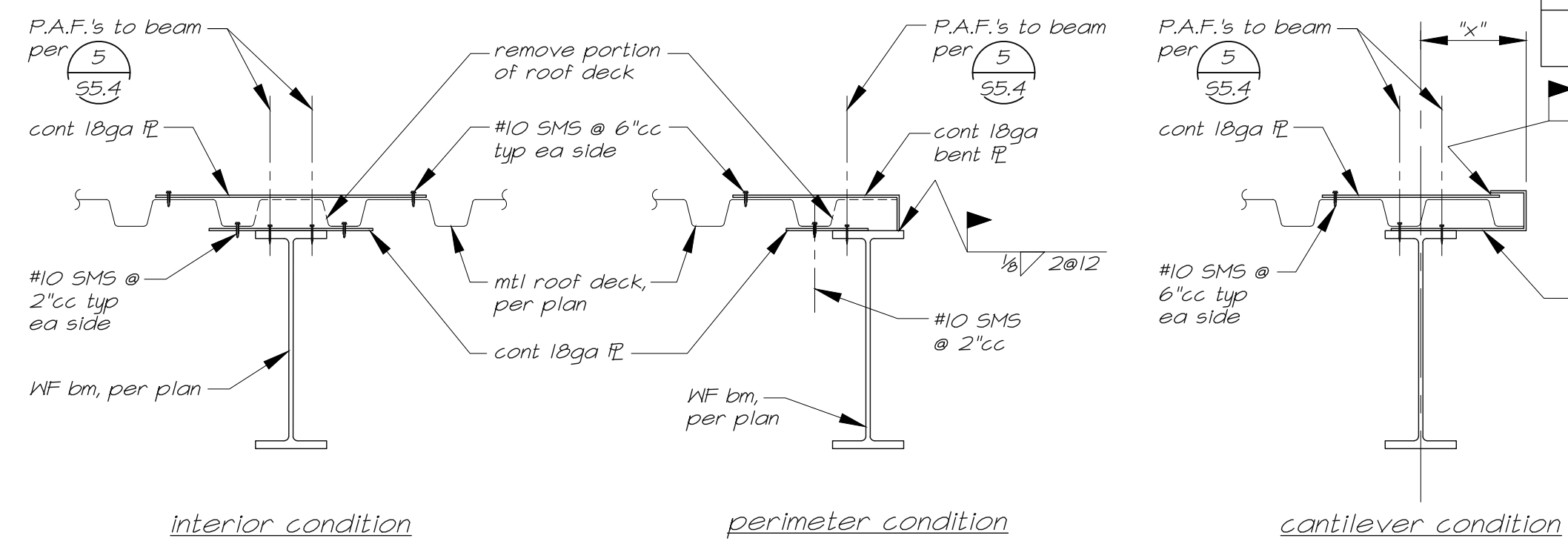
**10** Detail  
 55.5 1"=1'-0" grid I.I. roof parapet at header



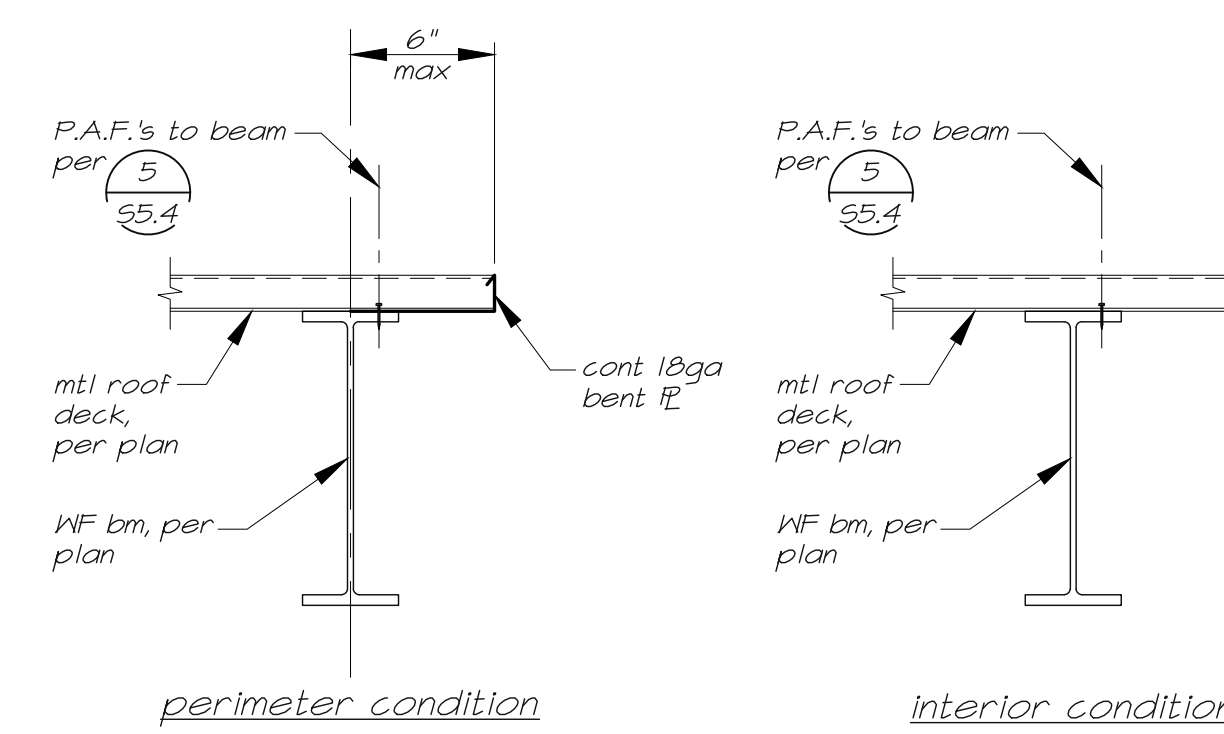
**11** Detail  
 55.5 1"=1'-0" grid I.I. extr. curtain wall @ fir



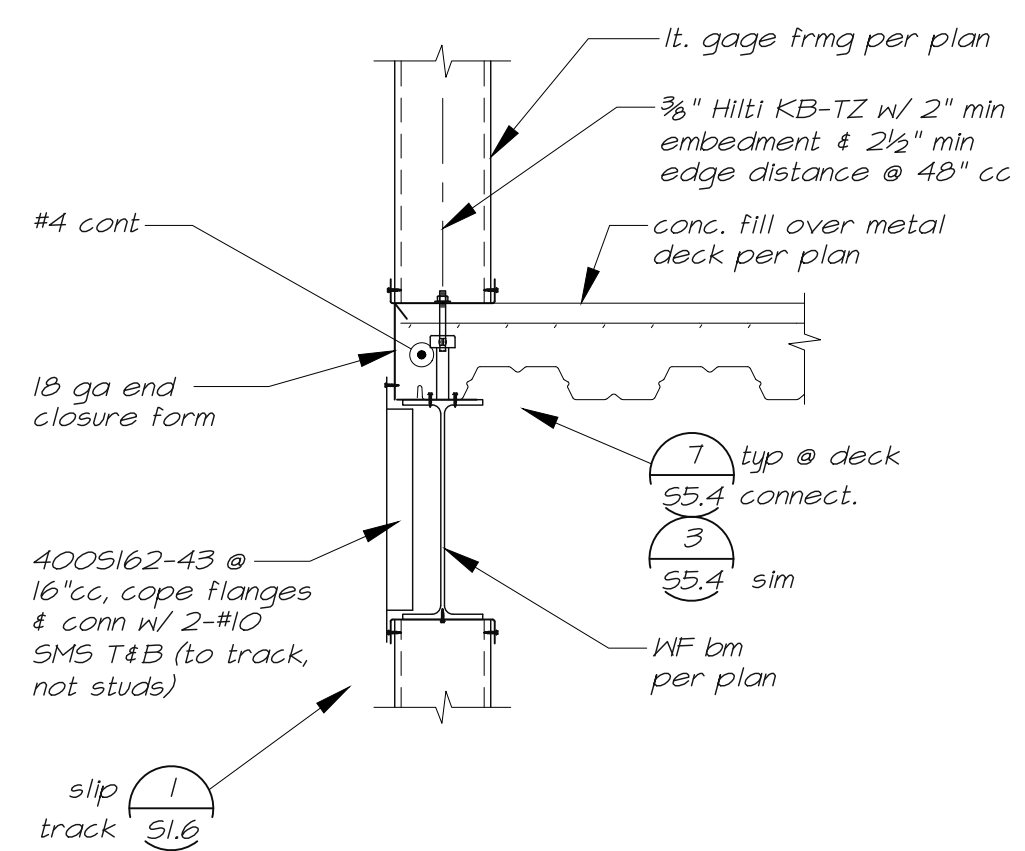
**5** Detail  
 55.5 1"=1'-0" stud to structure at roof



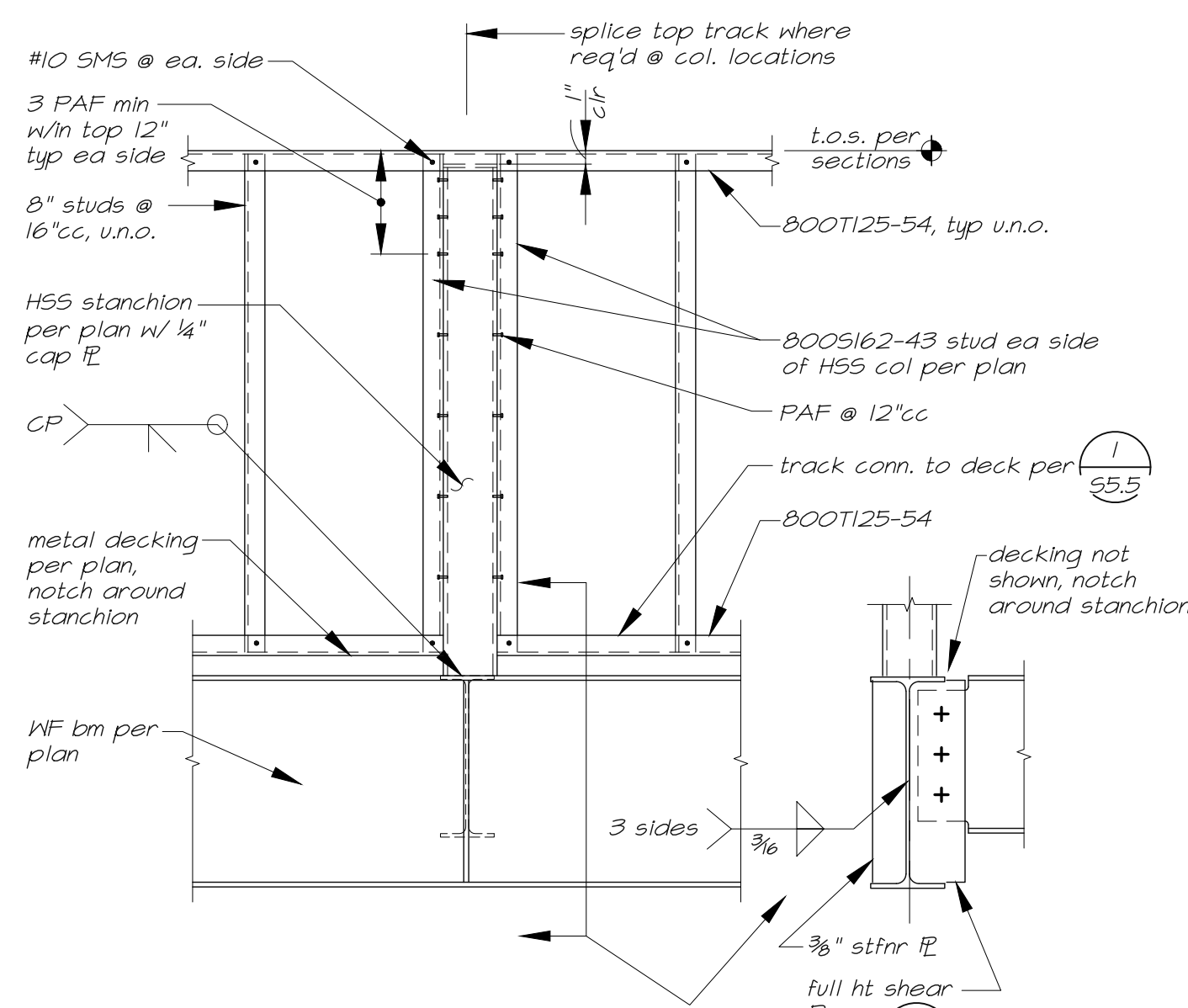
**6** Detail  
 55.5 1/2"=1'-0" roof deck parallel to beam



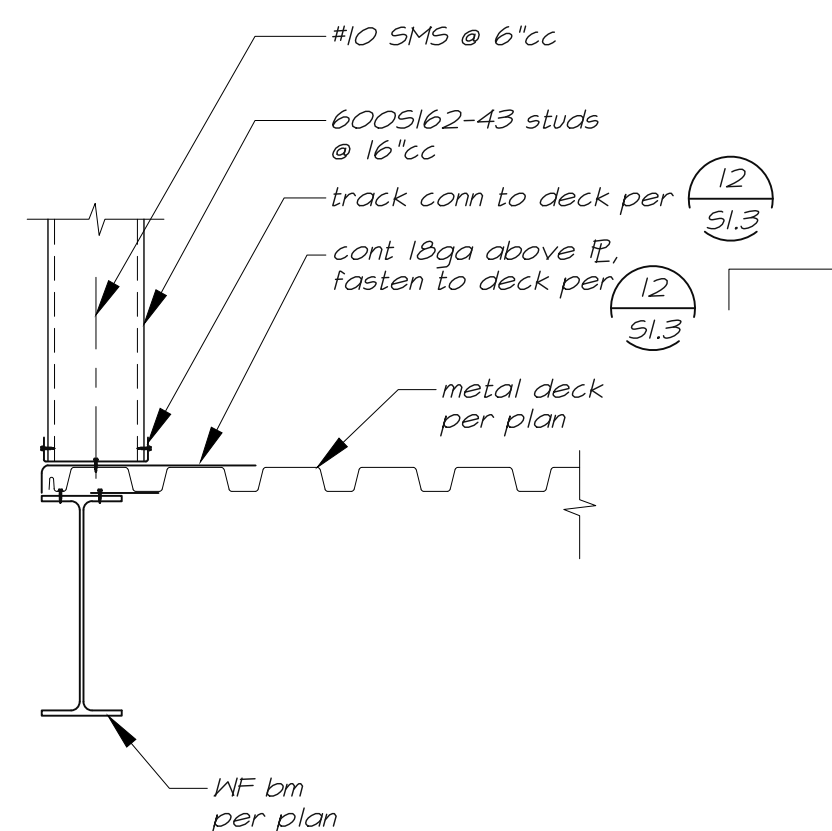
**7** Detail  
 55.5 1/2"=1'-0" roof deck perpendicular to beam



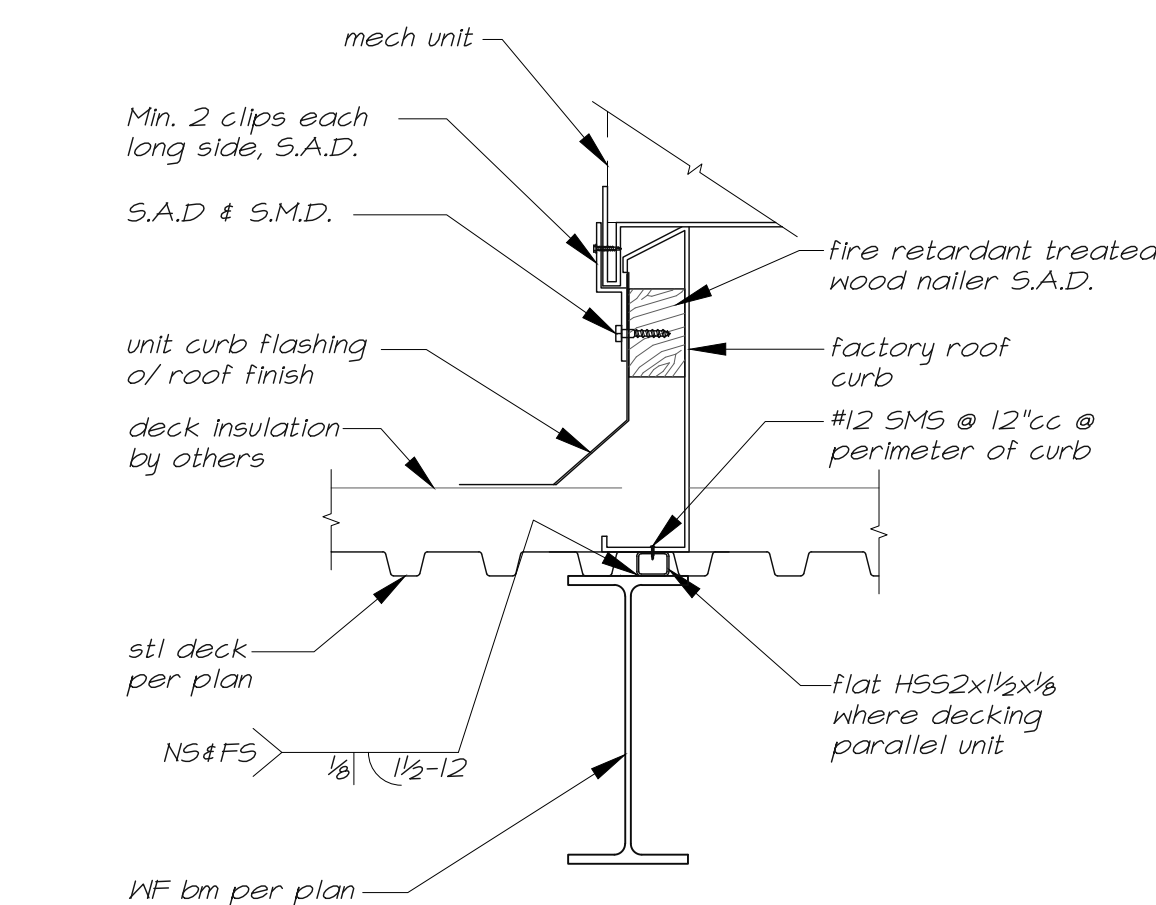
**1** Detail  
 55.5 1"=1'-0" stud to structure @ floor



**2** Detail  
 55.5 1"=1'-0" parapet stanchion



**3** Detail  
 55.5 1"=1'-0"



**4** Detail  
 55.5 1"=1'-0" mechanical curb



555 West Benjamin Holt Drive, Suite 423  
 Stockton, California 95270  
 P: (209) 952-5850  
 F: (209) 952-2442  
 E: hello@architechnica.net  
 www.architechnica.net

© 2024 ARCHITECTURA  
 2479 Sunrise Blvd.  
 Gold River, CA 95670  
 Tel: (916) 631-3030  
 Fax: (916) 631-8996  
 Web: www.bevier.net  
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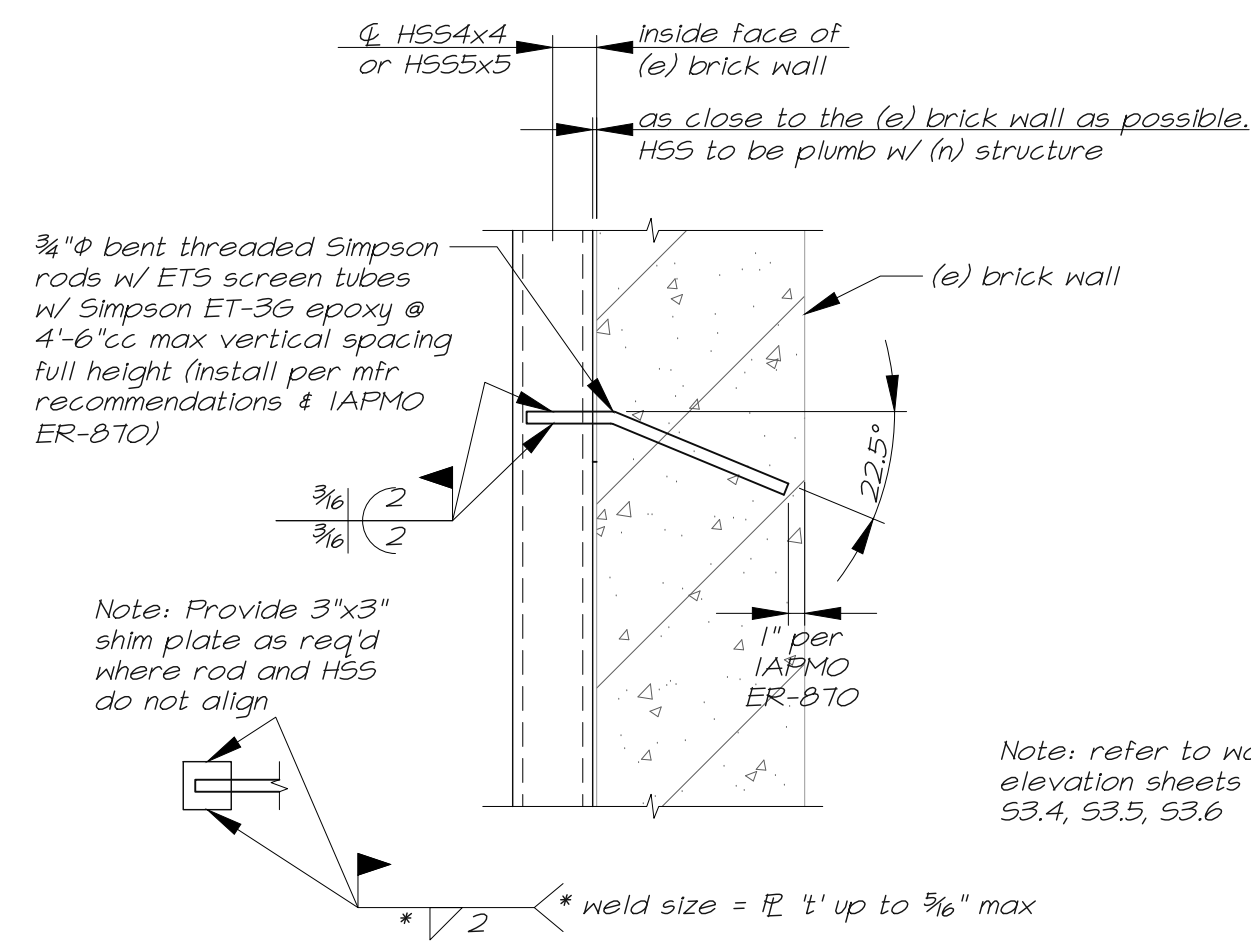
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Bid Set 11/06/2024	
Addendum #5 12/04/2024	

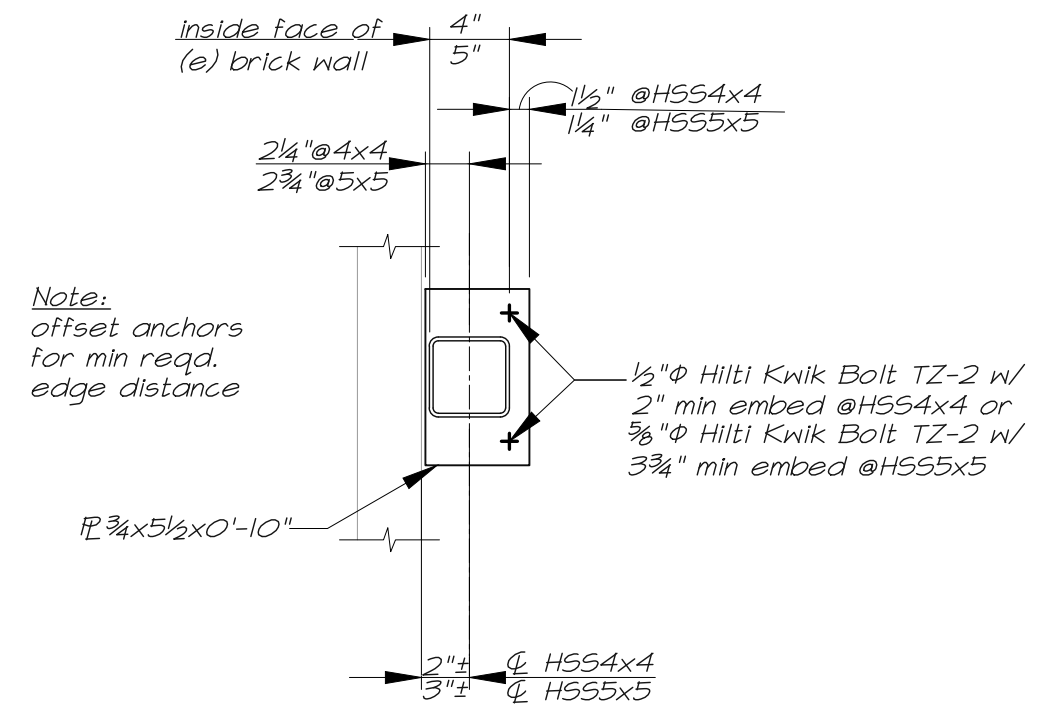
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 ISSUE SET: BID SET  
 ISSUE DATE: 12.04.24  
 DRAWN BY: TB, JRW, MC

**DETAILS**

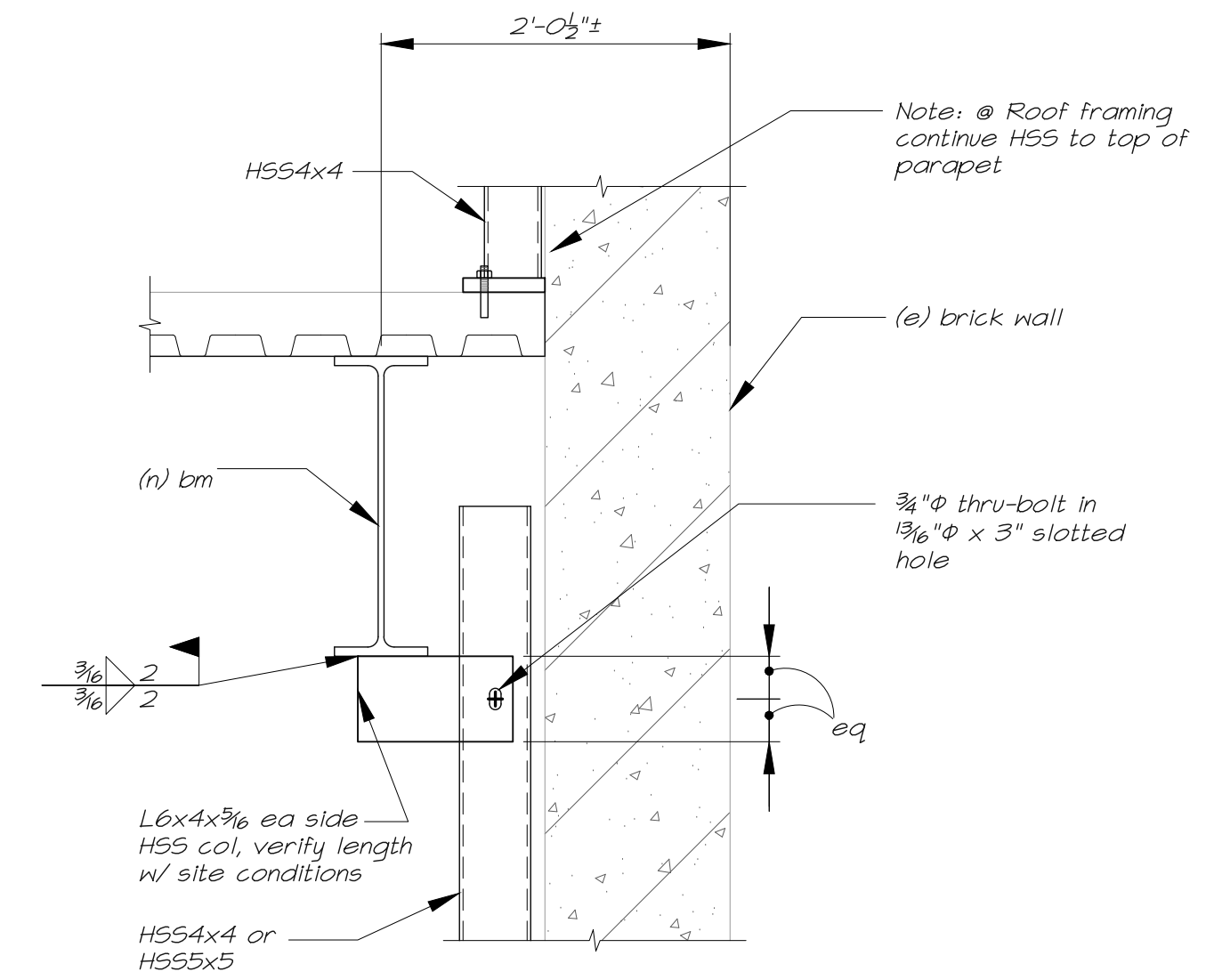
**S5.5**



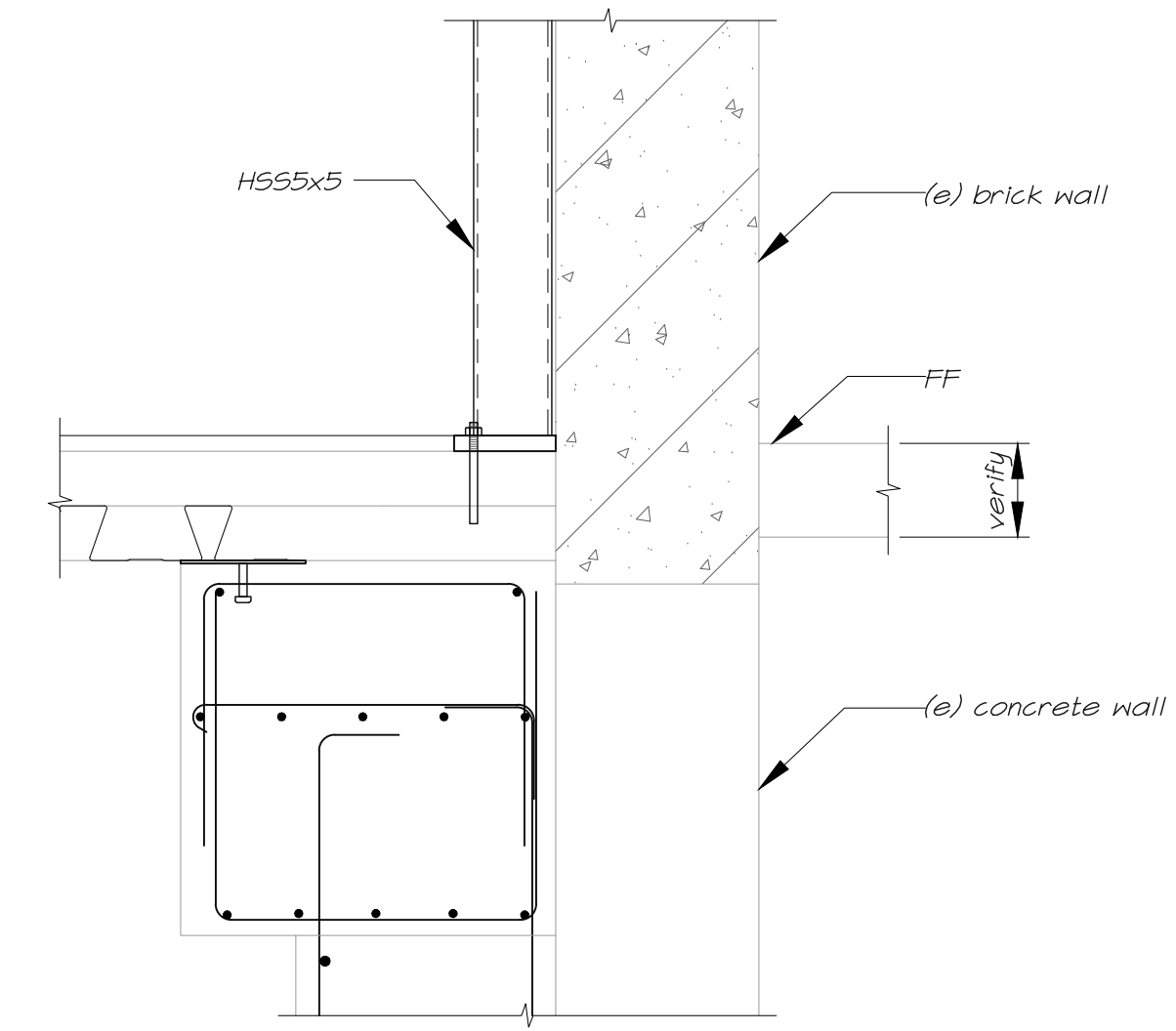
8  
55.6  
1"=1'-0"



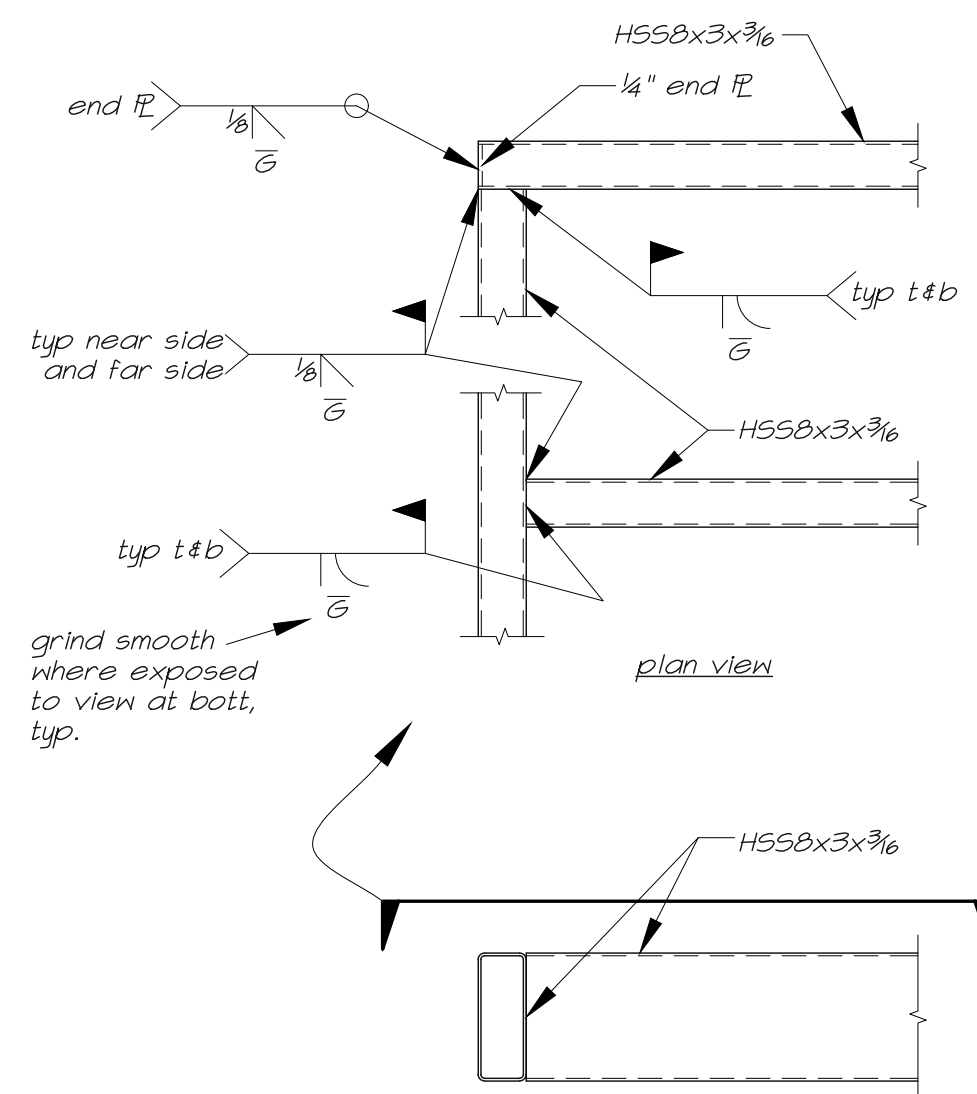
9  
55.6  
1"=1'-0"



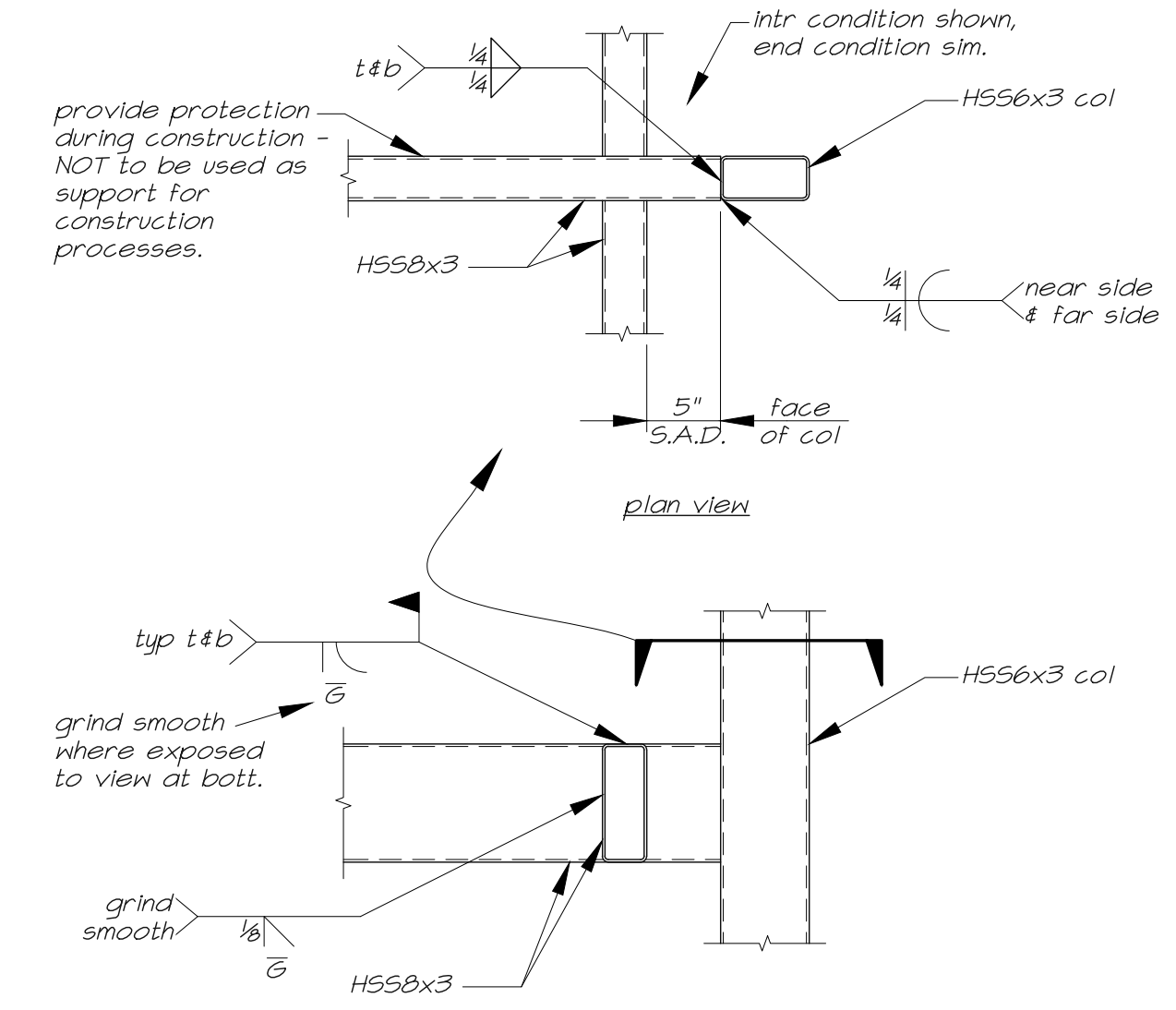
11  
55.6  
1"=1'-0"



7  
55.6  
1"=1'-0"



3  
55.6  
1"=1'-0"



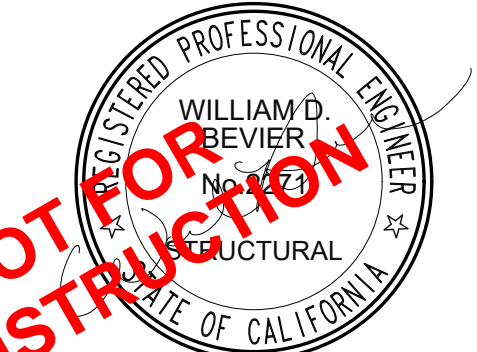
4  
55.6  
1"=1'-0"



555 West Benjamin Holt Drive, Suite 423  
 Stockton, California 95207  
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2479 Sunrise Blvd.  
 Gold River, CA 95670  
 Tel: (916) 631-3030  
 Fax: (916) 631-8996  
 Web: www.bevier.net  
 Bevier Job No: 23011



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 CONSULTANT



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 STOCKTON, CA 95202

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DETAILS